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Oskarshamn site investigation Drilling of cored borehole KSH01

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March 2004

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Keywords: Core drilling, Bedrock, Measurement while drilling, Flushing water monitoring, Water sampling, Wireline tests, Air-lift pumping, Telescope hole.

This report concerns a study which was conducted for SKB. The conclusions and viewpoints presented in the report are those of the authors and do not necessarily coincide with those of the client.

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1 Introduction

SKB, the Swedish Nuclear Fuel & Waste Management Company, performs site investigations in order to evaluate the feasibility of locating a deep repository for spent nuclear fuel /1/. The investigations are performed in two Swedish municipalities: Östhammar and Oskarshamn. The investigation area in Oskarshamn is situated close to the nuclear power plant at Simpevarp /2/, see Figure 1-1.

Drilling and investigations in boreholes are fundamental activities in order to facilitate characterisation of rock and groundwater properties at depth. Three main types of boreholes are produced: core drilled boreholes, percussion drilled boreholes in hard rock and boreholes drilled through unconsolidated soil.

The drilling of KSH01 and all related on-site operations were performed according to a specific Activity Plan (AP PS-02-004, SKB internal document).

KSH01 was the first deep cored borehole within the Oskarshamn site investigation.

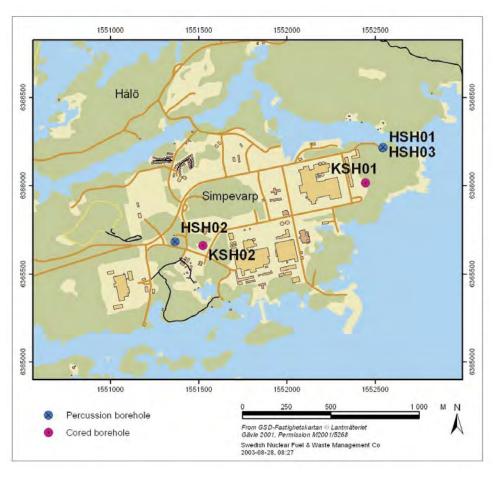


Figure 1-1. Locations of the first drillholes in the Simpevarp subarea. Percussion drilled holes are marked "HSH" and cored boreholes are marked "KSH". This report will be concerned with activities performed during the drilling of KSH01.

2 Objective and scope

This report will describe the methods employed and the results achieved during the drilling of KSH01. A number of related activities, such as wireline hydraulic tests, water sampling and monitoring of drilling parameters that were performed in conjunction with drilling will also be reported here.

The main reasons for drilling borehole KSH01 was to gain geological information at depth of the eastern part of the Simpevarp peninsula and to facilitate a means for further investigation at depth.

The 1,000 m deep hole, KSH01A, was constructed as what is known in SKB parlance as a "telescope hole of chemistry type".

"Telescope" means that the hole was built with an upper 100 m percussion drilled part of wider diameter and a lower core drilled section.

"Chemistry type" means that the hole was planned to be subjected to a complete chemical characterization, which in turn put specific requirements on the cleaning level for down-hole equipment.

A separate 100 m cored hole, KSH01B, was drilled close to the telescope hole in order to obtain a complete coverage of core from surface to depth.

3 Overview of the drilling method

The drilling and all related on-site operations were performed according to a specific Activity Plan (AP PS-02-004, SKB internal document). Reference is given in the activity plan to procedures in the SKB Method Description for Percussion Drilling (SKB MD 610.003, Version 1.0), SKB Method Description for Core Drilling (SKB MD 620.003, Version 1.0) and relevant method instructions for handling of chemicals, surveying and evaluation of cuttings. Method descriptions and instructions are SKB internal documents.

3.1 The SKB telescope drilling method

In brief, the method is based on the construction of a larger diameter percussion hole (200 mm diameter) to a length of 100 m followed by a cored section to full length.

The purpose of the upper section is to allow the removal of water from the hole by air-lift pumping and minimize the introduction of foreign substances (flushing water and cuttings) to the surrounding bedrock, to enable the use of submersible pumps for tests and to facilitate the installation of multi-packer systems for ground water pressure recordings.

After percussion drilling 0–100 m, equipment for air lift pumping is installed in the borehole. The air-lift pumping will create a pressure drawdown and help remove water and cuttings while core drilling between 100 m and 1,000 m, see Figure 3-1. The effect of drawdown is dependent on the depth and capacity of major groundwater conductors.

During the core drilling phase several measurements and sampling exercises are performed through the drilling monitoring system (DMS), wireline tests for hydraulic purposes and the collecting of discrete chemical samples.

After the core drilling is completed to full length, a conical guide of stainless steel is installed between the telescope part and the deeper core drilled part, see Figure 3-2.

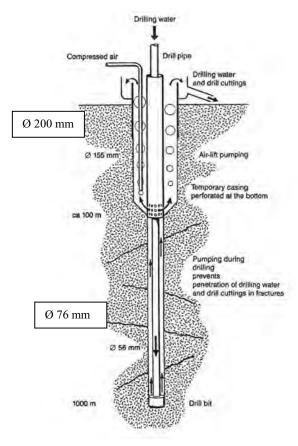


Figure 3-1. Overview of the telescopic drilling method with air-lift pumping for retrieval of drilling water and cuttings.

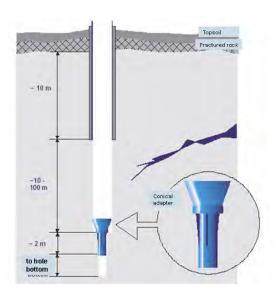


Figure 3-2. Installation of the conical guide.

3.2 The flushing water system

The handling of flushing water includes a source of water, tanks and pumps for introduction into the hole, air-lift pumps for raising the water from the bottom of the telescope part to surface. The return water is led to settling containers before discharge, see Figure 3-3.

Nitrogen gas is bubbled through the drilling water to remove dissolved oxygen. This is done to avoid introduction of oxygen to the formation water and thereby disturbing the virgin chemical properties.

In order to monitor possible mixing of formation and drilling water, a uranine tracer dye is added to the drilling water to a fixed concentration, see Figure 3-4.

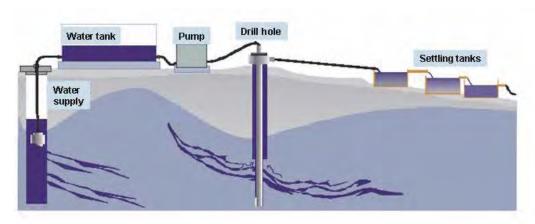


Figure 3-3. The flushing water system from source to discharge point.

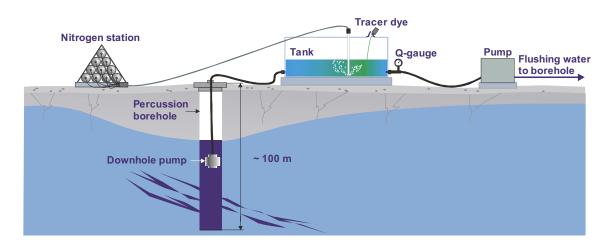


Figure 3-4. Preparation of flushing water. Uranine is added to the water in the tank as a tracer dye. Nitrogen is bubbled through the water to remove dissolved oxygen.

3.3 Measurements and sampling during drilling

Drilling cuttings are collected manually during percussion drilling. If water occurs in the hole, observations of changes in return water flow and colour are noted and water samples are taken. The drill penetration rate is logged manually.

At the final stage of the percussion drilling phase a recovery test is made by blowing in compressed air to remove all water in the hole. The recovery of the water table is then measured manually.

The sampling and measurements during the core drilling phase consist of:

- The drilling monitoring system.
- Wireline tests and water sampling.
- Discrete water sampling.

Drilling monitoring system (DMS)

Drilling is monitored on-line by continuous registration of drill rig parameters (logged every centimetre of bit penetration) and flushing water parameters (logged every 10 seconds). The data is compiled into a database called <u>drilling monitoring system</u> (DMS).

Wireline tests and water sampling

Tests for hydrogeological purposes (pumping tests and measurements of absolute pressure) and sampling for water chemistry (if possible) are performed every 100 m down the hole. The wireline tests are done in accordance with SKB Method Description MB 321.002, SKB Internal Document.

Discrete water sampling

Water samples of flushing and return water are taken at between 10 and 15 m intervals for analysis of uranine content and conductivity.

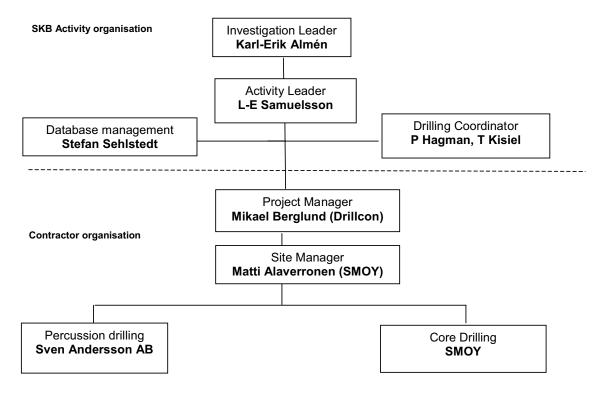
4 Contractor and equipment

4.1 Contractor

The main contractor for drilling was Drillcon Core AB, with subcontractor for core-drilling Suomen Malmi OY (SMOY), Finland and subcontractor for percussion drilling Sven Andersson AB.

An overview of the organisation for the drilling activity is given in Table 4.1.

Table 4.1. Drill activity organisation.



4.2 Percussion drilling equipment

The equipment used was a Puntel MX1000 percussion drill rig with an Atlas Copco XRVS 455 Md air compressor. The down-the-hole hammer was a Secoroc 8" or 6" and the drill rods were Driqoneq 114 mm. The casing utilized was St 37 406.5 mm ("NO-X 365"), SS 2343 208x4 mm and 273x4 mm. The casing dimensions are presented as outer diameter and thickness.

4.3 Core drilling equipment

Core drilling in KSH01A and B was made with a B 20 P Atlas Copco fully hydraulic machine fitted with a modern and environmentally adapted diesel engine. The rods were of the type Corac N3/50 NT with a 76 mm wireline triple tube core barrel system which gives a core diameter of 50.2 mm.



Figure 4-1. Compressor and drill rig. Gas tubes for nitrogen saturation and the flushing water tank can be seen in the lower left corner.

4.3.1 Drilling monitoring system

During the core drilling phase continual monitoring was made of several flushing water parameters (oxygen content, electrical conductivity and water flow) and measurement-while-drilling (MWD) parameters (bit feed force, rotary pressure, rotation, penetration rate, drill rig water pressure and bit position).

The data is compiled into the DMS database.

The drill rig (MWD) parameters include:

- Rotational pressure (bar).
- Bit force (kN).
- Flush water flow in (l/min).
- Water pressure at bit (kPa).
- Rotation (rpm).
- Penetration rate (cm/min).

The flushing water parameters include:

- Water level in the telescope part of the borehole (kPa).
- Oxygen level of flushing water in (mg/l).
- Flow of return flushing water (l/min).
- Electrical conductivity of flushing water in (mS/m).
- Electrical conductivity of return water (mS/m).
- Air pressure (kPa).

Data from on-line monitoring of flushing water parameters were stored on two different logging units (CR10 and CR23). A separate logging unit was used for the measurement-while-drilling (MWD) dataset. The data from the loggers was downloaded either continuously (CR10 and CR23) or by diskette or CD-ROM to the DMS database.



Figure 4-2. The CR23 logging unit for parameters "air-pressure" and "electrical conductivity".

4.3.2 Wireline measurements equipment

The method is developed in such a way that water sampling, pump tests and measurements of absolute pressure can be made without having to lift the drillstem. The wireline probe equipment has been especially developed by SKB.

Measurements are made with a wireline probe as specified in method description SKB MD 321.002 (SKB internal document). The principal components of the system are:

- an inflatable packer,
- a probe fitted with pressure gauges for the test section and for the packer,
- a water sampler,
- a submersible pump (placed in the upper part of the drillstem),
- a flow meter (placed at the ground surface).

The probe and packer are lowered through the drillstem into position at the drill bit. The test section is between the lower end of the packer and the bottom of the borehole, see Figure 4.3.

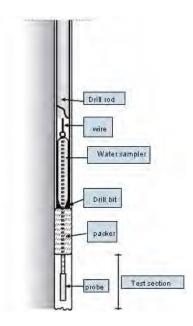


Figure 4-3. The wireline probe and its emplacement in the hole.

Before the pumping tests, the measurements for absolute pressure are started and a leakage test of the drillstring, is made.

Hydraulic tests performed during drilling are generally affected to some degree by disturbances caused by the drilling operations. Transients from changes in pressure, temperature and salinity might affect the hydraulic response curves.

Pumping tests

The wireline probe is emplaced at the bottom of the drillstem. A submersible pump is lowered into the upper part of the drillstem at a length of about 40 m. The test section is hydraulically connected to the drillstem by opening a valve in the probe at a predetermined pressure. This creates a passage between the test section and the water column in the drillstem. The packer remains expanded during the entire test. Water is pumped from the drillstem and the pressure in the test section and packer are recorded in a data logger in the probe. The pumped surface flow rate is recorded to a data logger on the ground surface. The pressure transducer is situated 1.10 m below the lower end of the packer. The test consists of a pressure drawdown phase and a recovery phase. Typically the pumping time is three hours with a recovery of the same duration. However, the duration is sometimes adapted to the hydraulic situation of the tested section. The tests are normally carried out in sections of about 100 m length.

Absolute pressure measurement

The wireline probe is placed in position at the drill bit. The packer is inflated and the pressure build-up in the test section is recorded for a period of at least eight hours, typically this is done overnight. The measuring range for the pressure gauge is 0-20 MPa ($\pm 0.05\%$ FSD).

Water sampling

The wireline probe is placed at the bottom of the borehole. The single-packer then isolates the test section between the single-packer and the bottom of the borehole. The water volume in the section is renewed at least three times by pumping water out of the drillstem. The water at depth is replaced by formation water and a sample is collected. The wireline probe is then brought to the surface. The theoretical sample volume is 5 litres.

Pumping tests and water sampling are normally performed as an integrated activity.

4.3.3 Equipment for deviation measurements

Deviation measurements were performed in the boreholes using a Reflex MAXIBORTM (non-magnetic) optical equipment.

5 Execution and results

5.1 Percussion drilling

Percussion drilling, reaming and installation for air-lift pumping was made between August 22 and October 1, 2002.

5.1.1 Preparations

A cement pad for emplacement of drill rig, fuel container and compressor was built.

Cleaning of all DTH-equipment was done with a high-capacity steam cleaner.



Figure 5-1. Drill site preparations.

5.1.2 Construction

The construction of the upper telescope section (0–100 m) was made in steps:

- Drilling through the overburden to 2.2 m depth with NO-X 365 mm equipment. This gave a 420 mm diameter hole and a casing diameter of 406.5 mm.
- Inner supportive casing for guidance for the drill string was mounted and a pilot percussion hole of 161 mm was drilled to a depth of 12.12 m (to fresh rock). The inner casing was removed and the pilot hole was reamed in two steps to 250 mm and then to 350 mm diameter.
- The casing (273x4 mm diameter) through fractured rock was installed and gap injection with cement was made as described in Figure 5-2 and 5-3.
- A new set of inner supportive casing was installed and a new pilot hole to 100.34 m was drilled with a diameter of 161 mm. The inner guiding tubes were removed.
- The stainless steel casing of diameter 208x4 mm was installed to a depth of 12.10 m and gap injected with cement.
- The hole was reamed between 12.12 m and 100.24 m length to a diameter of 198–200 mm (at 12 m the diameter of the bit was 200 mm and at a 100 m the diameter of the drill bit was 197.8 mm).

The main steps are shown in Figure 5-2.

A detail of the gap injection method is given in Figure 5-3.

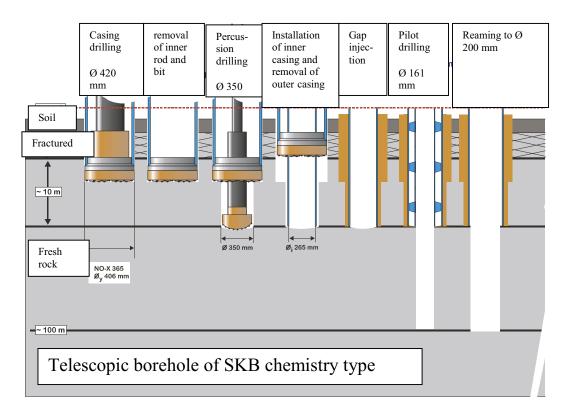


Figure 5-2. Method for drilling and installation of the first 100 m. In addition to the steps shown in the figure an extra set of casing (208 mm) was emplaced in KSH01A.

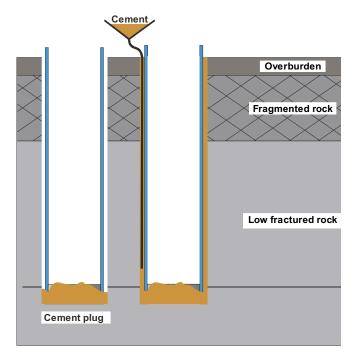


Figure 5-3. Gap injection technique- a cement plug is emplaced at the bottom and allowed to harden. The gap filling cement is introduced between the casing and the rock wall.

5.1.3 Sampling and measurements

Sampling and measurements done by the drill coordinators during drilling included:

- Collecting of drill cuttings was made by taking three grab samples over a length of three metres resulting in one composite sample per three metres. The samples were stored for later logging. A soil sample was collected between 0.4 and 1.4 m.
- Penetration rate (expressed as seconds per 20 cm) was manually recorded.

The penetration rate is presented in Figure 5-4.

No noticeable changes in water flow and colour of return water could be recorded.

A recovery test of the 160 mm hole was made when the percussion drilling was completed to full depth. The hole was rinsed from drill cuttings by blowing air with the compressor at maximum capacity for 30 minutes. The inflow of water to the borehole was calculated based on water level measurements.

Percussion drill penetretion rate KSH01A

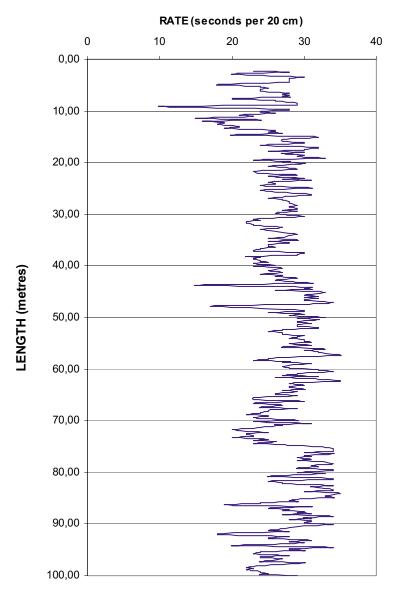


Figure 5-4. Penetration rate for percussion drilling.

Table 5-1. Recovery test – water level measurements in the 160 mm hole (0–100.34 m).

Date Time	Water level (metres below top of casing)	Calculated inflow (mL/min)
2002-08-30 07:30	99.64	
2002-09-03 08:30	97.43	8
2002-09-04 09:18	97.18	7
2002-09-05 09:15	96.15	9

5.2 Core drilling

Core drilling of holes KSH01A and KSH01B was conducted between September 26, 2002 and January 25, 2003.

The main work in KSH01A after pre-drilling to 100 m depth consisted of the following steps:

- preparations for core drilling,
- drilling,
- wireline tests,
- drill monitoring,
- measurements of flushing water,
- deviation measurements.

A summary of the two holes is given in Tables 5-2 (KSH01A) and 5-3 (KSH01B).

Table 5-2. KSH01A summary.

General	Technical		
Name of hole: KSH01A	Percussion drill rig Puntel MX 1,000		
Location: Simpevarp, Oskarshamn	Percussion drill hole diameter 200 mm		
Municipality, Sweden	Percussion hole length 100.24 m		
Contractor for drilling Drillcon AB Subcontractor percussion drilling Sven Andersson AB Subcontractor core drilling Suomen Malmi OY (SMOY) Drill start date August 22, 2002	Core drill rig B 20 P Atlas Copco		
	Core drill dimension 76 mm		
	Cored interval 100.24–1003 m		
	Average core length retrieved in one run 2.41 m Number of runs 375		
			Diamond bits used 7 Average bit life 128 m
	Completion date January 17, 2003	Position KSH01A (RT90 RH70) at top of casing: N 6366013,45 E 1552442,99 Z 5,31 (masl)	
		Azimuth (0–360)/ Dip (0–90) 174,148 / –80,598	
	Position KSH01A (RT90 RH70) at top of casing: N 636568,937 E 1552402,273 Z –963,699 (masl)		
	Azimuth (0–360)/ Dip (0–90) 210.28 / –69.57		

Technical data from borehole KSH01A is given in Figure 5-5.

Technical data

Borehole KSH01A

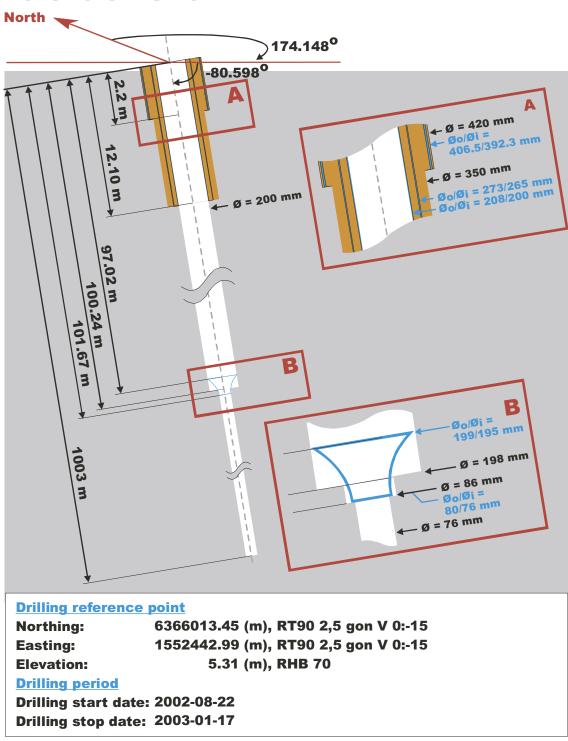


Figure 5-5. Technical data from KSH01A.

In order to retrieve a complete core from surface to full depth an additional drilling was made to 100.25 m about one metre from the percussion drill hole. The core and the borehole are called KSH01B.

Soil drilling was done with HW 114.3 mm diameter to 1.35 m depth, which corresponds with the actual soil depth. From 1.35 m to 6.04 m the hole was drilled with 76 mm diameter and reamed to 98 mm. A stainless steel casing (89/77 mm) was installed and grouted.

Drilling with 76 mm diameter was continued to 100.25 m. The geology of KSH01B is presented in Appendix 1.

The reference point for down-hole measurements was the concrete pad (the top-of-casing is 24 cm above the concrete pad).

Table 5-3. KSH01B Summary.

Name of hole: KSH01B	Core drill rig B 20 P Atlas Copco		
Location: Simpevarp, Oskarshamn	Core drill dimension 76 mm		
Municipality, Sweden	Cored interval 0–100.25 m		
Contractor for drilling	Position KSH01B (RT90 RH70) at concrete pad: N 6366014,039 E 1552442,890 Z 5,203 (masl)		
Drillcon AB			
Subcontractor core drilling	Azimuth (0–360)/ Dip (0–90) 177,762 / –87,880 Position KSH01B (RT90 RH70) at bottom of hole:* N 6366010,343 E 1552443,035 Z –94,733 (masl) * calculated from starting point (not measured)		
Suomen Malmi OY (SMOY)			
Drill start date January 17, 2003			
Completion date January 25, 2003			
completion date candally 20, 2000			

Technical data from borehole KSH01B is given in Figure 5-6.

N.B. Unless otherwise stated the rest of this report will deal with results from KSH01A.

Technical data

Borehole KSH01B

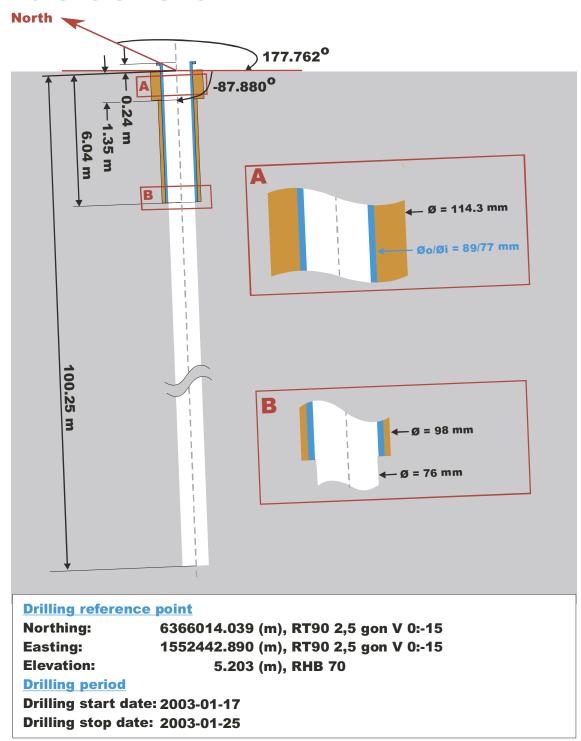


Figure 5-6. Technical data from KSH01B.

5.2.1 Preparations

The preparations for the core drilling of KSH01A consisted of installation of air-lift pumping equipment and supportive casing for alignment of the core drill rods.

The installation of supportive casing was done in two steps:

- An outer casing with a diameter of 98/89 mm, fitted with fins to a diameter of 193 mm was installed to a length of 100.28 m.
- After drilling to a length of 100.81 m with a diameter of 86 mm, an inner supportive casing with diameter 84/77 mm was installed.

The supportive casings have a perforated section between 90 and 100 m length so that water from the borehole can be evacuated to the surface through air-lift pumping. A pressure meter for monitoring of the water level was emplaced at a length of 90 m.

The discharge header was fitted to collect the return water.

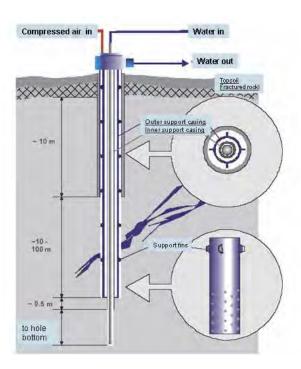


Figure 5-7. In the telescopic part of the drill hole a temporary installation is made with casing tubes for support and alignment and equipment for air-lift pumping. In the uppermost part the return water discharge header is mounted. The water discharge is led to the settling containers.



Figure 5-8. Fitting the flushing water discharge header. The supportive casing and the hoses for air-lift pumping can also be seen.

The return water from drilling was led to a series of containers in order to collect sludge before discharge to the Baltic, see Figure 5-9.



Figure 5-9. Aligning the flushing water discharge containers. The sludge and cuttings is allowed to settle in the containers.

5.2.2 Drilling

Drilling was made with T-86 equipment between 100.24 m and 100.81 and produced a core of 72 mm diameter.

Core drilling with 76 mm triple-tube, wireline equipment was conducted from 100.81 m to 1,003 m which gave a core of 50.2 mm diameter.

A total of seven drill bits were used for KSH01A, see Figure 5-10.

The elasticity of the drillstem expressed as the difference in length between a hanging and a standing set of drill rods is presented in Figure 5-11. The method for conducting this test was to note the length of the drillstem standing at the bottom of the hole without any downward pressure on the string. A small, consistent rotational torque was applied to the drillstem and the rods slowly lifted. When rotation of the stem could be observed, the drill string was considered as hanging freely and the difference in length was noted.

No further analysis of the non-linear behaviour of the data in Figure 5-11 has been done.

Further results from drill monitoring in drill penetration rate and various measurements will be presented in Chapter 5.3 "Drilling monitoring results" and in Appendix 1.

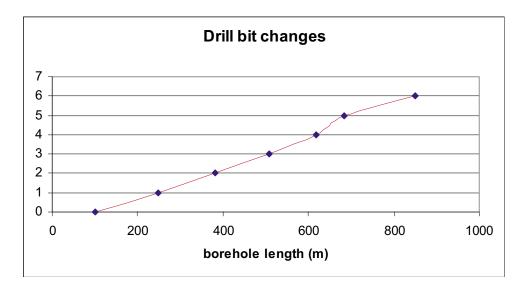


Figure 5-10. Changes of drill bit during wireline drilling.

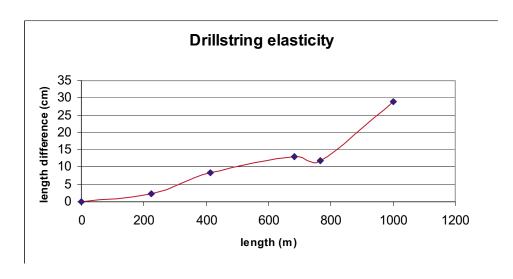


Figure 5-11. Elasticity of the drill rods expressed as the difference between the length of a hanging and a standing drillstem.

5.2.3 Wireline tests

Sampling of groundwater in borehole KSH01A was conducted with wireline tests in conjunction to core drilling. The main purpose for the sampling was to characterize the hydrochemistry as well as the hydrology in the bedrock when the conditions are less affected by hydraulic short circuiting in the borehole.

A number of wireline tests were conducted:

- Five tests for absolute pressure.
- Nine pumping tests were conducted at various intervals, only six gave useful results.

The hydrological results from wireline tests are shown in Section 5.5.

A recovery test of the hole was made after drilling to full depth. The results are given in Section 5.5.

In total, three water samples (SKB nr: 3824, 3825 and 3831) were collected with the wireline probe during pumping tests in connection with core drilling in KSH01A, the times and lengths for the samples are given in Table 5-4.

Sampling and analysis were performed according to the SKB class 3 with all options included. The parameters B_{10} and Cl_{37} were not analysed. The results are shown in Section 5.6 and Appendix 2. The methodology is described in Appendix 3.

A total of 45 samples for laboratory testing of uranine content and electrical conductivity in flushing and returning water were taken along the borehole. The results are shown in Section 5.6.

Table 5-4. Sample dates and length during core drilling in KSH01A.

SKB number	Date	Test section, length (m)	SKB Chemistry class
3824	2002-10-24	197.00–313.42	3
3825	2002-11-16	585.00-593.00	3
3831	2002-11-20	531.00-619.42	3

5.2.4 Drill monitoring system DMS

Drill hole KSH01A was the first real test for the monitoring system and some problems were encountered both with logging units and data transfer. The problems were corrected as they occurred or as they were discovered. The work with establishing routines for quality assurance of DMS data is still going on in March 2004. The results presented in this report have been checked in accordance with routines that are in preparation. Results from monitoring are presented in Section 5.3 and Appendix 1.

5.2.5 Measurements of flushing water

The amount of flushing water entering the hole is counted manually as a record of the number of 5 m³ tanks is kept throughout the drilling process. The return water is logged via the drilling monitoring system.

As can be seen in Figure 5-12 the amount of flushing water entering and leaving the hole is very well balanced. The flushing water consumed during drilling of KSH01A amounted to 1,100 m³, giving an average consumption of 1.22 m³ per metre drilled.

The weight of cuttings in the settling containers could not be established with any confidence as the procedure was not fully established at the time of drilling.

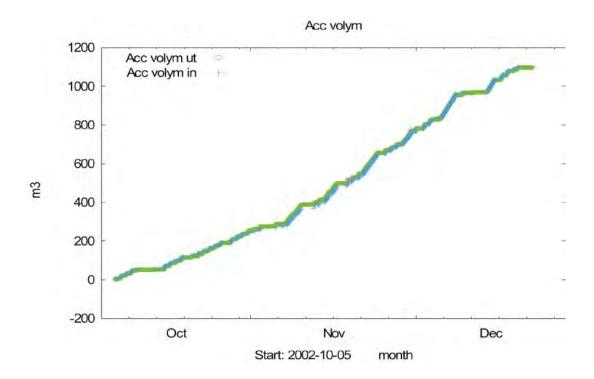


Figure 5-12. Flushing water balance as recorded by the drilling monitoring system over time in KSH01A.

5.2.6 Deviation measurements

Deviation measurements with the Maxibor method were made five times during drilling in KSH01A.

The results of the final run covering the entire borehole length are given in Appendix 4.

5.2.7 Borehole completion

Reaming of depth reference slots was done at intervals as shown in Table 5-5. The purpose for the depth reference slots was to be used for depth calibration of down-hole equipment for subsequent investigations in the hole.

After all core drilling activities were concluded, the air lift pumping equipment was removed and a steel conical guide was installed between 97.02 m and 101.67 m depth. The guide tapers from an inner diameter of 195 mm to 76 mm.

The borehole was secured by mounting a lockable steel cap fastened to the concrete pad. All equipment was removed, the site cleaned and a joint inspection was made by representatives from SKB and the contractor to ensure that the site had been satisfactorily restored.

Table 5-5. Depth reference slots.

Depth (m)	Depth (m)	
110.000	550.000	
150.000	600.000	
200.000	650.000	
250.000	700.000	
300.000	750.000	
350.000	800.000	
400.000	850.000	
450.000	899.000	
500.000	950.000	

5.3 Drilling monitoring results

The DMS database contains substantial amounts of drilling monitoring data. A selection of results primarily from the monitoring of the flushing water parameters are presented in Figures 5-13 through 5-15 below.

Selected parameters from the drill rig (MWD parameters) are presented in Appendix 1. The MWD parameters require some explanation:

- Drillability ratio- this parameter is defined as penetration rate divided by feed force.
- Flushing water ratio- this is defined as flushing water flow divided by flushing water pressure.
- Water pressure (of the water entering the drillstem).
- Flushing water flow (flow of ingoing water).

- Penetration rate (rate of drill bit penetration as measured on the surface on the drillstem).
- Hydraulic indication- this parameter is defined as penetration rate divided by flushing water flow.

In order to maintain reasonable size data files, a reduction in the number of points incorporated in the pictures has been done in Figures 5-13 through 5-15.

Figure 5-13 depicts the drill bit position (green) over time and the water level (red) in the telescope part of the drill hole. The water level, given as pressure of the overlying water column reflects the air-lift pumping activity in the hole.

Figure 5-14 shows the flushing water flow (green) entering the hole and the return water flow (red).

Figure 5-15 shows the conductivity of the ingoing flushing water (yellow). The gradual increase reflects the increasing salinity over time from the water source (HSH03). The conductivity of the return water (green) shows high peak values that should, in theory, correspond with the presence of saline formation water at depth. The oxygen content of the flushing water (red) is typically low. It should be remembered that the oxygen content is registered also during periods when there is no actual water flow.

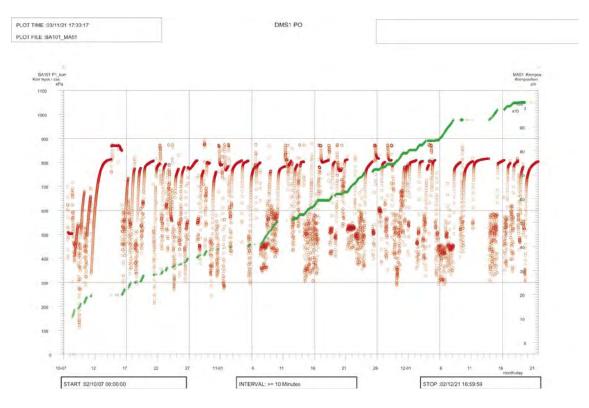


Figure 5-13. Drill bit position (green) and water level from air-lift pumping (red). The water level is expressed as the pressure in kPa of the water column overlying the pressure gauge. The pressure is corrected for the ambient air-pressure has been subtracted. The pressure gauge is emplaced at 90.0 m borehole length. The drill bit position is given in cm \times 10³.

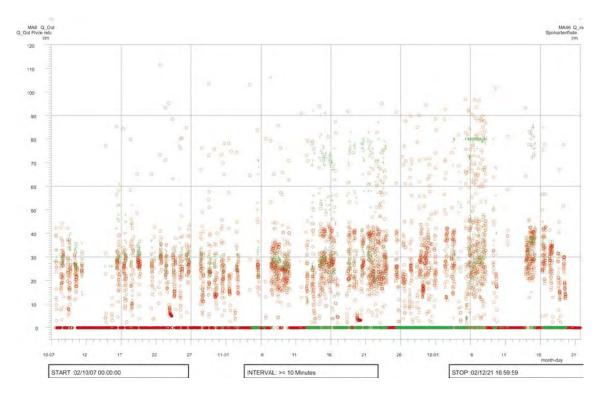


Figure 5-14. Flushing water flow (green) and return water flow (red) in litres per minute.

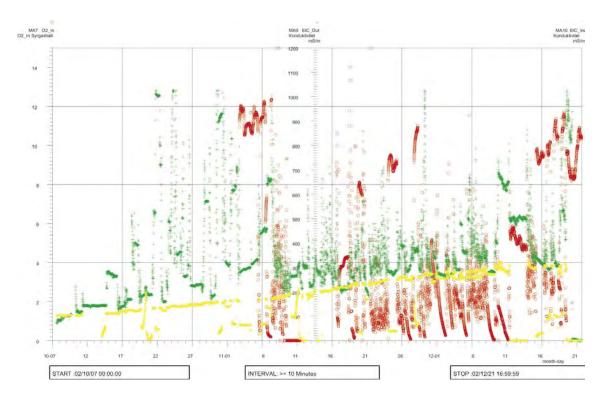


Figure 5-15. Conductivity of flushing water in mS/m (yellow) and return water (green). The oxygen content in mg/l of the flushing water (red) is also shown.

5.4 Geology

The geological results, based on the Boremap logging /3/, are shown in Appendix 1.

The dominant rock types are quartz monzodiorite and dioritoide with minor intercalations of mafic rock and granite. The amount of granite increases below 620 m, to dominate the core between 800 and 920 m. The upper 630 m contain oxidized sections, typically in conjunction with an increased fracture frequency.

The fracture frequency is normally less than 5 fractures per metre. Minor crushed sections do occur, notably at 570 m, 630 m, 720 m, 830 m, and 900 m. An interval with slightly higher than average fracture frequency occurs at 250 m, 430 m and between 550 m and 650 m. At depth, below 730 m, the fracture frequency is greatly reduced.

The borehole plot as can be seen in Appendix 1 has not been adjusted for the difference in reference levels between the two holes, nor is it compensated for differences in azimuth or dip.



Figure 5-16. Unloading the core barrel.

5.5 Hydrogeology

5.5.1 Hydraulic tests during drilling (wireline tests)

Results from pumping tests and absolute pressure measurement from borehole KSH01A are presented in Tables 5-6 and 5-7.

For the pumping tests the flow value of the end of the drawdown phase is used for calculating the specific capacity (Q/s) and transmissivity (T_M), where "Q" is the flow rate in L/min, "s" is the drawdown in kPa and " T_M " is the transmissivity according to /4/.

A total of nine pumping tests were performed but only six were good enough for calculating the transmissivity. From two of the other tested sections a rough specific capacity was calculated while one of the sections did not give any usable data.

Pumping test plots are given in Appendix 5.

Absolute pressure measurements were performed in five sections as specified in Table 5-7 and Appendix 6.

After packer inflation the pressure stabilization phase often displays different types of transient effects, both of increasing and of decreasing pressure. The reason for these transients is not known. They might be attributable to previous disturbances in the borehole caused by the drilling operations such as pressure, salinity and temperature.

Table 5-6. Pumping tests with wireline probe in KSH01A.

Tested section [m]	Comments	Q/s [m²/s]	T _{Moye} [m²/s]
273.00–284.10	Pumping is started with too high rate which generates a continuous decline in rate and pressure The pressure recovery is initially affected by backflow from the hose since no check valve is installed in the pump. Thereafter the water level is recovering in a linear fashion. A rough steady state evaluation is done on the drawdown phase.	5.8·10 ⁻⁷	5.5·10 ⁻⁷
197.00–313.42	Flow decreasing flow 7.6–4.9 L/min and pressure stabilises quickly at about 1,750 kPa! The pressure recovery is initially affected by backflow from the hose since no check valve is installed in the pump. Thereafter the water level recovers linearly. Stationary evaluation is done on the drawdown phase.	6.9·10 ^{−6}	9.2·10 ⁻⁶
311.95–409.55	A drawdown of 575 kPa is generated in the test section but no flow data is recorded. The pressure recovery is initially affected by backflow from the hose since no check valve is installed in the pump. Thereafter the water level recovers linearly with an inflow of 0.17 L/min. A rough specific capacity may be estimated if it is assumed that this inflow is generated by the previous drawdown of 575 kPa. This would yield a Q/s= 5·10-8m²/s. During the recovery the flow gauge registers a constant flow of 28 L/min. This suggests that the gauge was faulty.	5·10⁻⁵	See comments
408.00-532.64	There is a covariation of pressure in the test section, above the section and in the packer. Furthermore the pressure in the packer decreases to formation pressure at that level which causes its sealing not to perform adequately.		
	No results were calculated		
585.00-593.10	Stable values of pressure and flow	8.0.10-7	7.2·10 ⁻⁷
531.00-619.42	Stable values of pressure and flow	1.7·10-6	2.2·10-6
591.31–744.51	Relatively stable values of pressure and flow	4.1·10 ⁻⁷	5.6·10 ⁻⁷
742.00–842.10	No stable conditions were reached during the tests. A linearly decreasing flow from34 L/min to16 L/min caused a corresponding decrease in pressure from 7,185 to 6,608 kPa. A very small recovery of about 1.5 kPa during 25 min was recorded. This generates an inflow to the borehole of 0.017 L/min. A rough value on the specific capacity was estimated assuming that the generated drawdown of 480 kPa was causing the flow.	1.7·10-10	
842.00– 1,003.00	It proved impossible to obtain a stable flow or pressure. Throughout the pumping phase the flow was highly variable, fluctuating between 0 and 8 L/min approximately. The pressure decreases continuously in a stepwise manner. At the end of the pumping phase a certain degree of pressure stabilization is achieved during which Q=1 L/min and dh= 480 kPa. The pressure recovery is initially affected by backflow from the hose since no check valve is installed in the pump.		
		3.5·10 ⁻⁷	4.8·10 ⁻⁷

Table 5-7. Absolute pressure measurements.

Tested section [m]	Last pressure reading during build up	Duration of pressure build-up	Borehole length to pressure gauge
t1	[kPa]	[h]	[m]
102.00–197.00	1,017	24.3	103.10
312.10–392.78	2,997	12.6	313.20
492.00-593.00	4,773	5.6	493.10
741.00–842.10	6,689	4	742.10
842.00-1,003.00	8,436	19.1	843.10

5.5.2 Drawdown and recovery test

After drilling to full depth the borehole was flushed with water and airlift pumping tests were conducted. The drawdown and recovery of the water level was measured over a period of 24 hours. The recovery lasted about 14 hours. The inflow of water to the borehole after ceasing the flushing operation showed an inflow of 8.6 L/min at 10 min of recovery. The airlift pumping and recovery curve is shown in Figure 5-17.

From this a steady state transmissivity was calculated according to /4/, $T_M = 6.7 \cdot 10^{-6}$ m²/s and a specific capacity of Q/s = $4.1 \cdot 10^{-6}$ m²/s.

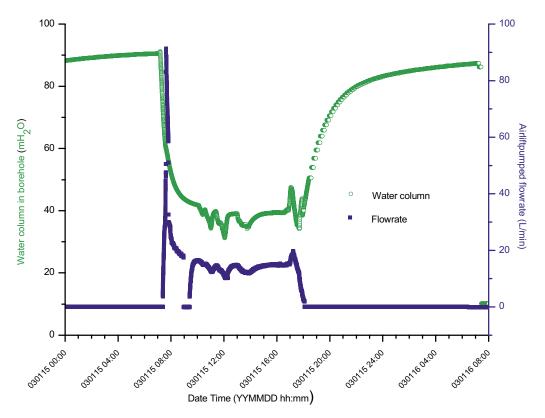


Figure 5-17. Air lift pumping in KSH01A showing flow rate and water column in borehole. The pressure transducer was positioned 90 m below top of casing, while the flowrate was measured at the surface.

5.6 Hydrochemistry

5.6.1 Results

In total, three water samples were collected from wireline tests in KSH01A during drilling. Only one of these samples (197–313 m) had a flushing water concentration less than 10%. In the other two samples the flushing water concentration was higher. The results from the samples from KSH01A are given in Appendix 2 together with results from sampling of the water source (HSH03). The concentration of Total Organic Carbon (TOC) in the samples collected in HSH03 was found to be in the range of 4.4–5 mg/L. These values were considered acceptable for the groundwater to be used as flushing water for the core drilled part of KSH01A without further filtration steps to lower the organic carbon content.

The results from sampling for control of the flushing water, uranine concentration and electric conductivity, are shown in Figure 5-18.

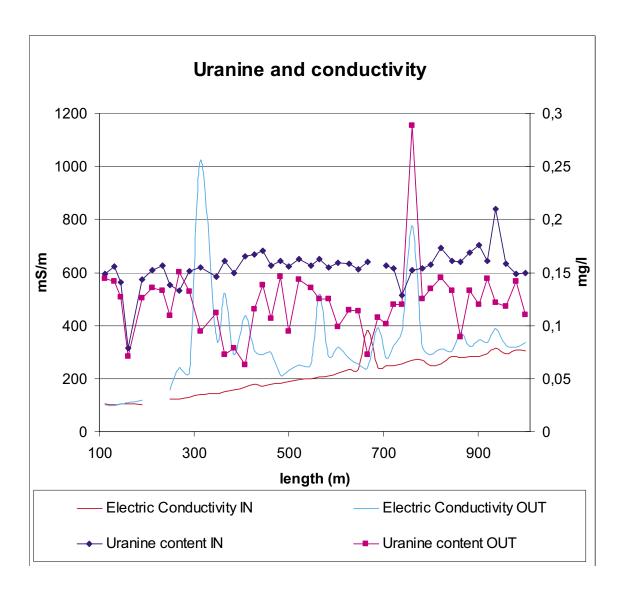


Figure 5-18. The uranine concentration and electric conductivity in KSH01A during drilling.

5.6.2 Quality of the analyses

The charge balance errors give an indication of the quality and uncertainty of the analyses of the major components. The relative charge balance errors are calculated for the selected sets of data from KSH01A, see Appendix 2. The errors do not exceed \pm 5% in any of the samples (3824, 3825 and 3831) which is fully satisfactory.

Figures 5-19 and 5-20 illustrate the consistency of the analyses. The figures are based on data compilation in Appendix 2. Electric conductivity values are plotted versus chloride concentrations in Figure 5-19.

The bromide and chloride concentrations are plotted in Figure 5-20. A plot of bromide versus chloride serves as a rough quality control of the bromide analyses.

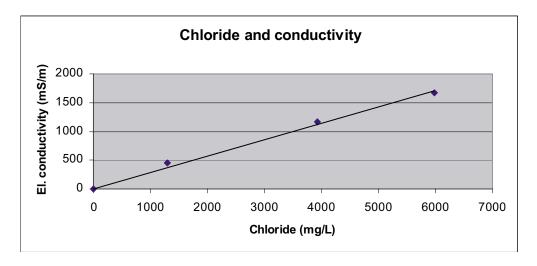


Figure 5-19. Plot of electric conductivity versus chloride concentration.

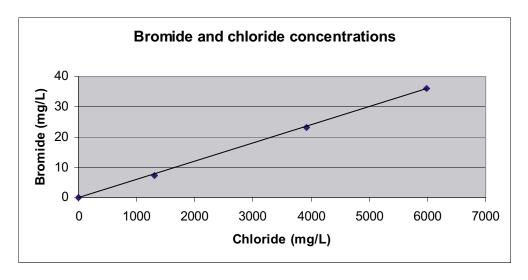


Figure 5-20. Plot of bromide concentrations versus chloride concentrations.

The following routines for quality control and data management are generally applied for hydrogeochemical analysis data, independent of sampling method or sampling object:

- Several components are determined by more than one method and/or laboratory. Moreover, control analyses by an independent laboratory are performed as a standard procedure on each fifth or tenth collected sample.
- All analytical results were stored in the SICADA database. The chemistry part of the database contains two types of tables, raw data tables and primary data tables (final data tables).
- Data on basic water analyses are inserted into raw data tables for further evaluation. The evaluation results in a final reduced data set for each sample. These data sets are compiled in a primary data table named "water composition". The evaluation is based on:
 - Comparison of the results from different laboratories and/or methods. The analyses are repeated if a large disparity is noted (generally more than 10%).
 - Calculation of charge balance errors. Relative errors within ± 5% are considered acceptable. For surface waters errors of ± 10%.
 - Rel. Error (%) = $100 \times (\sum \text{cations}(\text{equivalents}) \sum \text{anions}(\text{equivalents})$ ($\sum \text{cations}(\text{equivalents}) + \sum \text{anions}(\text{equivalents})$
 - General expert judgement of plausibility based on earlier results and experiences.

All results from "biochemical" components and special analyses of trace metals and isotopes are inserted directly into primary data tables. In those cases where the analyses are repeated or performed by more than one laboratory, a "best choice" notation will indicate those results which are considered most reliable. The analytical methods are indicated in Appendix 3.

5.7 Data handling

Data collected by the drilling contractor and the SKB drill coordinators were reported in daily logs and other protocols and delivered to the Activity Leader. The information was entered to SICADA (SKB database) by database operators. The field note numbers for entry into SICADA are:

- KSH01A Field note Simpevarp 10.
- KSH01B Field note Simpevarp 36.

5.8 Environmental control

The SKB routine for environmental control (SDP-301, SKB internal document) was followed throughout the activity. A checklist was filled in and signed by the Activity Leader and filed in the SKB archive.

All waste generated during the establishment, drilling and completion phases have been removed and disposed of properly. Water effluent from drilling was led to the Baltic Sea in accordance with an agreement with environmental authorities.

Recovered drill cuttings were collected in steel containers. After completion of drilling, the containers were removed from the site and emptied at an approved site.

5.8.1 Consumption of oil and chemicals

No significant amounts of oil or other lubricants were consumed during the drilling.

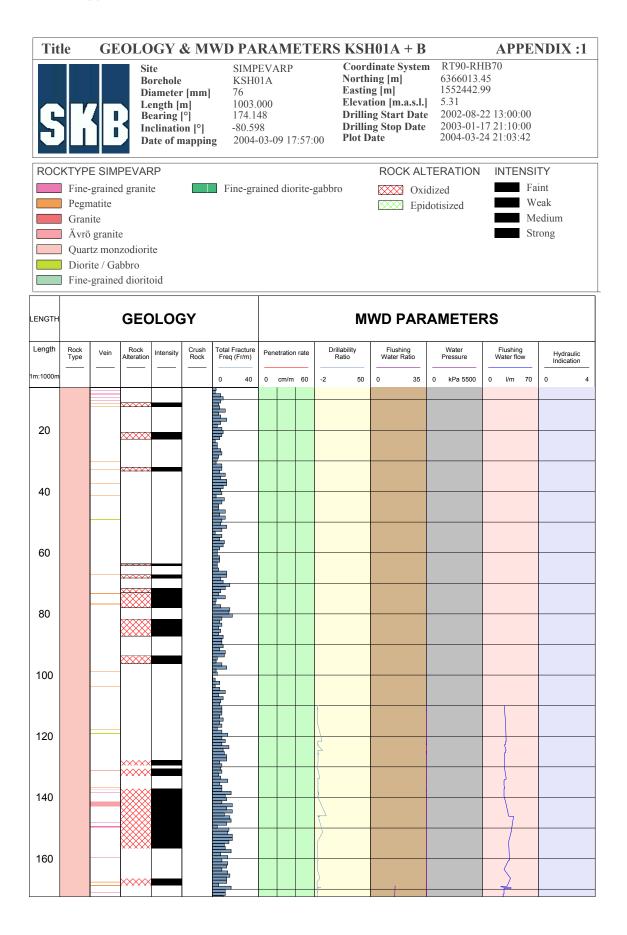
Gap filling from the surface to 12 m was done in two steps; first between the rock wall and the 273 mm casing and secondly between the 273 mm casing and the 208 mm casing. A total of 760 litres of cement paste, with a water/cement ratio of 0.6 was used for gap filling and 75 litres for the bottom plugs.

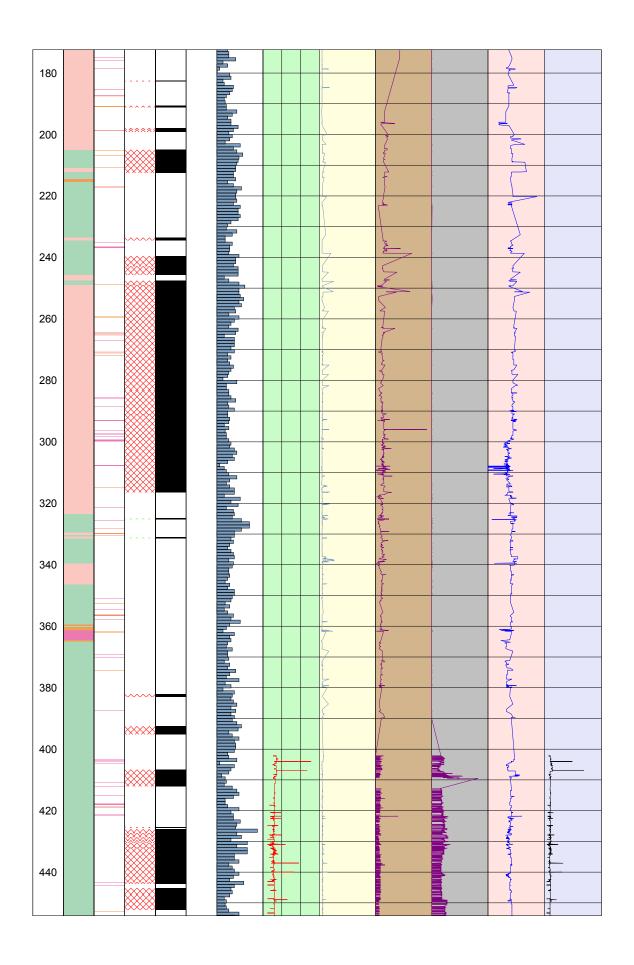
6 References

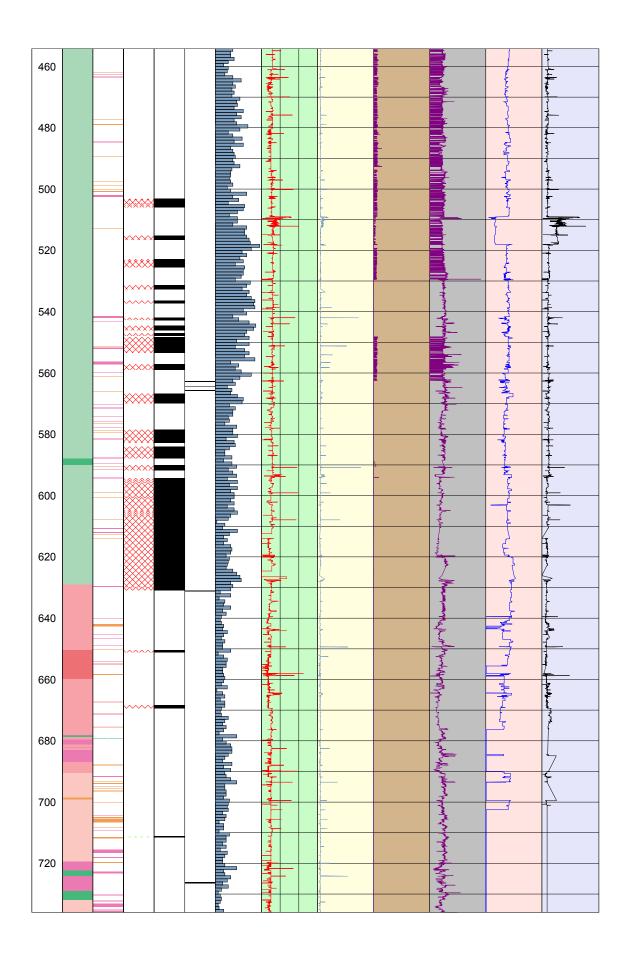
- /1/ **SKB, 2001.** Platsundersökningar. Undersökningsmetoder och generellt genomförandeprogram SKB R-01-10, Svensk Kärnbränslehantering AB.
- /2/ **SKB, 2001.** Geovetenskapligt program för platsundersökning vid Simpevarp. SKB R-01-44, Svensk Kärnbränslehantering AB.
- /3/ **SKB**, **2004.** Boremap mapping of telescopic drilled borehole KSH01 A and B. SKB P-04-01 (in prep), Svensk Kärnbränslehantering AB.
- /4/ Moye D G, 1967. Diamond drilling for foundation exploration. Civil Eng. Trans., Inst. Eng, Australia.

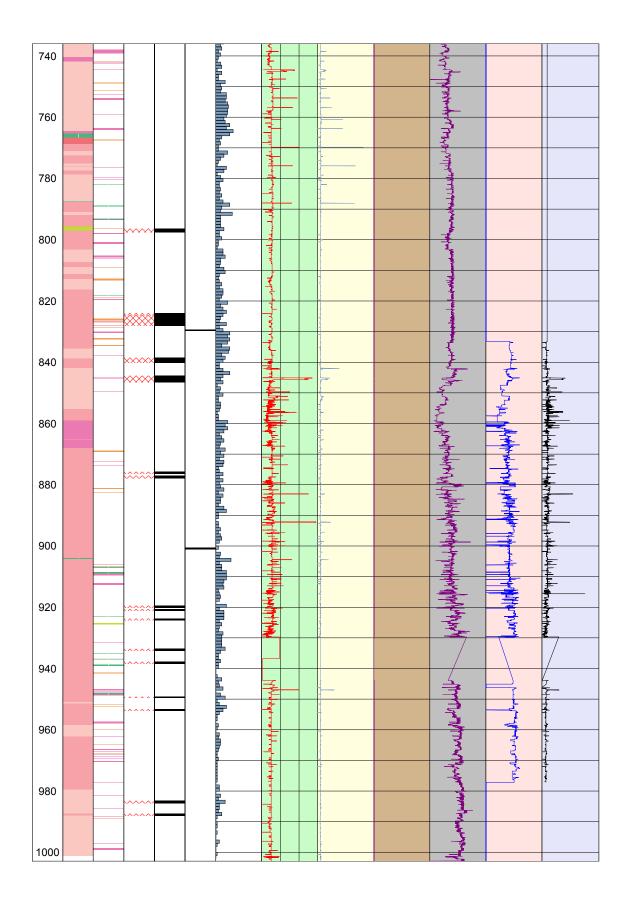
Appendix 1

Geology and MWD parameters KSH01A + KSH01B









Appendix 2

Chemical results

Borehole	Date of	Upper	Lower	Sample_no	Groundwater	Hd	Conductivity	N a	¥	Ca
	measurement	section limit	section limit		Chemistry		mS/m	mg/l	l/gm	l/gm
					Class					
KSH01A	2002-10-24 09:30	197,00	313,42	3824	က	7,11	1661	2440	13,1	1020
KSH01A	2002-11-16 21:40	585,00	593,00	3825	က	7,3	451	651	6,87	226
KSH01A	2002-11-20 08:30	531,00	619,42	3831	က	7,08	1160	1640	9,91	770
HSH03	2002-08-21 16:30	0,00	201,00	3759	က	8,21	76,8	152	3,31	13
HSH03	2002-08-22 18:00	0,00	103,00	3760	က	8,28	77,3	154	3,37	13,2
HSH03	2002-09-05 15:55	0,00	201,00	3761	က	8,12	111	192	3,56	21,1

PM C	% Modern	Carbon	36,35	59,81	57,08			
10 C	l/gm					4,5	4,4	2
ູ້ວ	mg/l		17,3	3,72	12,5	0,19	0,2	0,32
5	mg/l		0,335	0,089	0,241	0,016	0,016	
Z Z	mg/l		0,53	0,55	0,74	0,03	0,03	0,05
Ъ	l/gm	Total Iron	2,76	7,26	5,5	0,044	0,048	0,396
	mg/l		4,4	4,5	3,1	4,2	4,3	5,3
ш	l/gm		0,56	1,84	1,37	3,07	3,11	1,57
ā	mg/l		36,16	7,24	23,3	0,36	0,2	0,36
S04_S	mg/l	Total Sulphur	5,43	42,4	35,4	27,1	27,2	27,2
804	mg/l		18,5	167,34	96,21	84,29	84,95	
ច	mg/l		5982,3	1298,6	3922,9	53,1	55,1	131,8
HC03	l/gm	Alkalinity	21	137	25	255	248	233
Mg	mg/l		61,2	21,1	42,7	3,4	3,5	2

Sr-87	Sr-87/Sr86	0,72	0,72 0,72			
CI-37	dev SMOC			0,07	-0,07	0,07
S-34	dev SMOW	23,5	18,1			
B-10	dev SMOW B-10/B-11					
0-18	dev SMOW	1- 4- 1-	-10,6	-10,6	-10,7	-10,8
Ë	2	£ (o 4, 1 G	4,7	10	9,4
٥	dev SMOW	-102,5	-77,8 -86,1	-78	-76,1	9'92-
AGE_BP	Groundwater	age 8078	4077 4453			
C-13	dev PDB	4,02	-15,7			

Appendix 3

Chemistry - analytical method and preparation

Analysis	Sample bottle	Preparation	SKB label	Laboratory	
pH, conduktivity, alkalinity	250 ml	Filtering Pallfilter	green	Äspö/field	
Anions (F ⁻ , Br ⁻ , Cl ⁻ , SO ₄ ²⁻)	250 ml	Filtering Pallfilter	green	Äspö/field	
Uranine	100 ml brown glass		green	Äspö/field	
Main components (except Fe, Mn)	Analytica's 100 ml acid washed	1 ml HNO ₃ suprapur, filtering membrane filter	red	Analytica	
Archive samples	2 ea 250 ml	Filtering Pallfilter	green		
Option					
Deuterium, O-18	100 ml square		green	IFE	
Tritium	1000 ml dried	Flooded at least once	green	Waterloo	
Sr-87	100 ml square		green	IFE	
LE-12	Same as for Tritium		green	Waterloo	
B-10	Same as for main components	Filtering membrane filter	red	Analytica	
C-13, PMC	2*2 st 100 ml brown glass		green	Waterloo och Uppsala	
S-34	1000 ml		areen	IFE	

Deviation measurements

Origin	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured	Measured
alc Date	2003-06-16 08:37	003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	003-06-16 08:37	003-06-16 08:37	175,3 2003-06-16 08:37 Measured	2003-06-16 08:37 Measured	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37 Measured	003-06-16 08:37	2003-06-16 08:37 Measured	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	003-06-16 08:37	170,64 2003-06-16 08:37 Measured	003-06-16 08:37	003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37 Measured	2003-06-16 08:37 Measured	2003-06-16 08:37 Measurec	2003-06-16 08:37	2003-06-16 08:37	2003-06-16 08:37
aring (degrees)Ca	170,47	171,98 2	172,64 2	174,08 2	175,17 2	175,36 2	175,28 2	175,3 2	175,48 2	175,58 2	175,79 2	175,81 2	175,78 2	175,57 2	175,21 2	174,79 2	174,32 2	173,85 2	173,43 2	172,98 2	172,45 2	171,99 2	171,48 2	171,11 2	170,64 2	170,32	169,78 2	169,33 2	168,83 2	168,48 2	168,16 2	167,81 2	167,51 2	167,31 2	167,13 2	167,05 2	167,06 2	167,1 2
ination (degrees Be	-81,17	-80,85	-80,75	-80,47	6'08-	-80,26	6'08-	-80,28	-80,22	-80,15	-80,08	-79,94	8'62-	-79,65	-79,47	-79,32	-79,17	-79,02	-78,86	-78,71	-78,57	-78,39	-78,23	-78,05	-77,91	-77,73	9,77-	-77,43	-77,29	-77,1	-76,95	-76,76	9'92-	-76,43	-76,25	-76,1	-76,02	-76,03
Vertical Depth Incl	-2,96	0	2,97	5,93	8,89	11,84	14,8	17,76	20,71	23,67	26,63	29,58	32,53	35,49	38,44	41,39	44,34	47,28	50,23	53,17	56,11	20,65	61,99	64,93	98'29	20,8	73,73	99'92	79,59	82,51	85,44	88,36	91,28	94,2	97,12	100,03	102,94	105,85
Length (m) N	ဇု-	0	3	9	6	12	15	18	21	24	27	30	33	36	39	42	45	48	51	54	25	09	63	99	69	72	22	78	81	84	87	06	66	96	66	102	105	108
Coordinate System	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	38 6365992,45 1552446,05 -100,53 RT90-RHB70 108 105,85 -76,03 167,1 2003-06-16 08:37 M
Elevation (m)	8,28	5,32	2,35	-0,61	-3,57	-6,52	-9,48	-12,44	-15,39	-18,35	-21,31	-24,26	-27,21	-30,17	-33,12	-36,07	-39,02	-41,96	-44,91	-47,85	-50,79	-53,73	-56,67	-59,61	-62,54	-65,48	-68,41	-71,34	-74,27	-77,19	-80,12	-83,04	-85,96	-88,88	-91,8	-94,71	-97,62	-100,53
Easting (m) E	1552442,91	1552442,99	1552443,05	1552443,11	1552443,17	1552443,21	1552443,25	1552443,29	1552443,33	1552443,37	1552443,41	1552443,45	1552443,49	1552443,53	1552443,57	1552443,62	1552443,67	1552443,72	1552443,78	1552443,85	1552443,92	1552444	1552444,08	1552444,17	1552444,27	1552444,37	1552444,48	1552444,59	1552444,71	1552444,84	1552444,98	1552445,11	1552445,26	1552445,41	1552445,56	1552445,72	1552445,89	1552446,05
Point Seg Northing (m) E	1 6366013,92	2 6366013,47	3 6366012,99	1 6366012,52	5 6366012,02	3 6366011,52	7 6366011,01	3 6366010,51	9 6366010	0 6366009,49	1 6366008,98	2 6366008,47	3 6366007,94	1 6366007,41	5 6366006,88	3 6366006,33	7 6366005,78	3 6366005,21	9 6366004,65	0 6366004,07	1 6366003,49	2 6366002,9	3 6366002,3	4 6366001,7	5 6366001,08	3 6366000,46	7 6365999,83	3 6365999,2	9 6365998,56	0 6365997,91	1 6365997,25	2 6365996,59	3 6365995,92	1 6365995,24	5 6365994,55	3 6365993,86	7 6365993,16	3 6365992,45
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-225,87	-228,79	-231,7	-234,62	-237,53	-240,45	-243,37	-246,28	-249,2	-252,11	-255,03	-257,94	-260,86	-263,78	-266,69	-269,61	-272,52	-275,44	-278,35	-281,27	-284,18	-287,1	-290,01	-292,93	-295,84	-298,75	-301,67	-304,58	-307,49	-310,41	-313,32	-316,24	-319,15	-322,06	-324,98	-327,89	-330,81	-333,72	-336,64	-339,55	-342,46	-345,37
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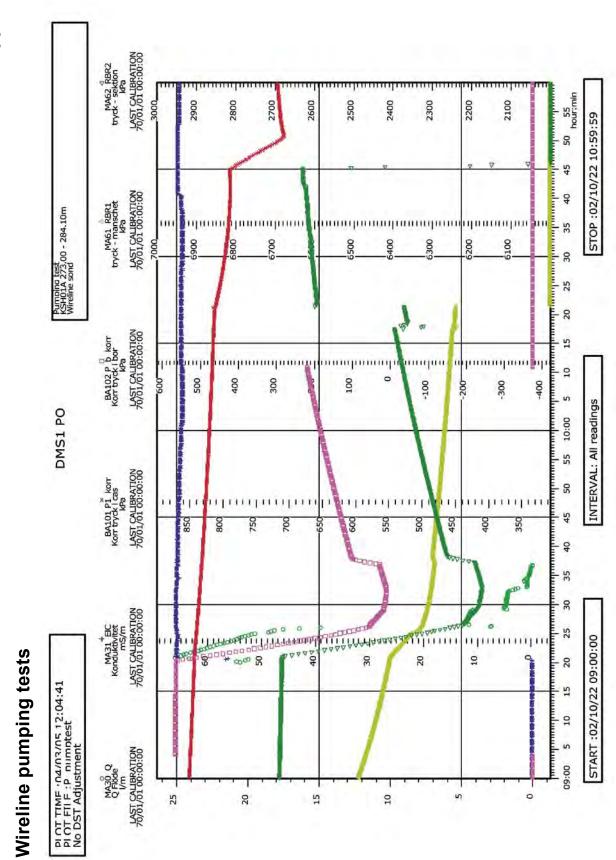
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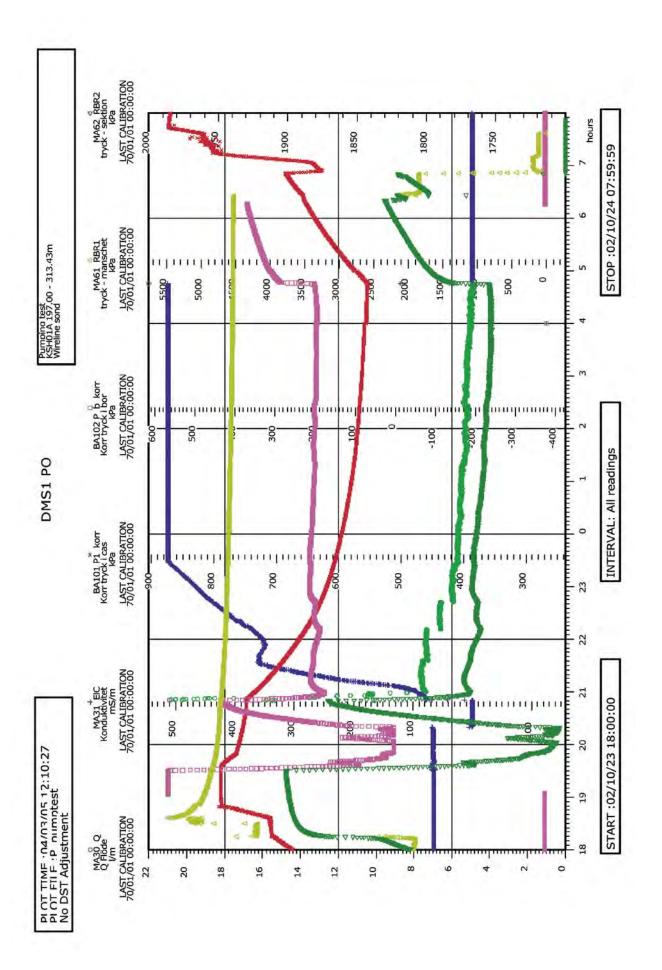
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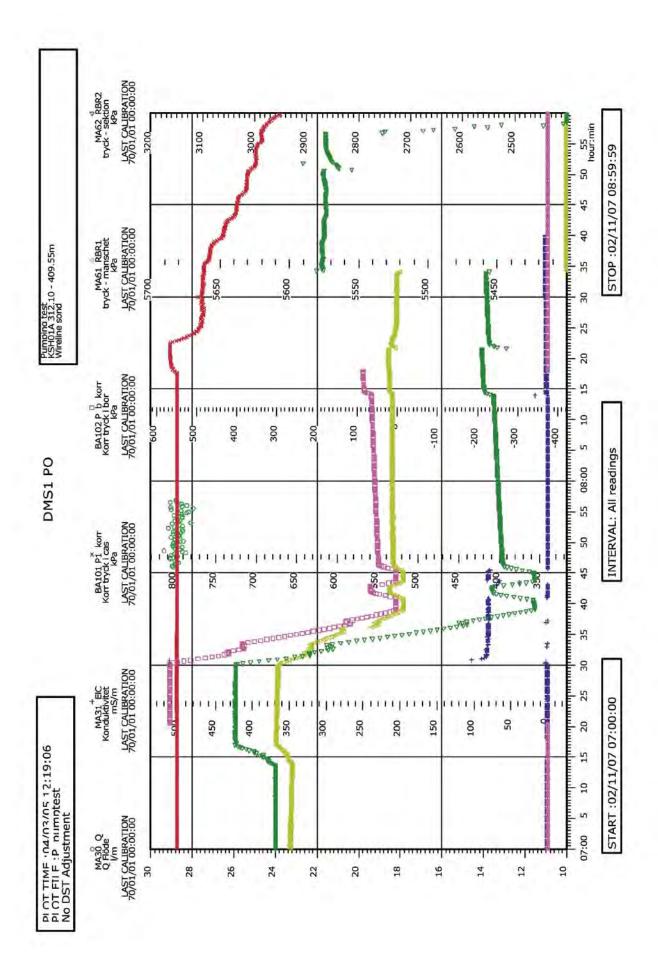
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RT90-RHB70	5 RT90-RHB70	S RT90-RHB70	RT90-RHB70	7 RT90-RHB70	3 RT90-RHB70	RT90-RHB70	RT90-RHB70	3 RT90-RHB70	RT90-RHB70	2 RT90-RHB70	3 RT90-RHB70	3 RT90-RHB70	RT90-RHB70	5 RT90-RHB70	RT90-RHB70	S RT90-RHB70	7 RT90-RHB70	3 RT90-RHB70	3 RT90-RHB70	RT90-RHB70	-653,59 RT90-RHB70	5 RT90-RHB70	RT90-RHB70	RT90-RHB70	2 RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	3 RT90-RHB70	RT90-RHB70	RT90-RHB70	5 RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	RT90-RHB70	7 RT90-RHB70	RT90-RHB70	-711,55 RT90-RHB70
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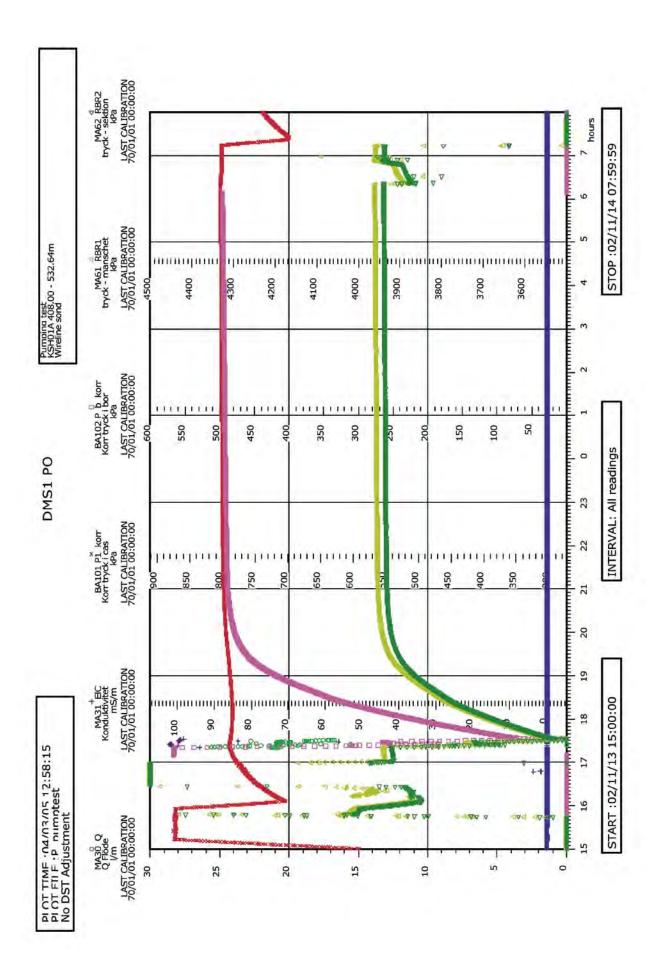
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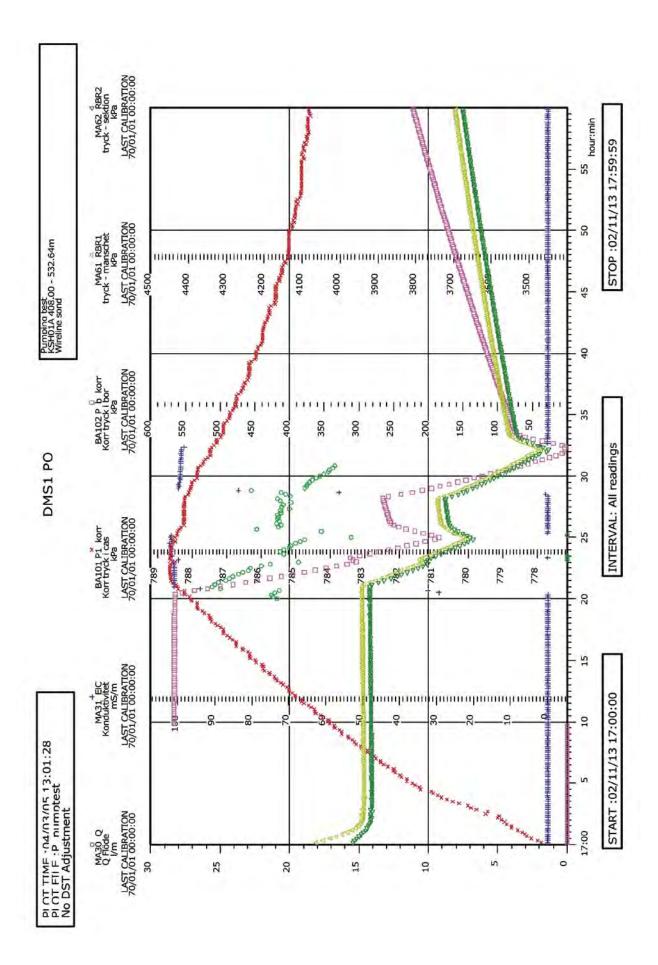
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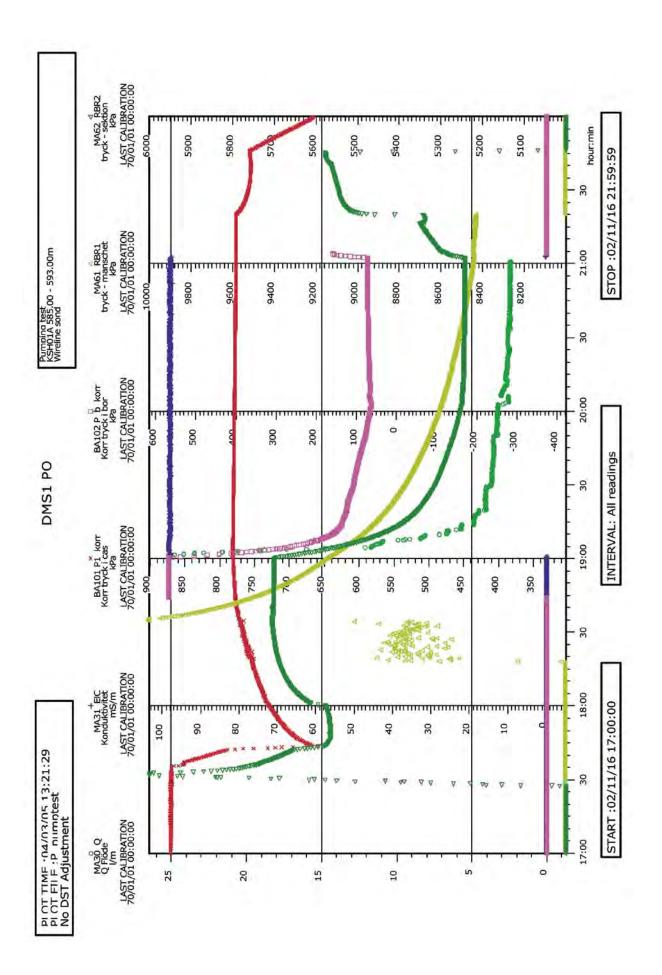


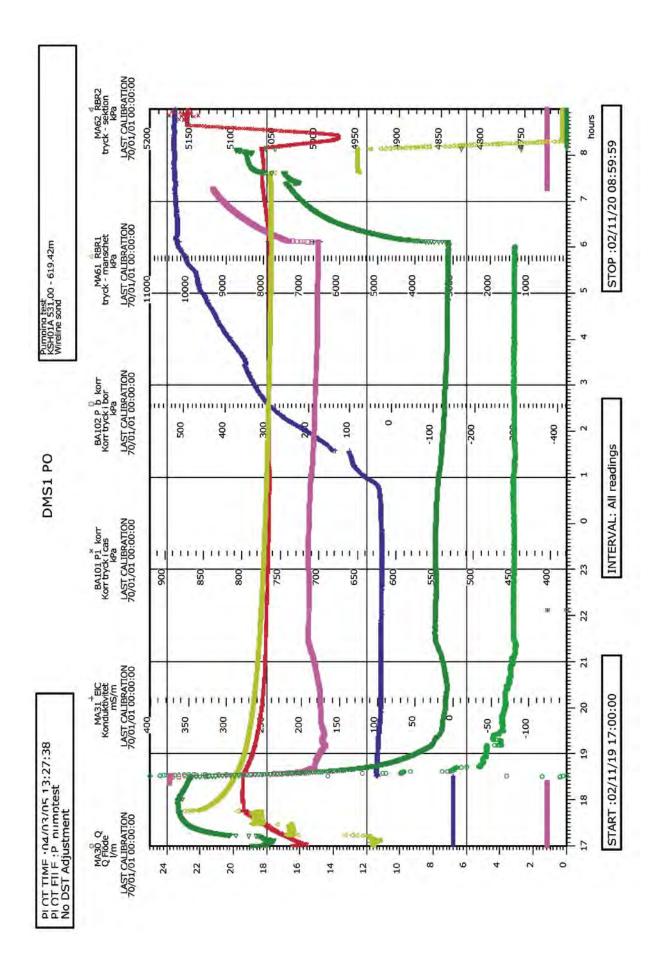


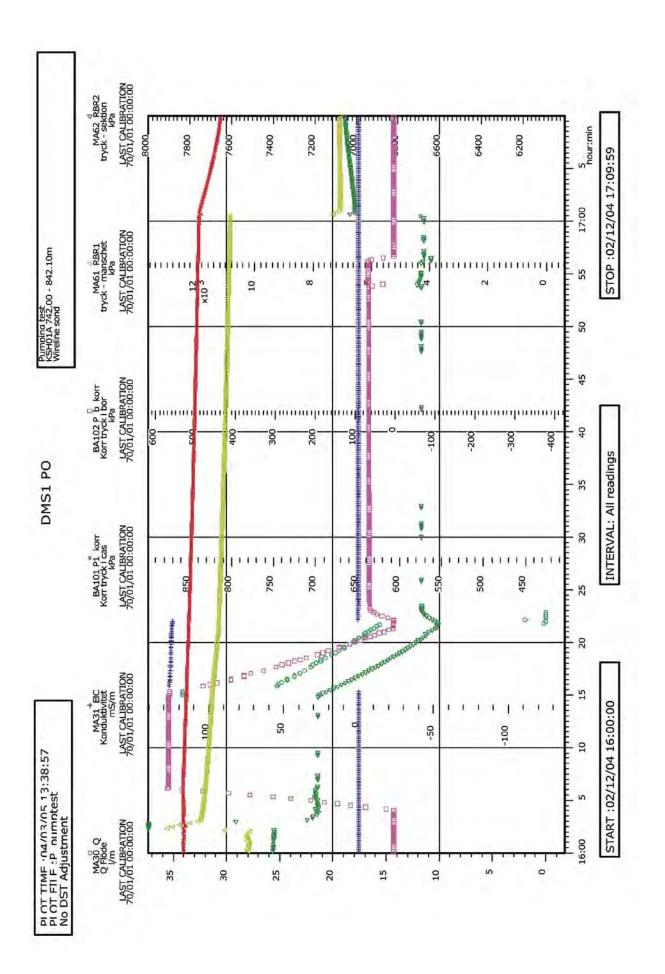


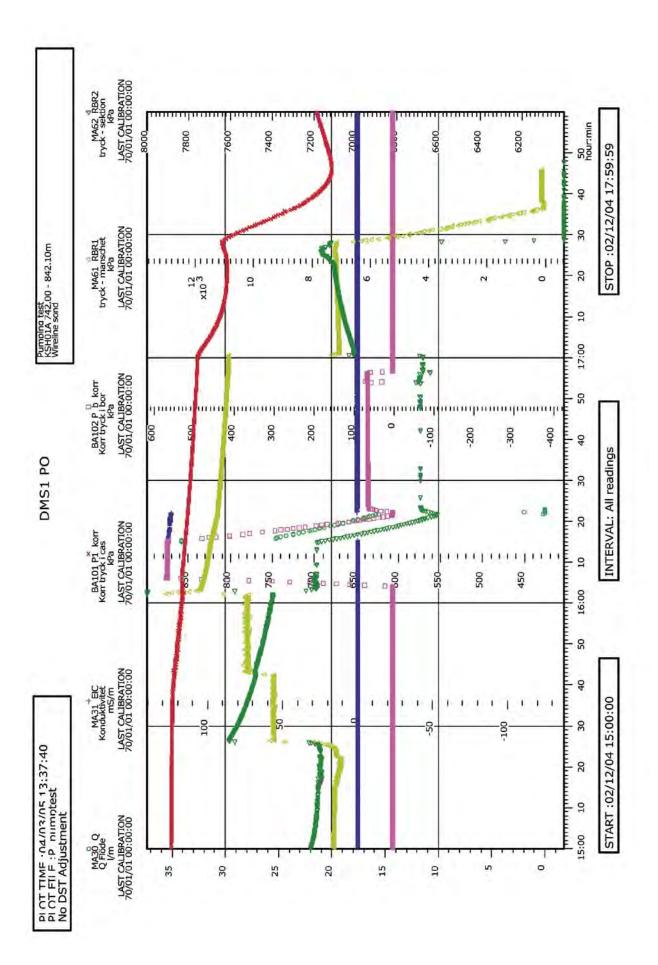


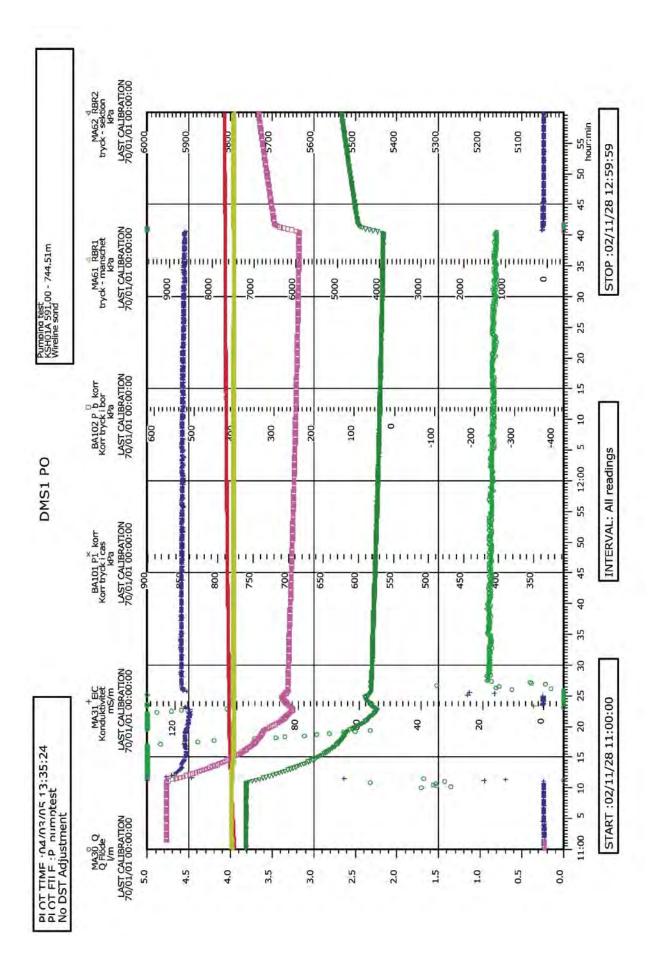


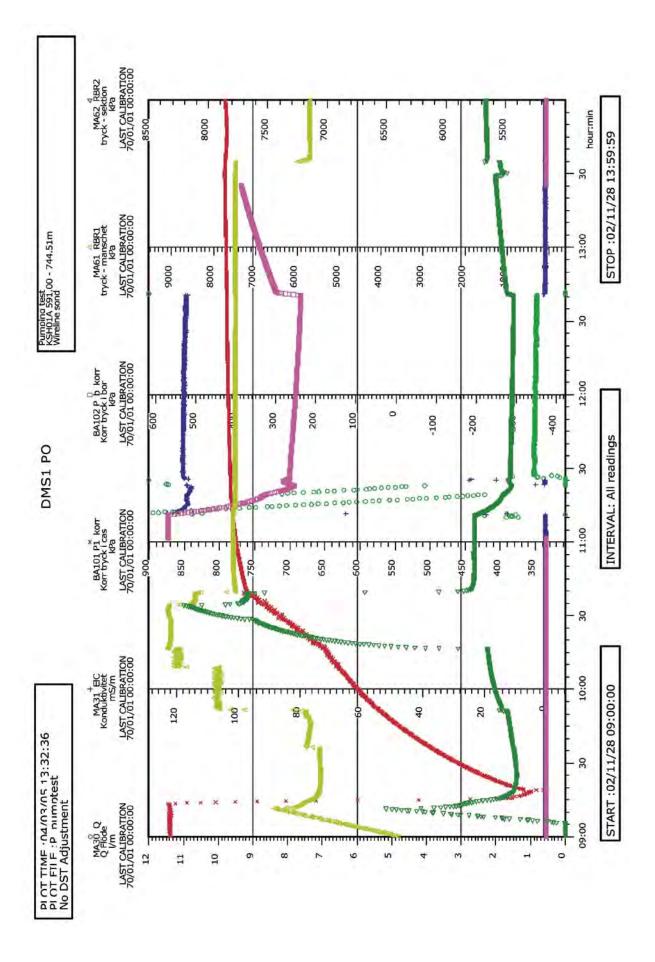


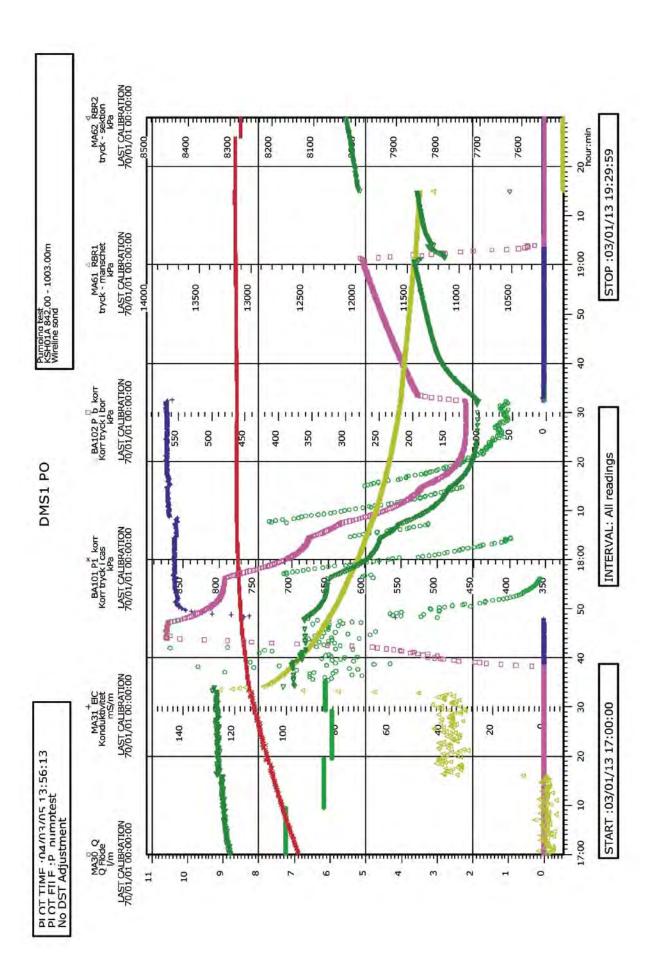












Appendix 6

MA62 RBR2 tryck - sektion kPa 16 hours 1019 1021 1023 STOP:02/10/15 16:59:59 Absolute pressure measurement KSH01A, 102-197m 000 % 12 OD 876 Time series of absolute pressure measurements in borehole KSH01A 10 MA61 RBR1 tryck - manschet kPa ×10 36 N -10 φ 8 INTERVAL: All readings DMS1 PO D BA102 P b korr Korr tryck i bor KPa ×10 -3 22 Control of the contro START:02/10/14 15:00:00 20 PLOT TIME :03/10/09 14:31:58 PLOT FILE :P_tryck_i _sektion 18 16 BA101 P1 korr Korr tryck i cas KPa 870 830 820 810 290 260 840 860 850 800 780 770

