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# Äspö Hard Rock Laboratory

Rock stress measurements  
in Oskarshamn

Hydraulic fracturing and core testing  
in borehole KOV01

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January 2002

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Äspö Hard Rock  
Laboratory



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*Keywords:* Rock stress measurement, hydraulic fracturing, density, ultrasonic velocity, fracture toughness, hydrofrac test on minicores, numerical simulation

This report concerns a study which was conducted for SKB. The conclusions and viewpoints presented in the report are those of the author(s) and do not necessarily coincide with those of the client.



# Summary

During 1. - 8.10.01 a total of 19 hydraulic fracturing tests / hydraulic stimulation tests on pre-existing fractures were carried out in borehole KOV01 using the MeSy wireline technique where both the double straddle -packer sonde and the impression packer unit for fracture orientation determination were moved within the borehole via a 7-conductor logging cable winch. The 19 tests covered the borehole interval from a depth of 280 m to app. 700 m.

For the generation of new fractures injection pressures up to app. 25 MPa were required for breakdown, while induced or pre-existing fractures could more easily be opened. Comparison of breakdown and refrac pressures yield a (hydraulic) tensile strength of the granitic rock of app. 5 to 6 MPa. Most tests showed rather distinct shut-in pressure values which generally increased with depth. From induced fractures only a small fraction of the injected water was recovered, while stimulated pre-existing fractures showed a significant water recovery rate. Most of the induced or stimulated fractures were axial or steeply inclined with a mean strike direction of NW - SE ( $145^\circ \pm 18^\circ$ ). This consistent fracture orientation did not allow to derive a reliable stress profile by an inversion calculation. Therefore, the principal horizontal stresses had to be calculated for each test at each of the 19 test sections. The derived stresses increase with depth but are significantly scattered and do not suggest a linear stress profile. In particular, rather high horizontal stresses can be observed at a depth of app. 600 to 700 m ( $S_h = 14$  to 18 MPa,  $S_H = 30$  to 45 MPa). The observed hydrofrac records could be simulated using a fracture mechanics approach and relevant physical rock property data derived from laboratory tests on the core material (fracture toughness tests, laboratory hydrofrac tests). This analysis also indicated the existence of microfractures of a characteristic dimension of app. 30 mm in the intact granitic rock.

The derived stress data in borehole KOV01 are in good agreement with the stress data obtained in the Laxemar borehole KLX02. If one adds the stress information from the two boreholes an almost gapless data profile exists which could be representative for the stress regime in the Oskarshamn - Laxemar - Äspö region.

Each hydrofrac test cycle in borehole KOV01 was preceded by a pressure pulse test where the pressure decay was observed. This test was mainly conducted to probe the suitability of a test section for hydrofracturing, but it also allowed to derive information on the hydraulic rock properties at each section. This was mainly possible due to the stiff hydrofrac test system. The tests yield a hydraulic rock mass conductivity of  $(0.3$  to  $46) \cdot 10^{-11}$  m/s depending whether the test section penetrated solid rock or contained pre-existing fractures.



# Sammanfattning

Den 1 till 8 oktober 2001 genomfördes totalt 19 stycken hydrauliska spräckningar och injektionsförsök i borrhål KOV01. För testerna användes MeSy:s wireline system där både dubbelmanschetten och avtrycksmanschetten transporterades i borrhålet med en kabelvinsch där kabeln innehåller 7 ledare för avläsning av instrument i manschetterna. De 19 testerna genomfördes i ett djupintervall från 280 m till 700 m.

I de fall hydrauliska spräckningar genomfördes krävdes spräcktryck på cirka 25 MPa medan befintliga sprickor kunde öppnas vid lägre tryck. En jämförelse av spräcktrycken och sprickornas återöppningstryck visar att den hydrauliska draghållfastheten i berget är cirka 5 till 6 MPa. Huvuddelen av testerna hade relativt distinkta återförslutningstryck vilka generellt ökade med djupet. Från de sprickor som skapades genom spräckning försvann huvuddelen av det injekterade vattnet i berget medan stimulerade befintliga sprickor gav tillbaka det mesta av det injekterade vattnet. Huvuddelen av både de skapade och stimulerade sprickorna var parallella med borrhålet eller brantstående med en medelstrykning på NV – SO ( $145^\circ \pm 18^\circ$ ). Eftersom de uppmätta sprickorienteringarna var så likartade blev det nödvändigt att beräkna den största horisontella huvudspänningen vid vart och ett av de 19 försöks sektionerna. De beräknade bergspänningarna ökar med djupet men har en så stor spridning att det inte är troligt att spänningsfördelningen är linjär mot djupet. Särskilt höga horisontalspänningar observerades från 600 till 700 m djup ( $S_h = 14$  till  $18$  MPa och  $S_H = 30$  till  $45$  MPa). Responserna från de hydrauliska testerna kan simuleras genom att använda en sprick mekanisk ansats tillsammans med data om bergets fysiska egenskaper erhållna från laborietester av borrhärnor. Analyserna som bland annat bestod av ”fracture toughness” test och hydrauliska tester på borrhärnorna påvisar en förekomst av mikrosprickor till ett avstånd på ca. 30 mm från borrhållsväggen.

De erhållna bergspänningarna i borrhålet KOV01 stämmer väl överens med resultat från tidigare bergspänningsmätningar genomförda i borrhålet KLX02 i Laxemar. Om resultaten från de två borrhålen superponeras erhålles en spänningsprofil med god överensstämmelse som troligen är representativ för spänningsfältet i Oskarshamn – Laxemar – Äspöregionen.

Varje hydraulisk spräckning genomförd i KOV01 föregicks av ett tryckpuls test där trycksänkningen övervakades. Testet genomförs i huvudsak för att se om vald sektion är lämplig att genomföra ett hydrauliskt spräckförsök i. Testet ger också viss information om bergets hydrauliska egenskaper i sektionen eftersom ett styvt hydrauliskt system används. Den hydrauliska konduktiviteten i de olika sektionerna ligger enligt denna metod mellan  $0,3$  till  $46 \cdot 10^{-11}$  m/s beroende på om testsektionen var belägen i homogent berg eller om den innehöll befintliga sprickor.





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## APPENDIX

- A: Records from in-situ hydrofrac - tests together with evaluation of characteristic pressure data
- B: Fracture traces on the impression packer sleeve
- C: Core piece data sheets
- D: Rock density test data
- E: Ultrasonic velocity data sheets
- F: Elasticity data sheets
- G: Fracture toughness data sheets
- H: Laboratory hydrofrac data sheets
- I: Pressure pulse test records and permeability analysis



## 1 Introduction

For the design and construction of underground repositories for nuclear waste the knowledge of the in-situ stress regime is of outmost importance. At the potential Swedish first nuclear waste storage site at Äspö, therefore, numerous in-situ stress measurements have been conducted since 1989, both in the Äspö Hard Rock Laboratory (HRL) and in deep boreholes drilled in the Oskarshamn - Laxemar - Äspö granite complex. The existing stress data were presented e.g. by Lundholm (1999) and Ekman et al. (1999), and were summarized e.g. by Ljunggreen and Klasson (1997), Ask et al. (2001), or by Ekman (2001). The existing stress data mainly originate from both, overcoring tests with Doorstopper and Triaxial cells and hydraulic fracturing tests.

The 76 mm diameter borehole KOV01 located in the harbour of Oskarshamn was drilled in 2000 (completed in February 2001) to app. 1000 m depth to generally evaluate methods for testing of crystalline rock and, in particular, to complement the existing stress data base in the area by additional hydraulic fracturing / hydraulic stimulation tests on pre-existing fracture / weakness planes, and to compare stress data derived by different methods and by different research teams. It was originally planned to conduct the tests in the depth interval between 280 m and 700 m concentrating especially on the intervals between 300 and 400 m, 500 and 550 m, and 600 and 700 m depth. However, since MeSy proposed to carry out the test program with its wireline testing technique where both, the fracturing straddle packer tool and the impression packer unit for fracture orientation can quickly be moved within the borehole via a wireline winch on a 7-conductor logging cable, it seemed appropriate to randomly select suitable test sections at variable depths in order to derive a representative stress - depth profile.

In addition MeSy proposed to carry out a laboratory test program on core samples from the borehole to determine hydrofrac relevant fracture mechanics rock properties. Using these parameters in a MeSy fracture simulation program allows the interpretation of in-situ pressure records and thus yields a quality control for derived in-situ stress data.



## 2 Borehole Data

Borehole KOV01 is located in the harbour of Oskarshamn inside an instrument store building belonging to SKB. Drilling was completed in February 2001. After some initial logging tests the hole was available for testing from 17.9.01 onwards. The core material was displaced for inspection in the instrument store building. The borehole location is shown in Figures 2.1 and 2.2, the available information on the borehole is summarized in Table 2.1.

**Table 2-1. Technical data of borehole KOV01.**

location	Oskarshamn, Smaland, Sweden
borehole	KOV01
geogr. coordinates	N 57°15'39", E 16°28'01" (MeSy GPS)
nat coordinates (RT 38 2.5 W system)	N 6348516 m, E 1539942 m
altitude (RH 70 system)	3.05 m
borehole length / depth	1000.97 m / 969.0 m
inclination / bearing	app. 70° / 205° at 290 m app. 82° / 206° at 750 m
borehole diameters	3 inches / 76 mm
casing depth	to app. 100 m
casing diameter	200 mm ID
borehole fluid	water
fluid level	at surface
cored section	101.07 to 1000.97 m
core diameter	47 mm
drilling contractor	Drillcon AB
completed logs	camera log



Figure 2-1. Borehole location Oskarshamn.



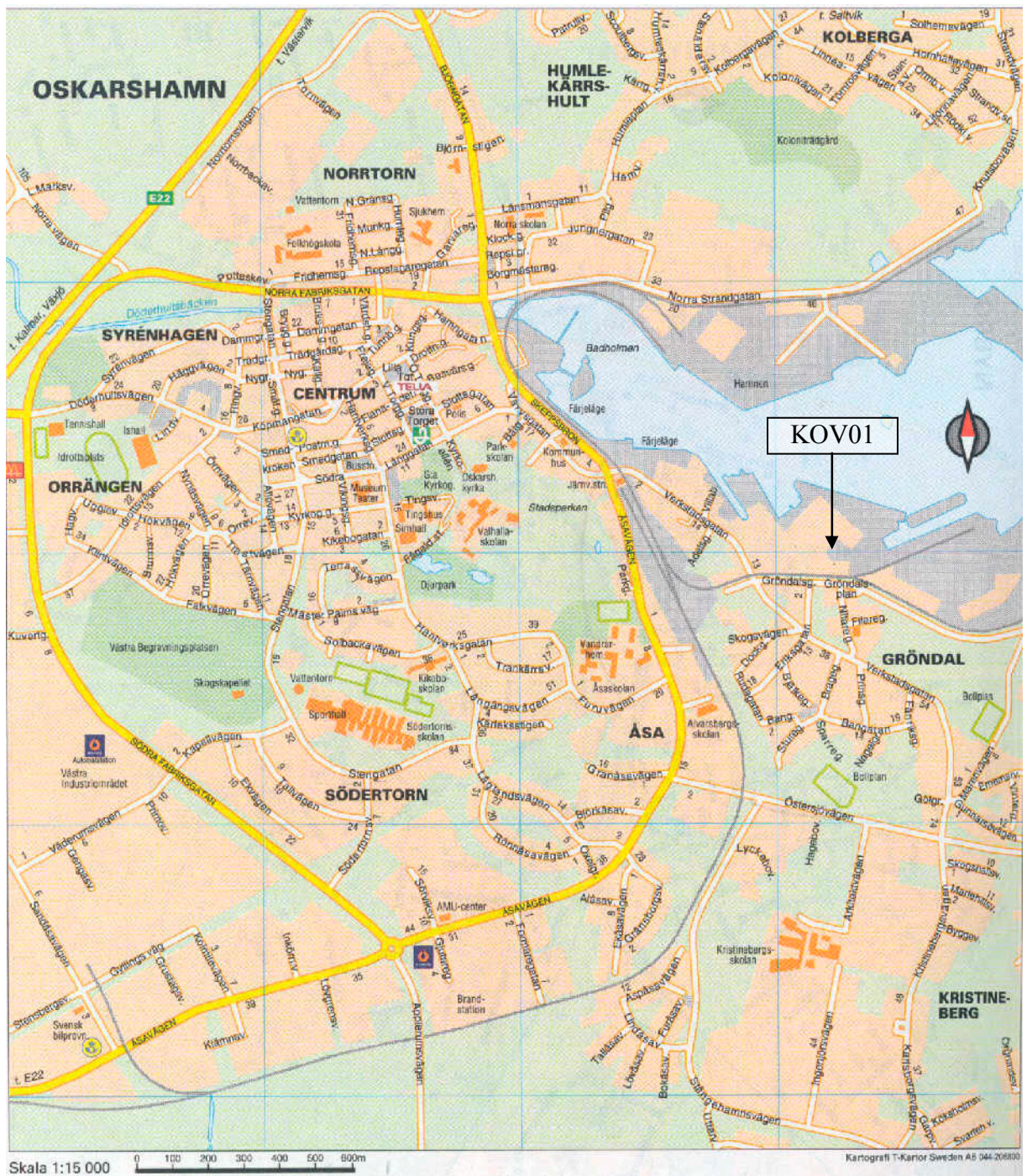


Figure 2-2. Borehole location at Oskarshamn.





## 3 Testing Procedures

### 3.1 Hydraulic Fracturing Principle

The term hydraulic fracturing is used for fluid injection operations in sealed-off borehole intervals to induce and propagate tensile fractures in the borehole wall-rock. It was first applied in oil industry to stimulate productivity from low permeable oil-bearing formations (Clark 1949). It was first proposed by Scheidegger (1962) to derive the state of stress from such hydrofrac operations. The classical concept for the interpretation of hydrofrac pressure records was developed by Hubbert & Willis (1957). The concept is based on the Kirsch (1898) equation for the tangential stress  $\sigma_{\theta}$  around a pressurized borehole oriented perpendicular to the far-field principal stresses  $S_1$  and  $S_3$ . For compressive far-field stresses the maximum tensile stress at the borehole wall is given by

$$\sigma_{\theta, \max} = -S_1 + 3S_3 - P \quad (1)$$

when the borehole is pressurized by a fluid pressure  $P$  in the borehole.

If the pressure  $P$  reaches a critical value,  $P_c$ , a radial tensile fracture will initiate:

$$P_c = 3 S_3 - S_1 + P_{co} \quad (2)$$

where  $P_{co}$  is the (hydraulic) tensile strength of the rock.  $P_c$  generally is called breakdown pressure.

After fracture initiation the pressure fluid will penetrate into the fracture and will force the fracture to propagate. During this phase the pumping pressure  $P_p$  during constant rate injection generally decreases (if the supply rate is less than the propagating fracture can consume). For isotropic rock we may also assume that the fracture will propagate the easiest path, i.e. perpendicular to the direction of the least principal far-field stress  $S_3$ .

In case the injection suddenly is interrupted (pumping system shut-in) the dynamic fracture propagation comes to a halt and the injection pressure in the pressurized borehole interval rapidly decreases. During this stage we may assume that a static equilibrium is reached, where the pressure in the pressurized borehole interval is equal to the fluid pressure within the fracture, which is determined by the minimum principal stress  $S_3$  :

$$P_{si} = S_3 \quad (3)$$

where  $P_{si}$  is generally called shut-in pressure.

After venting the test interval the fracture is drained and therefore will close. By subsequent injection the pressure must exceed the tangential stress at the borehole wall to reopen the induced fracture :

$$P_r = 3S_3 - S_1 \quad (4)$$

Therefore, we denote this pressure as reopening (or refrac) pressure  $P_r$ , and the comparison of eq. (2) and (4) provides a simple relation for the rock tensile strength  $P_{co}$ :

$$P_{co} = P_c - P_r. \quad (5)$$

Equations (2) to (4) commonly are known as Hubbert & Willis equations of hydraulic fracturing which are being used to derive the principal stresses  $S_1$  and  $S_3$  from the characteristic pressure values  $P_r$  and  $P_{si}$ .

As one can imagine from the above formulations, the H & W concept is based on several severe assumptions, such as rock isotropy and intact (unfractured) rock. In particular, hydrofrac experiments in a rock mass have to take into account the existence of pre-existing fractures which are not aligned with respect to the orientation of the three principal stresses  $S_1$ ,  $S_2$  and  $S_3$ . Several approaches to attack this problem have been suggested in the past which mostly are focussed to pressurize and statically open pre-existing fractures intersecting the test interval at some angle by low-rate injection, and thus measure the normal stress acting across the fracture. If a sufficient number of pre-existing fractures with different orientations can be opened in different test sections, then this approach also allows an exact determination of the three principal stresses using some kind of inversion calculations.

Detailed information on the H & W stress analysis and the stress evaluation from pressurizing pre-existing fractures (PSI - method, HTPF - method) can be found in Amadei & Stephansson (1997), Cornet (1993), Rummel (1987) or Rummel (2001).

Finally, some remarks on the fracture mechanics approach to hydraulic fracturing for stress determination. Rocks per se contain fractures of different scales, i.e. microfractures which must not be initiated but propagate whenever the stress intensity at the crack tip reaches the fracture toughness of the rock. For tensile crack propagation the intensity of the stress field around the crack tip can easily be formulated in spite of the rather complex loading situation during hydraulic fracturing, using the principle of superposition :

$$K_I = K_I(S_1) + K_I(S_3) + K_I(P) + K_I(P_a) \quad (6)$$

where  $K_I$  denotes the stress intensity factors for  $S_1$ ,  $S_3$ ,  $P$  is the fluid pressure in the borehole, and  $P_a$  is the fluid pressure within the fracture of length  $a$ . The instability condition is given by

$$K_I \geq K_{IC} \quad (7)$$

where  $K_{IC}$  is the fracture toughness, a material property. The theory results in the following hydrofracturing relation :

$$P_c = \frac{K_{IC}}{(h_o + h_a)\sqrt{R}} + k_1 S_1 + k_2 S_3 \quad (8)$$

where  $h_o$ ,  $h_a$ ,  $k_1$  and  $k_2$  are normalized (known) stress intensity functions. For  $S_1 = S_3 = 0$  eq. (8) yields a physical relation for the (hydraulic) tensile strength  $P_{co}$  (see eq. 5) :

$$P_{co} = \frac{K_{IC}}{(h_o + h_a)\sqrt{R}}.$$

For further information see Rummel (1987) or Rummel (2001).

## 3.2 In - Situ Test Equipment

The hydraulic / hydrofrac tests in the 76 mm diameter borehole KOV01 were carried out using the wireline technique where the packer sondes are moved within the borehole on a 7-conductor logging cable with the MeSy cable winch MKW 1500 - 20 kN. The set-up of the cable winch on top of the borehole within the SKB store building is shown in Figure 3.1. The following downhole and uphole equipment components were used (Figure 3.2) :

### Downhole :

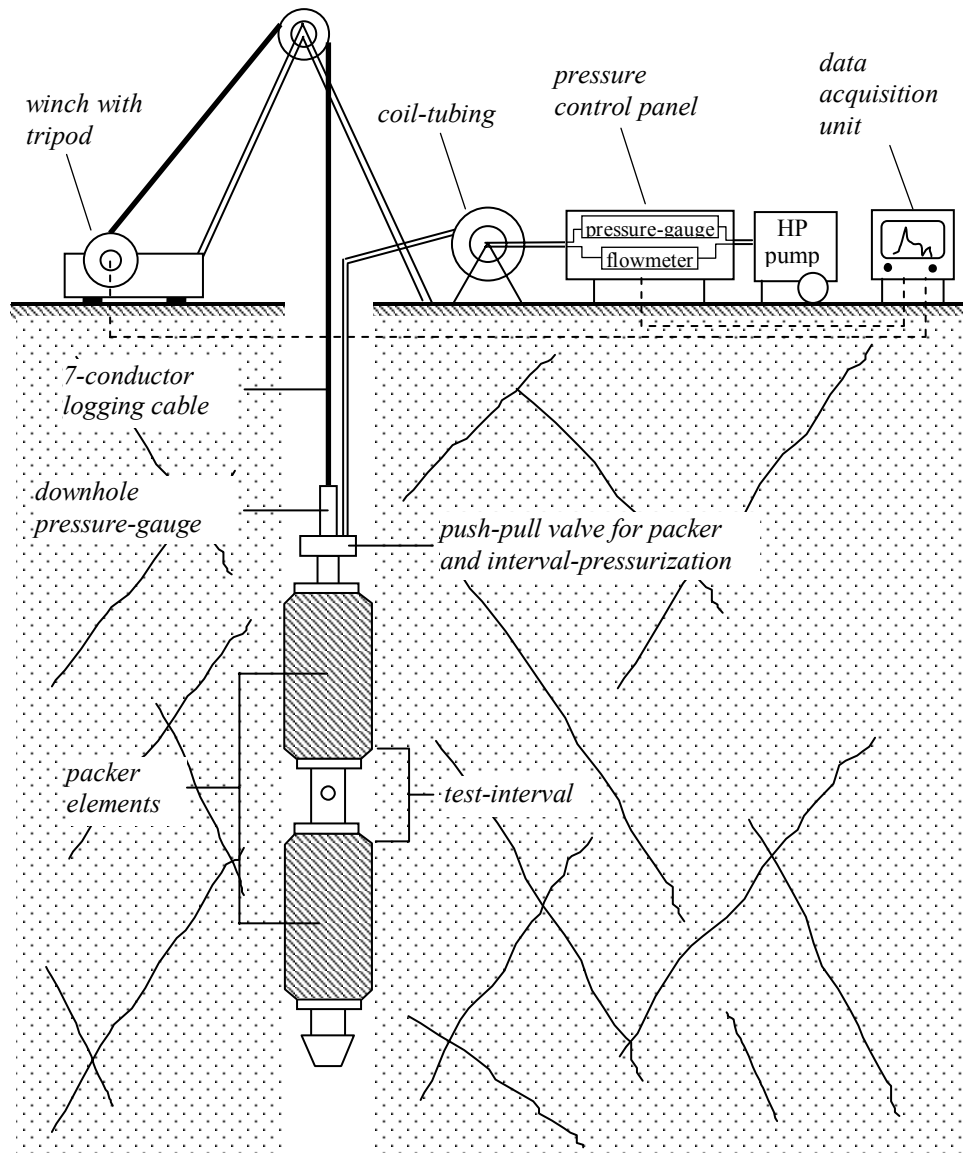
- double straddle packer sonde PERFRAC II (Figure 3.3) equipped with nylon-reinforced packer elements of S&K-type TK 48-V (length 1300 mm, sealing length app. 1000 mm, OD 70 mm) and with an injection interval between the packers of app. 700 mm,
- impression packer sonde PERFRAC II equipped with one S&K-type TK 48-V packer element wrapped with a soft rubber skin, and an adapter to a magnetic single-shot orientation device,
- single-shot unit EW type RG with a magnetic compass of range 0 - 20° / 0 - 90° and a clock range up to 990 minutes,
- 7-conductor logging cable of Rochester type 7-H - 314 A with GO cable head OD 8.2 mm,
- pressure transducer Keller type PA-23 (0-40 MPa) within the cable-head connector on top of the hydrofrac sonde,
- stainless steel coil tubing (OD 10 mm, ID 8 mm) as hydraulic line from surface to the packer sondes.

### Uphole :

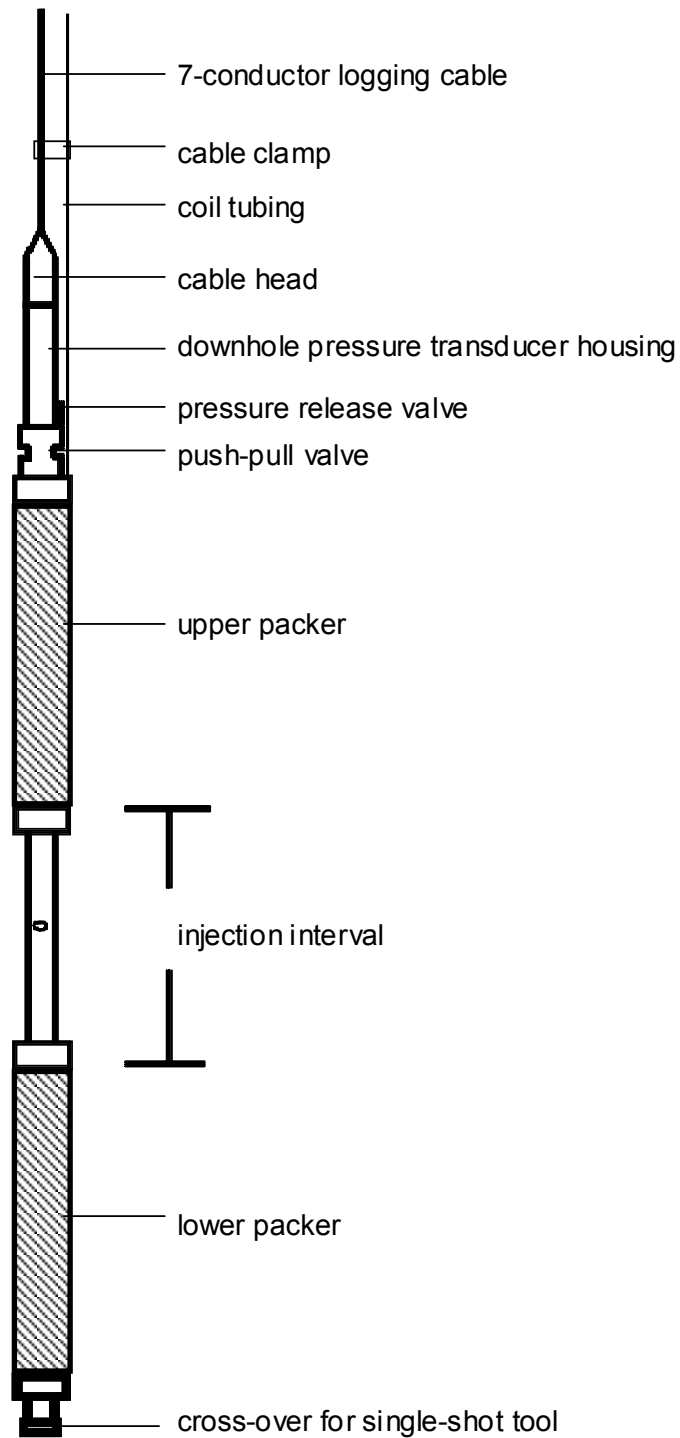
- electric -driven high pressure pump type Speck HP 400 / 2 - 12 lpm for fluid injection,
- cable winch MKW 1500 - 20 kN
- pressure control unit with 2 pressure transducers type Keller PA-23, 0 - 60 MPa and a flow meter UNIMESS OPT04, 0 - 10 lpm,
- digital data acquisition unit SILVI, 8 channels, 16 bit, 5 Hz,
- analog paper strip chart recorder BBC SE - 400, 4 channels



*Figure 3-1. Set-up of the MeSy winch MKW 1500 - 20 kN on the top of borehole KOV01 in the SKB store building at Oskarshamn*



**Figure 3-2.** Schematic view of the wireline hydrofrac system.



*Figure 3-3. Downhole system components.*

### 3.3 Test Section Selection, In - Situ Test Conduction

Prior to hydrofrac testing suitable test sections were selected on the basis of an overview core inspection. Initially, 15 test sections were selected for the borehole intervals between 300 - 400 m, 500 - 550 m, and 600 - 700 m, where the cores did not show any visible fractures. As it turned out during testing that in some of the selected

test sections no typical breakdown event was observed (an indication that weakness planes in the rock were stimulated), it was decided to randomly select further test sections at such depths to equally distribute tests over the total borehole interval of interest (280 to 710 m). Of course, the core sections of each additionally selected test section were inspected prior to each test, but it was recognized that the depth correlation between core depth and wireline depth may be erroneous by 1 to 2 m (an exact depth calibration of the wireline system within the borehole was not possible). By this strategy it was possible to meet the intended scope of the work to both, test intact borehole sections and stimulate borehole sections with pre-existing weakness planes / fractures / joints.

Prior to testing we also determined the hydraulic system stiffness by pressurizing the straddle packer sonde in a test pipe of 76 mm ID. Injection into the test interval (via the hydraulic stainless steel coil tubing used for downhole testing) yielded a system stiffness of 40 - 42 MPa / liter. Then, in order to check the borehole caliper condition a steel dummy with 70 mm OD and app. 100 kg weight (equivalent to the weight of the packer tool) was run to a depth of 715 m (MD).

During 3.10. to 8.10.01 a total of 20 injection tests were conducted at depth sections from 296 m to 713 m MD (measured depth), 19 of them showed typical breakdown or stimulation events, one test at 417.7 m MD / 395.5 m TVD was abandoned for stress testing but used for rock mass permeability determination. At each of the 19 test sections an impression packer test was carried out after the completion of all hydraulic injection tests, in order to determine the spatial orientation of induced or stimulated fractures.

The following test sections in borehole KOV01 were selected (MD measured or driller depth, TVD true vertical depth) :

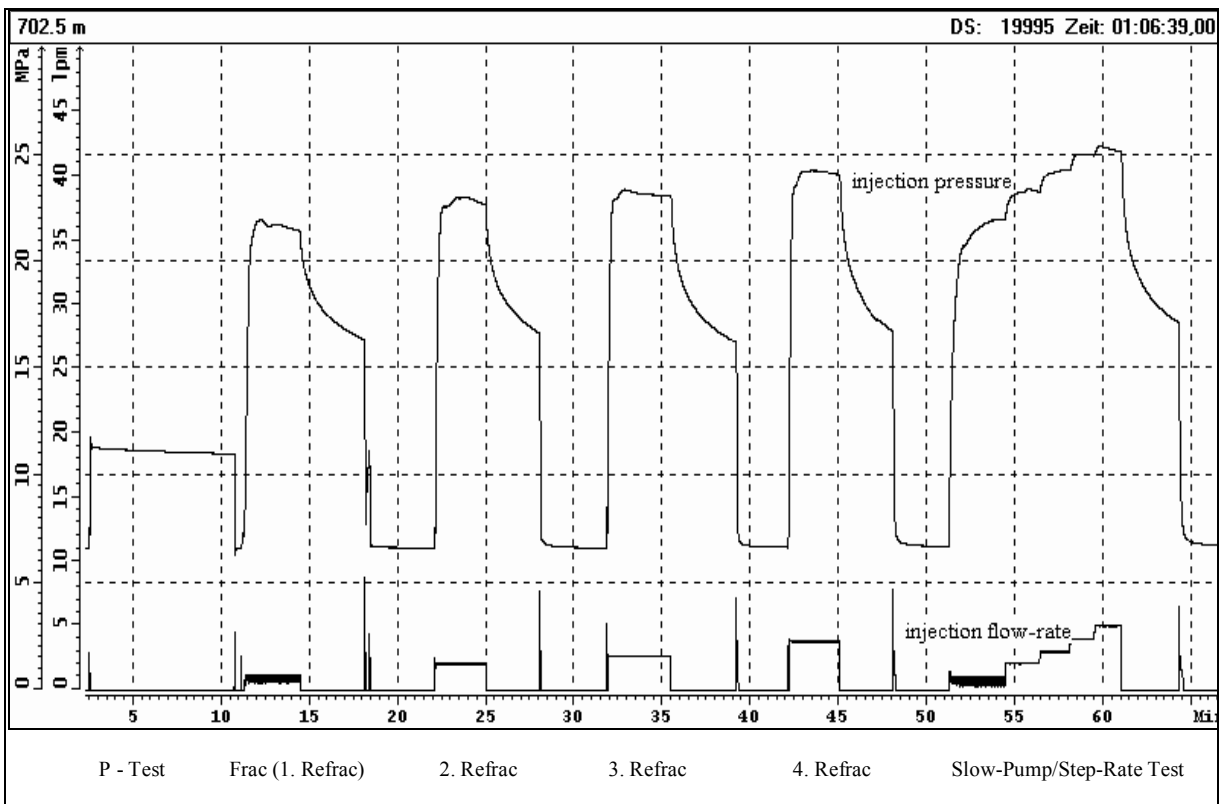
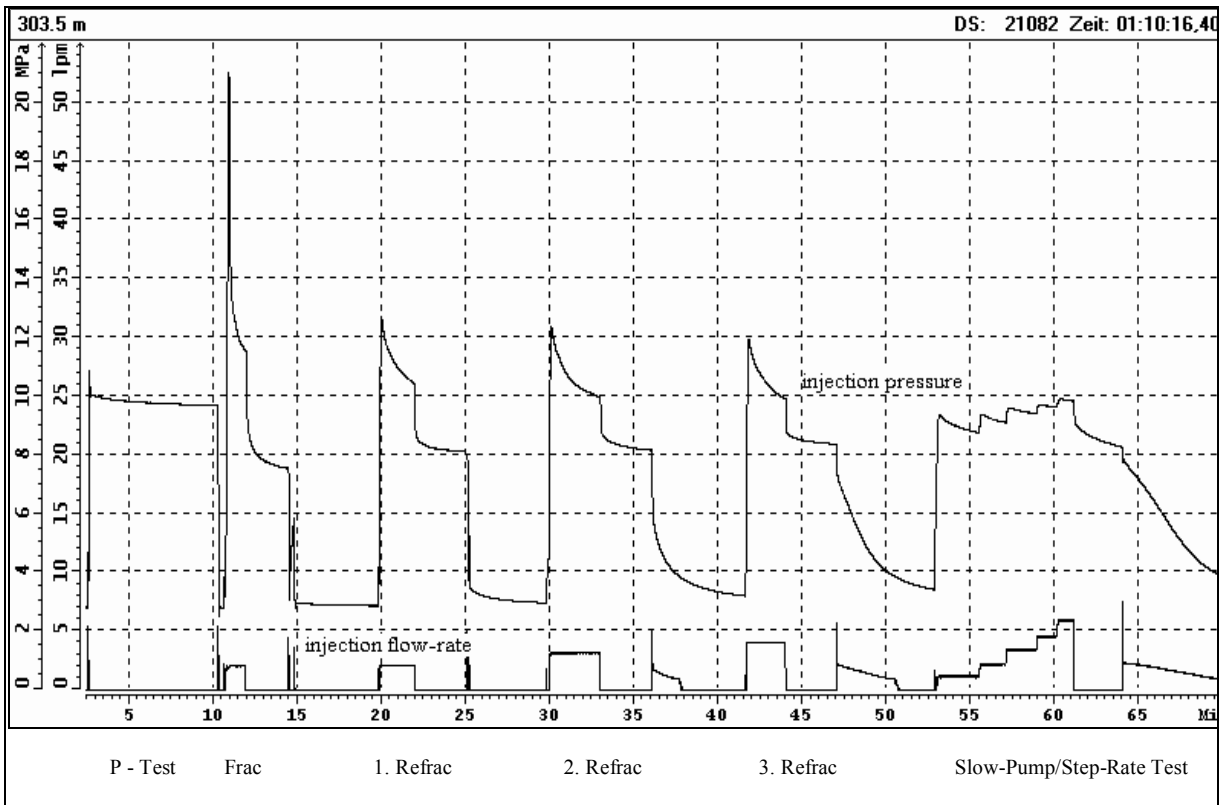
No.	MD (m)	TVD (m)
1	296.0	280.9
2	303.5	288.0
3	319.3	302.8
4	325.4	308.5
5	331.5	314.2
6	383.5	363.0
7	416.0	393.8
8	417.7	395.5
9	471.0	446.9
10	490.5	465.8
11	499.5	474.8
12	510.8	485.6
13	517.0	491.6
14	579.0	552.2
15	604.2	577.0
16	681.0	652.9
17	689.5	661.3
18	702.5	674.2
19	708.0	679.6
20	712.8	684.4

Two typical hydrofrac / hydraulic injection test records illustrating the test procedure are shown in Figure 3.4. The two examples were chosen to illustrate a test with a typical breakdown event (test at 303.5 m MD / 288.0 m TVD) and a test where obviously a pre-existing fracture was stimulated (test at 702.5 m MD / 674.2 m TVD). Each test consisted of the following injection cycles after packer inflation to a differential pressure of 15 - 20 MPa :

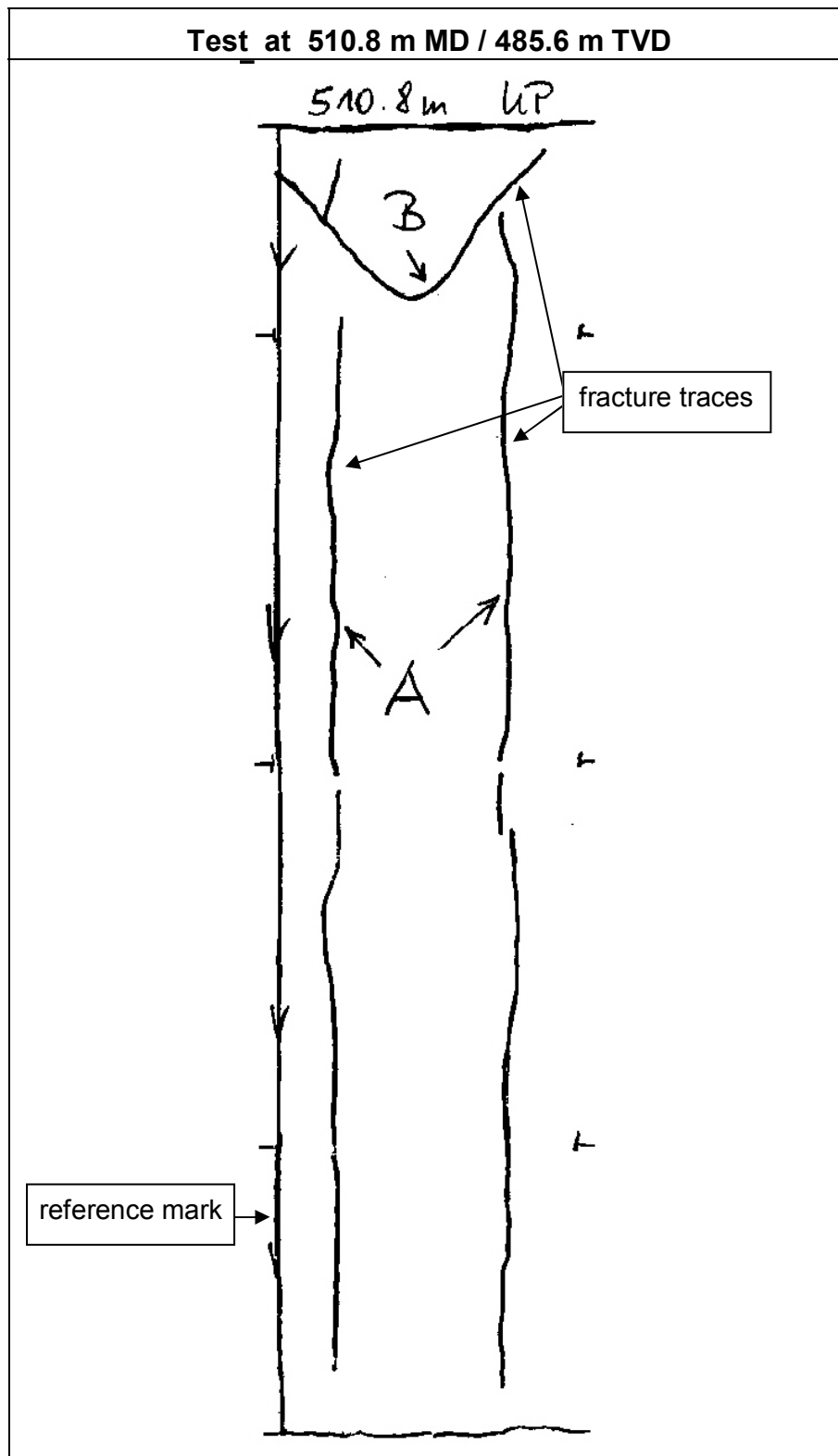
- rapid pressurization of the test interval to a differential pressure of app. 10 MPa and subsequent monitoring the pressure decay for several minutes (P-test),
- release of the interval pressure,
- pressurization of the test interval with an injection rate of 1 to 2 lpm and monitoring the peak pressure (Frac-test) or a rather flat injection pressure level for the case of the presence of a pre-existing fracture, then shut-in of the system by injection stop and closure of the pressure line valve on surface,
- release of the interval pressure and monitoring the recovered fluid volume,
- conduction of several repressurization cycles with injection rates of 1 to 5 lpm until constant injection pressure is observed (Refrac cycles), the system shut-in for determination of shut-in pressure values, and interval venting with fluid recovery monitoring after each Refrac cycle,
- conduction of a Step-Rate injection with stepwise increasing the injection rate and monitoring the corresponding pressure increase,
- finally shut-in and venting the test interval, then packer deflation and lifting the packer unit to the next interval (generally, testing during one trip is conducted starting from the deepest test section).

The impression packer tests were carried out by pressurizing the impression packer element to a pressure app. 20% above the fracture re-opening pressure for app. 30 minutes. After recovery of the packer tool to the surface, the fracture trace is marked on the packer sleeve and transferred to a transparent plastic cover sheet wrapped around the packer. The film disc of the single-shot unit was developed documenting the orientation of the reference mark with respect to magnetic North in Oskarshamn. A typical example of fracture traces marked on the plastic cover sheets is shown in Figure 3.5 for the test section at 510.8 m MD / 485.6 m TVD.





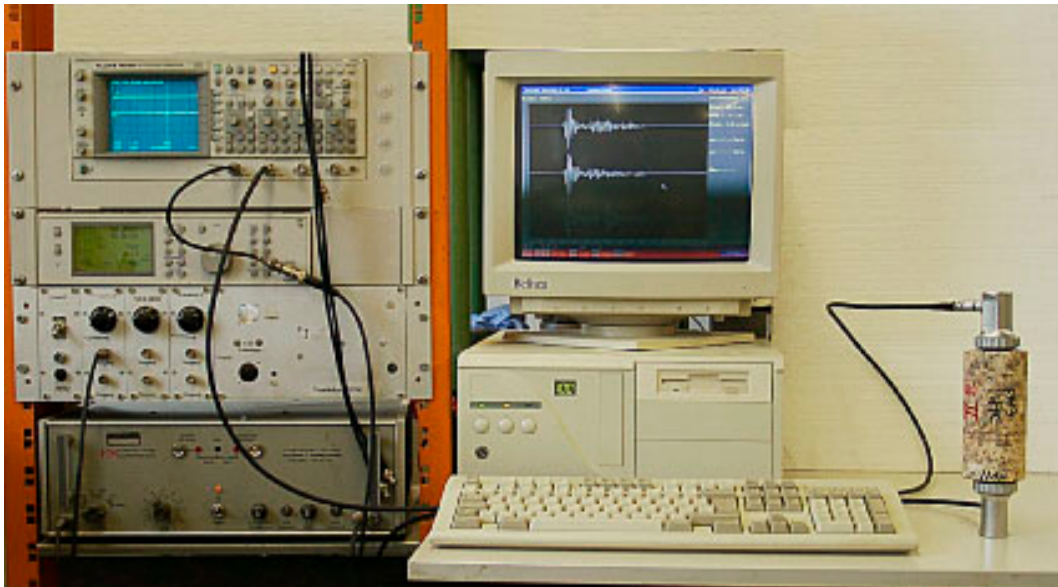
**Figure 3-4.** Test records from test sections at (a) 303.5 m (MD) / 288.0 m (TVD) and (b) at 702.5 m (MD) / 674.2 m (TVD).



**Figure 3-5.** A typical example of fracture traces on the packer sleeve of the impression packer test at 485.6 m (TVD) depth. The circumference of the packer is app. 230 mm, the length is app. 1000 mm.

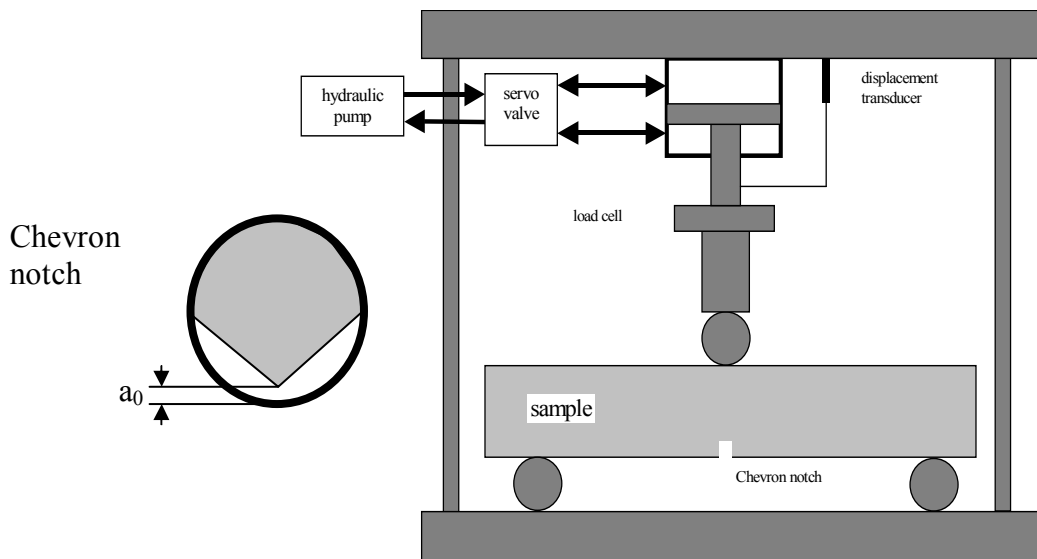
### 3.4 Laboratory Testing

The core material selected for material testing was first documented on a Core Piece Data Sheet (APPENDIX C). The core material is listed in Table 3.1. For core characterization density and ultrasonic velocities were measured on each core piece. Density measurements were done by the buoyancy method in water. Ultrasonic velocities were measured by pulse transmission across the core diameter according to the ISRM standard (1978). The experimental set-up is shown in Figure 3.6.



*Figure 3-6. Ultrasonic apparatus.*

Then, the original core pieces were used for fracture toughness tests after diamond-sawing of Chevron notches into the core pieces. Tests were done according to the ISRM standard (1988) by three-point loading with piston rate control. The piston displacement rate was app.  $1 \mu\text{m/s}$ . The experimental set-up is shown in Figure 3.7.

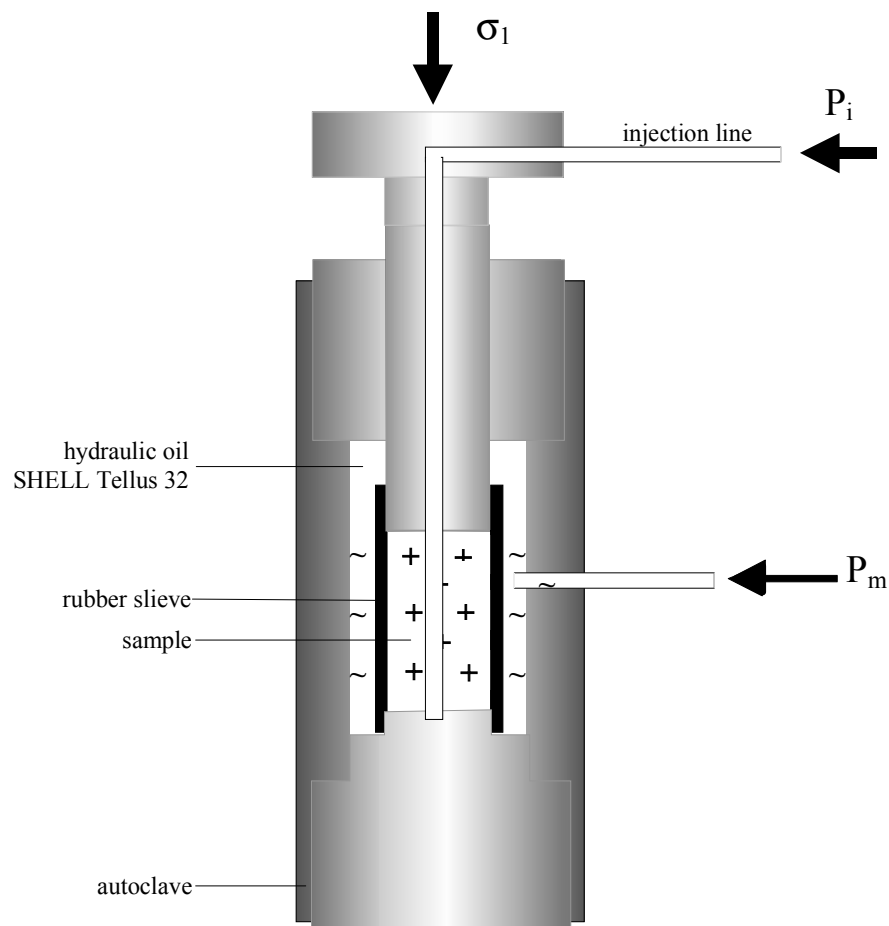


*Figure 3-7. Schematic view of the fracture toughness test apparatus.*

Laboratory hydrofrac tests were conducted on 50 mm long and 30 mm diameter core samples with a 3 mm diameter axial borehole for fluid injection. Sample preparation was carried out by diamond sawing, diamond drilling and diamond grinding using water as cooling fluid. Prior to hydrofracturing the samples were ultrasonic tested by axial pulse transmission for velocity determination. For hydrofrac tests the samples were jacketed with a rubber sleeve and subjected to axial stress and confining pressure. The experimental set-up is shown in Figure 3.8. The axial stress was slightly above the confining pressure. Internal pressurization was achieved by injection of oil (Shell Tellus 32, viscosity 25 cP) into the 3-mm diameter holes at a rate of 1 ml/s. Fracture initiation and dynamic fracture growth is characterized by a hydraulic short circuit between axial stress, confining pressure and injection pressure, which yields the breakdown pressure  $p_c$ . The dependence of  $p_c$  on the confining pressure  $p_m$  generally is given by a linear relation

$$p_c = p_{co} + k \cdot p_m$$

where  $p_{co}$  is the (hydraulic) tensile strength and  $k$  is the frac-coefficient. The confining pressure ranged from 0 to app. 40 MPa. Some hydrofrac tests with zero confining pressure were conducted on the original core pieces with axial holes of up to 8 mm diameter.



**Figure 3-8.** Test chamber for mini-frac testing.

**Table 3-1. Core material from borehole KOV01 for laboratory testing.**

core - piece no.	depth-range MD (m)	core-piece length min / max (mm)	diameter (mm)	remarks
1A	333.55 - 334.03	300 / 328	47.6	
1B		168 / 190	47.6	
2	382.96 - 383.35	390 / 402	47.4	
3A	438.12 - 438.53	140 / 150	47.4	
3B		145 / 165	47.4	
3C		105 / 120	47.4	
4A	463.60 - 464.10	160 / 175	47.4	
4B		175 / 202	47.4	
4C		140 / 180	47.4	
5	500.20 - 500.60	390 / 425	51.7	
6	559.70 - 560.10	400 / 420	47.2	
7	559.46 - 596.85	390 / 395	47.2	
8A	648.68 - 649.10	75 / 98	47.2	
8B		305 / 350	47.3	
9A	692.58 - 692.98	255 / 380	51.9	
9B		55 / 120	51.9	
10A	709.33 - 709.77	204 / 308	51.9	
10B		105 / 240	51.9	
G1 - G3 <sup>1)</sup>	app. 500	app. 500	47	3 pieces

<sup>1)</sup> This core from app. 500 m depth and app. 500 mm length consisted of 3 pieces. The core was provided to MeSy prior to in-situ testing for preliminary physical property evaluation. No core piece data sheet was made.



## 4 Results of In - Situ Tests

### 4.1 Description of In - Situ Test Results

The test records are presented in APPENDIX A. For each test an overview plot is given which contains the pressure and injection rate graphs for each test cycle plus information on the injected and recovered fluid volumes, zoomed graphs for the identification of breakdown and refrac pressure values, 3 different plots for the determination of the characteristic shut-in pressure value from different test cycles, and a detailed plot of the step-rate test together with a pressure vs. injection rate graph. The impressions on the impression packer sleeve are shown in APPENDIX B.

The MeSy Routine Analysis used in Appendix A to identify reliable characteristic hydrofrac pressure data, breakdown pressure  $P_c$ , refrac or reopening pressure  $P_r$ , and shut-in pressure  $P_{si}$  consists of the following procedures :

(i) The breakdown pressure defined as the peak pressure during the first pressurization cycle (frac cycle) is obtained from a zoomed pressure  $P$  vs. time  $t$  plot.

(ii) The refrac pressure is taken from the zoomed pressure  $P$  vs. injected fluid volume  $V$  plots, generally from the first refrac cycle, as the pressure level when the plot significantly deviates from the linear pressure increase determined by the system stiffness  $dP / dV$ .

(iii) The shut-in pressure  $P_{si}$  is determined by the following three-step procedure :

- the upper limit of  $P_{si}$  is derived from zoomed plots of pressure  $P$  vs. injection flow rate  $Q$  for different refrac cycles.  $P_{si,max}$  then correlates to the pressure level when  $Q = 0$ .

- The lower limit of  $P_{si}$  is derived from a Muskat - type plot of the difference between pressure  $P$  and an asymptotic pressure level  $P_a$  vs. time  $t$ , assuming that the linear portion of the plot is due to radial flow, i.e. the stimulated fracture is nearly closed.

- Then, the most reliable value of  $P_{si}$  is between the two limits and characterized by the transition of the initial rapid linear pressure drop to the beginning of a diffusion dominated slow pressure decay. This transition can be determined from the tangent to the linear pressure drop in the zoomed  $P$  vs.  $t$  plots for the shut-in stages of the various refrac cycles.

The  $P_{si}$  - value derived by this three-step analysis can be checked by the low step-rate injection test which is analysed similar as a classical Lugeon test using steady-state  $P$ - $Q$  data pairs from the  $P$  vs.  $t$  plot.

It should be mentioned that various techniques to derive  $P_{si}$  -values are used by different researchers. A good summary of the different methods is given by Guo et al. (1993).

(iv) The monitoring of the recovery fluid volume from a fracture after test completion may be used to speculate whether the induced fractures has connected to the natural fracture network in the rock mass during its propagation. Low recovery rates may indicate that the fracture has connection to a natural joint system which easily consumed fluid. High recovery rates are generally observed from tests on pre-existing fractures

which are connected to a large fluid reservoir. However, this field is still open to further research in geohydrology.

In order to evaluate the quality of the tests, first a short description of each test is presented:

#### ***Test at 296.0 m MD / 280.9 m TVD***

During the P-test the interval pressure rapidly decays from 8 to 5 MPa during 7 min., indicating a pre-existing fracture.

During the 4 subsequent injection cycles the fracture is repeatedly re-opened at a pressure level of 10 MPa (the detailed analysis yields a value of  $P_r \approx 9.5$  MPa).

After system shut-in the pressure rapidly drops to app. 8 MPa (the detailed routine analysis yields a value of  $P_{si} = 8.2$  MPa). The step-rate tests indicates that the fracture accepts 1 lpm at already 7.8 MPa.

It is interesting to note that app. 50% of the injected volume of 64 l is recovered.

The impression shows an inclined ( $46^\circ$ ) fracture trace with a strike direction of 161 degrees.

#### ***Test at 303.5 m MD / 288.0 m TVD***

The P-test shows a completely tight test interval.

Consequently, we observe a typical Frac-cycle with  $P_c = 21$  MPa, and almost identical Refrac-cycles with peak pressures of app. 12 MPa (the detailed analysis yields  $P_r = 11.7$  MPa), and we can derive a  $P_{co}$  value of app. 9 MPa.

After shut-in the pressure rapidly drops to app. 8.5 MPa (the detailed routine analysis yields  $P_{si} = 8.8$  MPa). During the Step-Rate test the induced fracture is already open at 9.3 MPa for a injection rate of 1.1 lpm.

A large quantity of the injected water was recovered (36%).

The impression test shows one inclined ( $26^\circ$ ) fracture with 137 degrees strike direction.

#### ***Test at 319.3 m MD / 302.8 m TVD***

The P-test yields a pressure drop of only 0.1 MPa during app. 6 min, indicating a tight test interval.

The frac cycle is characterized by a distinct breakdown event (frac initiation at 17 MPa). All 3 Refrac cycles yield rather distinct refrac pressure peaks of app. 14.5 MPa (the detailed analysis shows  $P_r = 13.5$  MPa which indicated a value of  $P_{co} = 3.5$  MPa).

After the injection is stopped the pressure rapidly decreases to app. 11 MPa (the detailed analysis shows  $P_{si} = 11.5$  MPa). During the step-rate test the induced fracture clearly is open at 13.4 MPa for an injection rate of 1.4 lpm.

The impression shows 2 more or less axial fracture traces with almost identical strike direction of 144 and 148 degrees.



### ***Test at 325.4 m MD / 308.5 m TVD***

The P-test shows a rather tight test interval with a pressure decline of 0.5 MPa within 7.5 min.

The Frac-cycle does not show a distinct breakdown ( $P_c = 15.5$  MPa) when compared to the peaks of the 2 last Refrac-cycles (14.1 MPa), which indicates a low tensile strength of only app. 1.5 to 2 MPa (the detailed analysis yields a value of  $P_r = 13.5$  MPa).

After system shut-in the pressure rapidly drops to app. 12 MPa. The routine analysis derives a shut-in pressure value of  $P_{si} = 12.4$  MPa. The Step-Rate test shows that the fracture is already fully open at 12.6 MPa.

Only 9% of the injected water is recovered.

The impression shows 2 axial fracture traces with a strike direction of 141 degrees.

### ***Test at 331.5 m MD / 314.2 m TVD***

The P-test indicates some hydraulic leakage with a pressure decay of 0.9 MPa within 7.5 min.

In spite of the leakage the first injection cycle shows a distinct breakdown event with  $P_c = 13.8$  MPa. Subsequent cycles are to some extent anomalous (sudden pressure decrease after an initial peak, then again a pumping pressure peak). The detailed analysis shows  $P_r = 10$  MPa.

After injection stop the pressure declines rather gradual. The routine analysis yields a shut-in pressure of 9.5 MPa, however the Step-Rate test shows that the fracture is quite open at 9.5 MPa.

Only a small quantity of the injected water is recovered (4%).

The impression shows an inclined fracture with 176° strike direction.

### ***Test at 383.5 m MD / 363.0 m TVD***

The P-test indicates a tight test interval with a linear pressure decline of only 0.5 MPa during 8 min..

Consequently we observe a typical Frac-cycle with a breakdown at  $P_c = 17.7$  MPa. Subsequent injection cycles demonstrate distinct peaks at app. 14 MPa (the detailed analysis results in a re-opening pressure value of  $P_r = 12.4$  MPa).

After shut-in the pressure rapidly drops to app. 10 MPa in each of the cycles (the routine analysis determines a shut-in pressure of 10.1 MPa). The Step-Rate test indicates that the induced fracture is fully open at 11.2 MPa.

The impression shows an axial fracture with 132° strike direction.

### ***Test at 416 m MD / 393.8 m TVD***

During the P-test the pressure significantly decreases from app. 10 MPa to app. 6 MPa within 7 min. indicating the presence of a pre-existing fracture.

The peak pressure during the first cycle ( $P_c = 12.1$  MPa) is only slightly above the peak pressures of the subsequent cycles ( $P \approx 10.5$  MPa). The routine analysis of the first cycle yields the value  $P_r = 11$  MPa.

After shut-in the pressure drops rapidly to app. 8 MPa. The routine  $P_{si}$  - analysis derives a shut-in pressure value of  $P_{si} = 8.2$  MPa.

A very small quantity of the injected volume was recovered.

The impression shows one axial fracture trace with  $144^\circ$  strike direction.

#### ***Test at 471 m MD / 446.9 m TVD***

P-test : rapid pressure decline from app. 10 MPa to app. 7 MPa within 6.5 min..

Distinct peaks during the first 2 injection cycles (app.  $P_c = 13$  MPa), anomalous pumping pressure during the last 2 cycles, with a pumping pressure  $P_p$  of app. 9.5 MPa. The routine analysis of the first cycle yields a value of  $P_r = 12.0$  MPa.

After shut-in we only observe a small instantaneous pressure drop ( $P_{si} \approx P_p$ ). The routine analysis yields a shut-in pressure of 9.4 MPa, the Step-Rate tests demonstrates that the fracture is fully open at 9.5 MPa.

25% of the injected fluid is recovered.

The impression shows a complex set of fracture traces, two app. axial traces with  $158^\circ$  and  $107^\circ$  strike direction, and one almost horizontal fracture .

#### ***Test at 490.5 m MD / 465.8 m TVD***

P-test with almost zero pressure decay during 7 min. indicating an unfractured test section.

Consequently a prominent Frac-cycle with  $P_c = 22.4$  MPa. Subsequent injection cycles, however, are more difficult to understand. From the last 2 injection cycles a refrac pressure of  $P_r \approx 17$  MPa may present a valid value, which then results in  $P_{co} \approx 5$  MPa.

In each of the cycles we observe an instantaneous pressure drop to app. 10 MPa (the detailed routine analysis yields a value of  $P_{si} \approx 10.3$  MPa). The induced fracture is clearly open at 12 MPa (Step-Rate test).

Only a small fraction of water is recovered.

On the impression we observe an axial fracture trace with  $102^\circ$  strike direction.

#### ***Test at 499.8 m MD / 474.8 m TVD***

The P-test suggests the presence of an open pre-existing fracture which permits migration of water at 8 MPa.

The 3 injection cycles show pumping pressures  $P_p$  of app. 11 MPa independent on injection rates. The identification of a re-opening pressure in this case is difficult. The routine analysis would give a value of  $P_r = 6.6$  MPa. On the overview plot a major change in pressure increase in the last 2 Refrac-cycles can be observed at app. 10 MPa.

After shut-in we only observe a slight pressure drop ( $P_p \approx P_{si}$ ). The routine analysis for shut-in pressure determination confirms this result.

It is difficult to understand the large quantity of recovered water (31%).

On the impression we observe a set of 3 steeply inclined fracture traces with a mean strike direction of  $157 \pm 16$  degrees.

#### ***Test at 510.8 m MD / 485.6 m TVD***

P-test : a perfectly tight interval.

Consequently, a characteristic Frac-test with  $P_c = 24.1$  MPa. Independent of injection rate the fracture can be extended by a pumping pressure of app.  $P_p \approx 12.5$  MPa. The tensile strength can be estimated as  $P_{co} \approx 12$  MPa.

After shut-in the instantaneous pressure drop is small, the routine analysis yields  $P_{si} = 11.5$  MPa. The fracture is open at 11.4 MPa (Step-Rate test).

11% of the injected water is recovered.

The impression shows 2 steeply inclined (axial) fractures with a mean strike direction of  $172 \pm 4$  degrees.

#### ***Test at 517 m MD / 491.6 m TVD***

The P-test indicates the presence of a pre-existing fracture.

For a small injection rate during the first injection cycle no pressure peak is observed. During subsequent pressure cycles the mean pumping pressure is  $P_p \approx 13$  MPa. The fracture opens at about  $P_r = 9.6$  MPa (routine analysis).

After shut-in we can identify a rather clear shut-in pressure of 11.5 MPa. At 11.7 MPa the fracture is clearly open (Step-Rate test).

12% of the injected water is recovered.

The impression shows an axial fracture with  $161^\circ$  strike.

#### ***Test at 579 m MD / 552.2 m TVD***

The P-test suggests a tight test section.

The Frac-test yields a breakdown of  $P_c = 17.6$  MPa, subsequent injection cycles show distinct re-opening pressures of app. 14.5 MPa and a pumping pressure of app. 15 MPa (the routine analysis yields  $P_r = 13.4$  MPa).  $P_{co}$  can be estimated as app. 4 MPa.

No distinct pressure drop can be observed after system shut-in ( $P_{si} \approx P_p$ ). The detailed routine analysis yields  $P_{si} = 14.7$  MPa, which is confirmed by the Step-Rate test (14.4 MPa).

Only a small fraction of water is recovered (6%).

The impression shows an axial fracture trace with  $179^\circ$  strike direction.

### ***Test at 604.2 m MD / 577 m TVD***

The P-test shows a gradual pressure decline indicating the presence of a permeable fracture.

During the injection cycles the peak pressure drops from  $P = 14.2$  MPa for the first cycle to app.  $P_p = 12.5$  MPa during the last two cycles, where the peak pressure corresponds to the pumping pressure. The fracture opens significantly at app. 12 MPa (the detailed routine analysis suggest a value of  $P_r \approx 9.8$  MPa).

After shut-in we only observe a small instantaneous pressure drop to app. 12 MPa (the routine analysis yields a value of  $P_{si} = 12.1$  MPa). The Step-Rate test, however, indicates an open fracture at 11.4 MPa for an injection rate of app. 1 lpm.

The recovered water volume is 23%.

The impression shows 4 traces of axial and steeply inclined fractures with a mean strike of  $107 \pm 27$  degrees.

### ***Test at 681 m MD / 652.9 m TVD***

P-test : a perfectly tight test section.

Consequently, a typical Frac-cycle with a high breakdown pressure of  $P_c = 19.6$  MPa. Subsequent injection cycles show distinct pressure peaks at app. 14.5 MPa (the detailed routine analysis yields  $P_r = 14.3$  MPa for the first refrac cycle). As also observe in some other ideal frac experiments the pumping pressure increased during each cycle.

After shut-in we only observe a small instantaneous drop from the pumping pressure at app. 14.4 MPa. The routine analysis yields  $P_{si} = 13.7$  MPa. At 13.9 MPa the fracture is open (Step-Rate test).

About 20% of water is recovered after the test.

The impression shown an induced axial fracture of  $147^\circ$  strike.

### ***Test at 689.5 m MD / 661.3 m TVD***

Although the P-test shows a significant pressure decay, the subsequent injection results in a rather high breakdown pressure of  $P_c = 18.1$  MPa. During subsequent injection cycles the pumping pressure increases with time. The fracture opens at app. 14.2 MPa (the detailed analysis yields  $P_r = 14.5$  MPa for the first refrac cycle).

After all injection cycles we observe small but clear instant pressure drops however, to different pressure levels (13.7, 14.3, 15.0 MPa); the routine analysis yields  $P_{si} = 13.7$  MPa for the first cycle). The Step-Rate test shows that the fracture is clearly conductive at 14 MPa.

The recovery is 11%.

The impression shows axial to steeply inclined fracture traces with a mean strike direction of  $135 \pm 9$  degrees.

### ***Test at 702.5 m MD / 674.2 m TVD***

The P-test yields a pressure drop of only 0.3 MPa in 8 min, indicating a tight test interval.

None of the injection cycles demonstrates a pressure peak. Fluid can be injected at constant pressure levels depending on injection rate. Thus, a clear indication for a pre-existing fracture which is opened at app. 17 MPa and is hydraulically fully active at pressures above 20 MPa (see Step-Rate test).

After injection stop in the first cycle the pressure rapidly decreases to app. 20.7 MPa (the detailed analysis yields a value of  $P_{si} = 20.7$  MPa).

The impression shows 3 more or less axial fracture traces with a mean strike direction of 132 degrees.

### ***Test at 708 m MD / 679.6 m TVD***

No pressure drop during the P-test.

A breakdown at  $P_c = 24.9$  MPa, followed by refrac cycles with distinct peaks at app. 22 MPa.

Clear shut-in pressures of app. 16 MPa (the routine analysis shows 16.7 MPa). From the Step-Rate test we observe that the fracture is open at 17.7 MPa.

Almost no recovery.

The impression shows an axial pressure of 127 degrees strike.

### ***Test at 712.8 m MD / 684.4 m TVD***

The P-test shows an almost linear pressure drop of 0.8 MPa during 8.5 min.

None of the injection cycles demonstrates a distinct pressure peak. Fluid can be injected at a constant pressure level of app. 22 MPa almost independent of injection rate. Thus, a clear indication for a pre-existing fracture, which is opened at app. 18 to 19.5 MPa. The Step-Rate test shows that the fracture is hydraulically fully active above 19.9 MPa.

After injection stop the pressure only slightly decreases rapidly, followed by a more continuous pressure decay which makes it difficult to identify a valid shut-in pressure (the detailed analysis procedure gives the value  $P_{si} = 19.7$  MPa, with a minimum value of  $P_{si, min} = 15$  MPa).

The impression shows 1 axial fracture trace with a strike direction of 133 degrees and 2 inclined fracture traces with similar strike direction, dip, and dip direction.

## 4.2 Characteristic Pressure Data

The characteristic pressure data are listed in Table 4.1. Due to the uncertainty of the experimental data of the refrac- and shut-in pressures both, the exact values derived from the detailed routine analysis and the range of uncertainty determined from a more qualitative evaluation are given. The  $P_r$ - and  $P_{si}$  data from the routine analysis are presented in Figure 4.1 and Figure 4.2.

The value  $P_{max}$  generally is the breakdown pressure  $P_c$  at the moment of fracture initiation, but we note that the peak pressure during the first injection cycle can also be determined by the stimulation of a pre-existing fracture. Thus, we must differentiate two categories of tests:

- tests where unequivocally new fractures were induced at test sections with low pressure decay during the P-test and characterized by unique Frac-events with a high breakdown pressure; we observe 10 of such tests;
- tests on sections with obviously pre-existing fractures, generally characterized by a significant pressure decay during the P-test and by the absence of a typical breakdown event; we also observe 10 of such tests.

The fracture re-opening pressure  $P_r$  in some cases was difficult to identify since even "closed" fractures in crystalline rock accept significant amounts of fluid during low-rate injection during Step-Rate tests.

From the  $P_c$ - and  $P_r$  - data of tests on intact rock sections we can derive the (hydraulic) tensile rock strength  $P_{co}$ . The  $P_{co}$ -data range between 1.5 to 12 MPa with a mean of  $P_{co} = (5.2 \pm 3)$  MPa.

Similar, the shut-in pressure data  $P_{si}$  in most cases were difficult to determine, because either there was no instantaneous rapid pressure drop at system shut-in, or the shut-in pressure varied for the different test cycles. Nevertheless, the routine detailed analysis provides rather objective values which in most cases can be confirmed or at least supported from the results of the Step-Rate tests.

We realize rather high  $P_r$ - and  $P_{si}$  - values in the depth range from 290 to 310 m and in the range from 675 to 685 m, while the pressure values between almost linearly increase with depth.

**Table 4-1. Characteristic pressure data ( $P_{max}$ : maximum pressure, e.g. breakdown pressure,  $P_r$ : refrac pressure,  $P_{si}$ : shut-in pressure, MD: measured depth, TVD: true vertical depth).**

MD (m)	TVD (m)	$P_{max}$ (MPa)	routine analysis (MPa)	$P_r$ range <sup>1)</sup> (MPa)	routine analysis (MPa)	$P_{si}$ range <sup>1)</sup> (MPa)
296.0	280.9	10.5	9.5	8.2 - 10	8.2	8.1 - 8.2
303.5	288.0	20.9	11.7	9.3 - 12	8.8	8.4 - 8.8
319.3	302.8	17.0	13.5	13.4 - 14.5	11.5	9.2 - 11.5
325.4	308.5	15.5	13.5	12.6 - 14.1	12.4	10.9 - 12.4
331.5	314.2	13.8	10.0	9.5 - 10.5	9.5	7.8 - 9.5
383.5	363.0	17.7	12.4	12.5 - 14	10.1	8.8 - 10.1
416.0	393.8	12	11.0	9.5 - 11.0	8.2	7.2 - 8.2
417.7	395.5	-	-	> 7.5 <sup>2)</sup>	-	> 7.5 <sup>2)</sup>
471.0	446.9	13.5	12.0	8.9 - 12	9.4	9.0 - 9.4
490.5	465.8	22.4	18.3	12.5 - 18.3	10.3	9.4 - 10.3
499.5	474.8	11.5	6.6	10 - 11	11.1	10.2 - 11.1
510.8	485.6	24.1	10.7	10.7 - 12.5	11.5	10.3 - 11.5
517.0	491.6	14	9.6	9.6 - 13	11.6	10.8 - 11.6
579.0	552.2	17.6	13.4	13.4 - 15	14.7	13.8 - 14.7
604.2	577.0	14.2	9.8	9.8 - 12	12.1	11.2 - 12.1
681.0	652.9	19.6	14.3	13.9 - 14.3	13.7	12.9 - 13.7
689.5	661.3	18.1	14.5	14 - 14.5	13.7	12.6 - 14.0
702.5	674.2	24	17.2	17.2 - 20	20.7	18 - 20.7
708.0	679.6	24.9	21.2	17.7 - 21.2	16.7	15 - 16.7
712.8	684.4	22	19.4	19.4 - 20	19.7	15 - 19.7

<sup>1)</sup> The routine analysis values were derived as described in Section 4.1. The range values indicate a subjective uncertainty when values are directly determined from the overview plots (see test description in Section 4.1).

<sup>2)</sup> no stimulation conducted, but used for permeability estimation

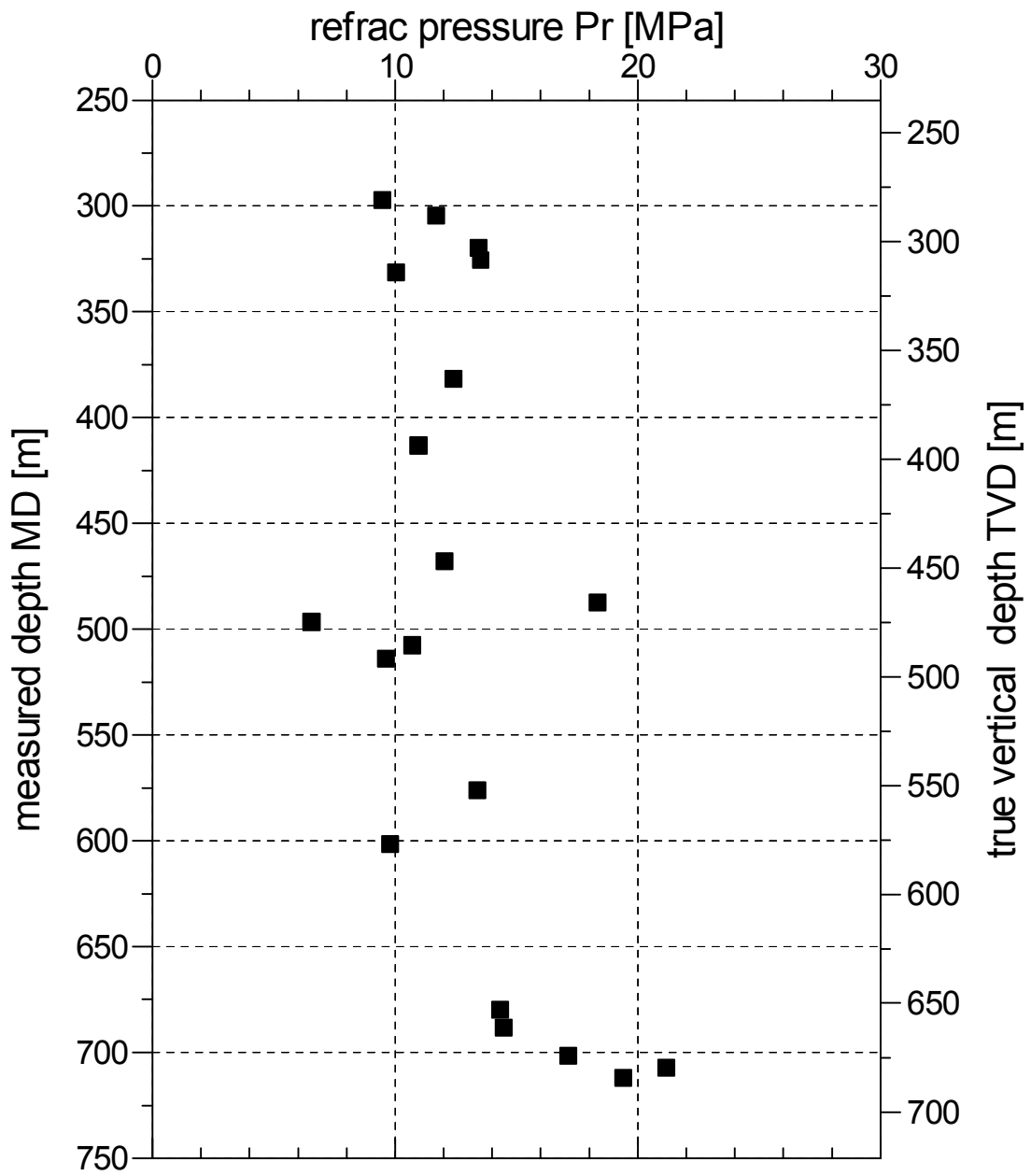


Figure 4-1. Refrac pressure  $P_r$ .



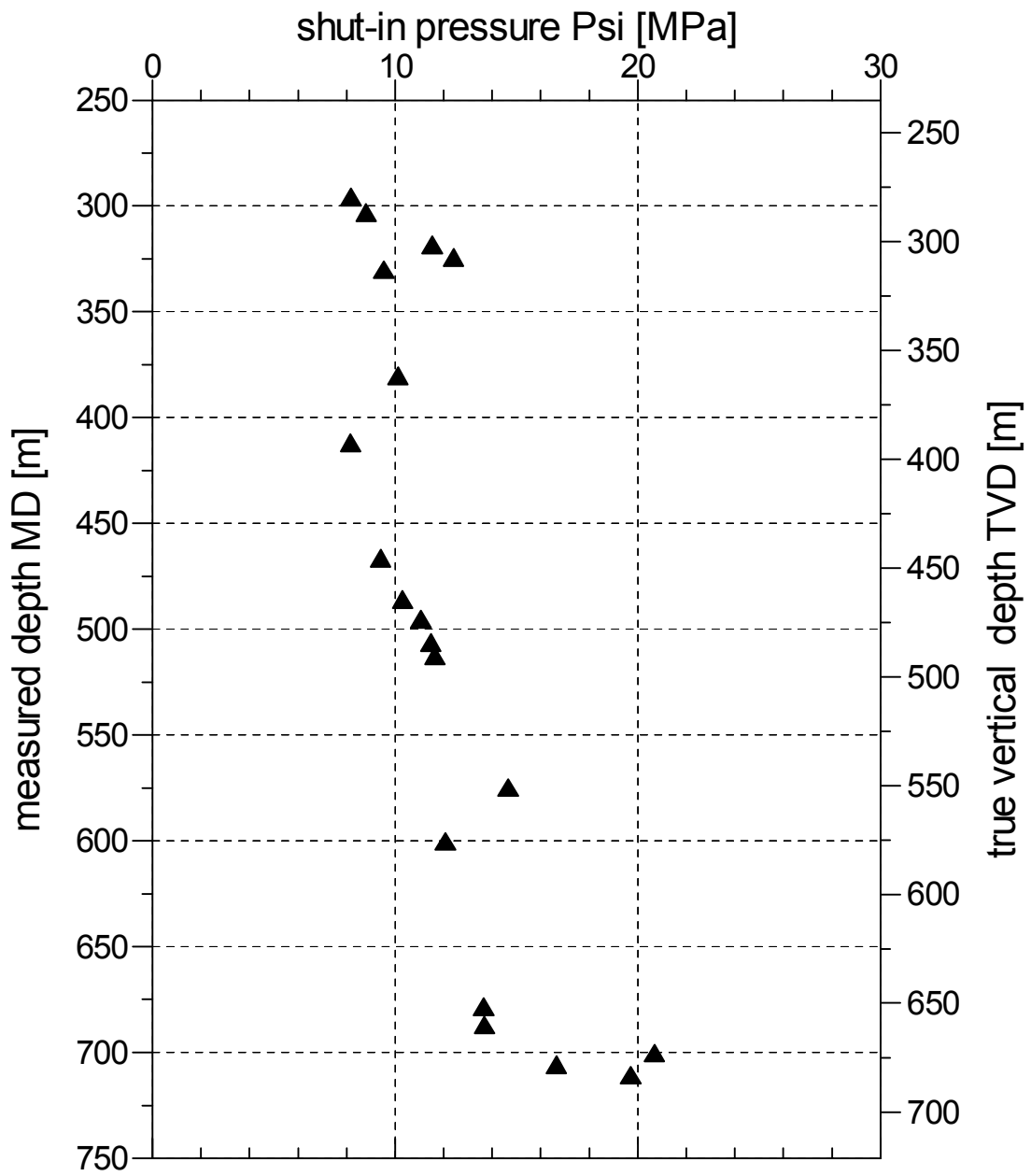


Figure 4-2. Shut-in pressure Psi.

### 4.3 Fracture Orientation Data

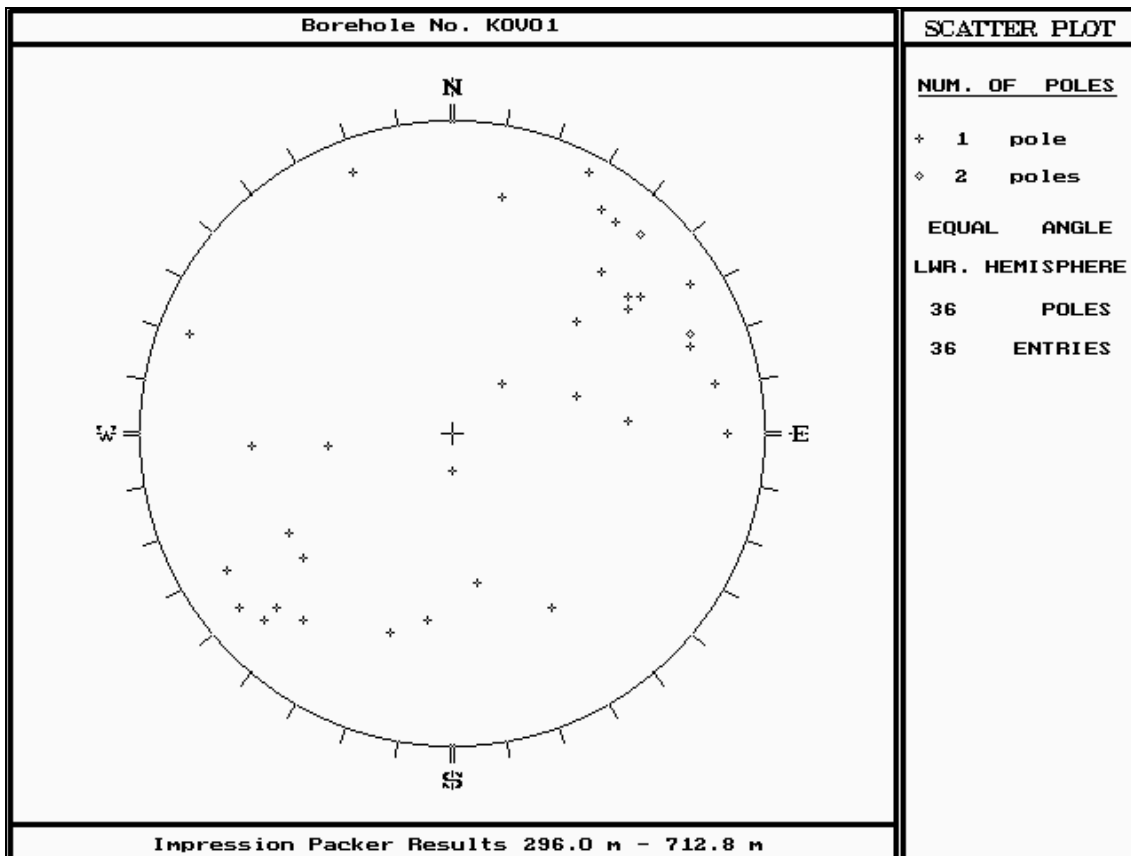
The information from the impression packer tests is summarized in Table 4.2, giving the strike, dip and dip directions of fractures identified from the fracture traces. Strike directions and dip are shown graphically in Figure 4.3 and in the lower hemisphere polar plot of Figure 4.4. It should be noted that the uncertainty of the orientation is app.  $\pm 15$  degrees, mainly due to the small borehole diameter. The angular values in Table 4.2 and Figures 4.3 and 4.4 are corrected with respect to the borehole inclination and with respect to the magnetic declination at Oskarshamn. To compensate the local magnetic declination 3 degrees have been added to the compass readings.

In most cases axial fractures (axial with respect to the borehole axis) were induced. Some of the stimulated inclined fractures (inclined with respect to the borehole axis) were nearly vertical fractures except the inclined fractures in the test sections at 280.9, 288 or 314.2 m (TVD). Surprisingly, all of the induced or stimulated fractures show azimuths between app. 102 and 175 degrees with a mean value of  $145 \pm 18$  degrees (induced axial fractures :  $148 \pm 16$  degrees, stimulated axial fractures :  $135 \pm 15$  degrees). No conjugated natural joints / fractures with NE - SW strike was observed which one generally expects in crystalline rock masses. This fact creates a major difficulty to apply the PSI- or HTPF - inversion method to derive stresses from shut-in pressure data.

**Table 4-2. Orientation of induced or stimulated fractures derived from impression packer testing in borehole no. KOV01.  $\theta$ : strike direction (counted North over East),  $\beta$ : dip direction (counted North over East),  $\alpha$ : dip (with respect to horizontal).**

measured depth MD (m)	true vertical depth TVD (m)	fracture trace	$\theta$ (deg)	$\beta$ (deg)	$\alpha$ (deg)	remarks
296.0	280.9	A	161	251	46	inclined fracture
303.5	288.0	A	137	227	26	inclined fracture
319.3	302.8	A	144	234	71	axial fracture
		B	148	58	81	inclined fracture
325.4	308.5	A	141	231	71	axial fracture
		B	141	51	83	inclined fracture
331.5	314.2	A	176	86	42	inclined fracture
383.5	363.0	A	132	222	70	axial fracture
416.0	393.8	A	144	234	74	axial fracture
471.0	446.9	A	158	248	78	axial fracture
		B	107	17	69	inclined fracture at upper end of impression tool
		C	92	2	13	horizontal fracture at lower end of impression tool
490.5	465.8	A	102	192	77	axial fracture
499.8	474.8	A	175	265	58	inclined fracture / joint
		B	158	248	79	axial fracture
		C	137	227	58	inclined fracture
510.8	485.6	A	168	258	80	axial fracture
		B	176	86	64	inclined fracture at upper end of impression tool
517.0	491.6	A	161	251	79	axial fracture
579.0	552.2	A	179	269	83	axial fracture
		B	21	110	83	inclined fracture at lower end of impression tool
604.2	577.0	A	119	209	90	steeply inclined fracture
		B	68	158	83	short axial single trace
		C	98	8	62	inclined fracture
		D	141	51	64	inclined fracture at lower end of impression tool
681.0	652.9	A	147	237	84	axial fracture
689.5	661.3	A	124	214	82	axial fracture
		B	135	45	79	inclined fracture at upper end of impression tool
		C	147	57	62	inclined fracture at lower end of impression tool
702.5	674.2	A	133	223	83	axial fracture
		B	135	45	76	inclined fracture
		C	128	38	76	inclined fracture at lower end of impression tool
708.0	679.6	A	127	217	82	axial single trace
712.8	684.4	A	133	223	82	axial fracture
		B	60	330	64	inclined fracture
		C	82	352	51	inclined fracture at lower end of impression tool





*Figure 4-4. Orientation of induced or stimulated fractures derived from impression packer testing in borehole no. KOV01 (lower hemisphere polar - plot).*

#### 4.4 Observation of Water Recovery

In the test summary on the overview plots for each test (APPENDIX A) the total volume of injected water and the recovered water after the test is given. The information is summarized in Table 4.3.

It is interesting to note that the water recovery from stimulated pre-existing fractures is significant (say 25%) compared to the poor recovery from new induced fractures (some % only). However, there are some exceptions:

- the induced fractures at 288 m and 652.9 m show high recovery of 36 and 20%, respectively,
- the stimulated fractures at 393.8, 674.2 and 684.6 show poor recovery (1 - 3.5%).

**Table 4-3. Injected and recovered water volumes.**1) *induced fracture*2) *stimulated fracture*3) *no stimulation test, but used for permeability analysis*

MD (m)	TVD (m)	P <sub>max</sub> (MPa)	V <sub>inj</sub> (l)	V <sub>rec</sub> (l)	V <sub>rec</sub> (%)
296.0	280.9	10.5 <sup>2)</sup>	64	30	47
303.5	288.0	20.9 <sup>1)</sup>	49	18	36
319.3	302.8	17.0 <sup>1)</sup>	52	0.6	1
325.4	308.5	15.5 <sup>1)</sup>	56	5	9
331.5	314.2	13.8 <sup>1)</sup>	47	2	4
383.5	363.0	17.7 <sup>1)</sup>	66	1	1
416.0	393.8	12 <sup>2)</sup>	59	0.4	1
417.7	395.5 <sup>3)</sup>	-	-	-	-
471.0	446.9	13.5 <sup>2)</sup>	57	14	25
490.5	465.8	22.4 <sup>1)</sup>	66	1	2
499.8	474.8	11.5 <sup>2)</sup>	95	30	31
510.8	485.6	24.1 <sup>1)</sup>	45	5	11
517.0	491.6	14 <sup>2)</sup>	52	6	12
579.0	552.2	17.6 <sup>1)</sup>	55	3.5	6
604.2	577.0	14.3 <sup>2)</sup>	68	16	23
681.0	652.9	19.6 <sup>1)</sup>	44	9	20
689.5	661.3	18.1 <sup>2)</sup>	53	6	11
702.5	674.2	24 <sup>2)</sup>	54	2	4
708.0	679.6	24.9 <sup>1)</sup>	50	2	4
712.8	684.4	22 <sup>2)</sup>	50	1	2

## 4.5 Analysis of the Pressure Pulse Test

Before each hydraulic fracturing test cycle a short pressure pulse test (P-test) was conducted to test the suitability of the test interval for the subsequent fracturing test, but also to determine the rock permeability of the test section. The permeability estimate is based on the classical method suggested by Cooper et al. (1967) for the analysis of conventional slug tests. For the high stiffness wireline packer system MeSy has developed the software code PERM where theoretical and measured pressure decays are compared (master curve matching method). The theoretical pressure decay curves are calculated with a variety of input parameters such as storage coefficient and rock permeability.

The analysis of the pressure pulse tests is given in APPENDIX I. The derived permeability and conductivity data for each test interval are given in Table 4.4 and are graphically shown in Figure 4.5. Tight test intervals without pre-existing fractures are characterized by a rock permeability of app. 1  $\mu$ Darcy while test intervals with obvious pre-existing fractures show apparent permeability values of some tens of  $\mu$ Darcy.

**Table 4-4. Test section permeability / conductivity<sup>1)</sup>.**

measured depth MD (m)	true vertical depth TVD (m)	permeability k ( $\mu\text{Darcy} = 10^{-18} \text{ m}^2$ )	conductivity K m/s
296.0	280.9	26.4	$26 \cdot 10^{-11}$
303.5	288.0	1.5	$0.1 \cdot 10^{-11}$
319.3	302.8	0.6	$0.6 \cdot 10^{-11}$
325.4	308.5	1.3	$1.3 \cdot 10^{-11}$
331.5	314.2	2.1	$2.0 \cdot 10^{-11}$
383.5	363.0	1.3	$1.3 \cdot 10^{-11}$
416.0	393.8	19.5	$19 \cdot 10^{-11}$
417.7	395.5	47.3	$46 \cdot 10^{-11}$
471.0	446.9	17.0	$17 \cdot 10^{-11}$
490.5	465.8	0.8	$0.8 \cdot 10^{-11}$
499.8	474.8	13.1	$13 \cdot 10^{-11}$
510.8	485.6	0.9	$0.9 \cdot 10^{-11}$
517.0	491.6	15.9	$15 \cdot 10^{-11}$
579.0	552.2	1.3	$1.3 \cdot 10^{-11}$
604.2	577.0	4.7	$4.6 \cdot 10^{-11}$
681.0	652.9	0.3	$0.3 \cdot 10^{-11}$
689.5	661.3	12.5	$12 \cdot 10^{-11}$
702.5	674.2	2.0	$2 \cdot 10^{-11}$
708.0	679.6	0.4	$0.4 \cdot 10^{-11}$
712.8	684.4	2.5	$2.5 \cdot 10^{-11}$

$$1) \quad K = \frac{k \cdot \rho_{\text{water}} \cdot g}{\eta}$$

- $\rho_{\text{water}}$  : density of water ( $10^3 \text{ kg} / \text{m}^3$ )  
 $g$  : gravity acceleration ( $9.81 \text{ m/s}^2$ )  
 $\eta$  : viscosity of water ( $1 \text{ cP} = 10^{-3} \text{ Pa}\cdot\text{s}$ )

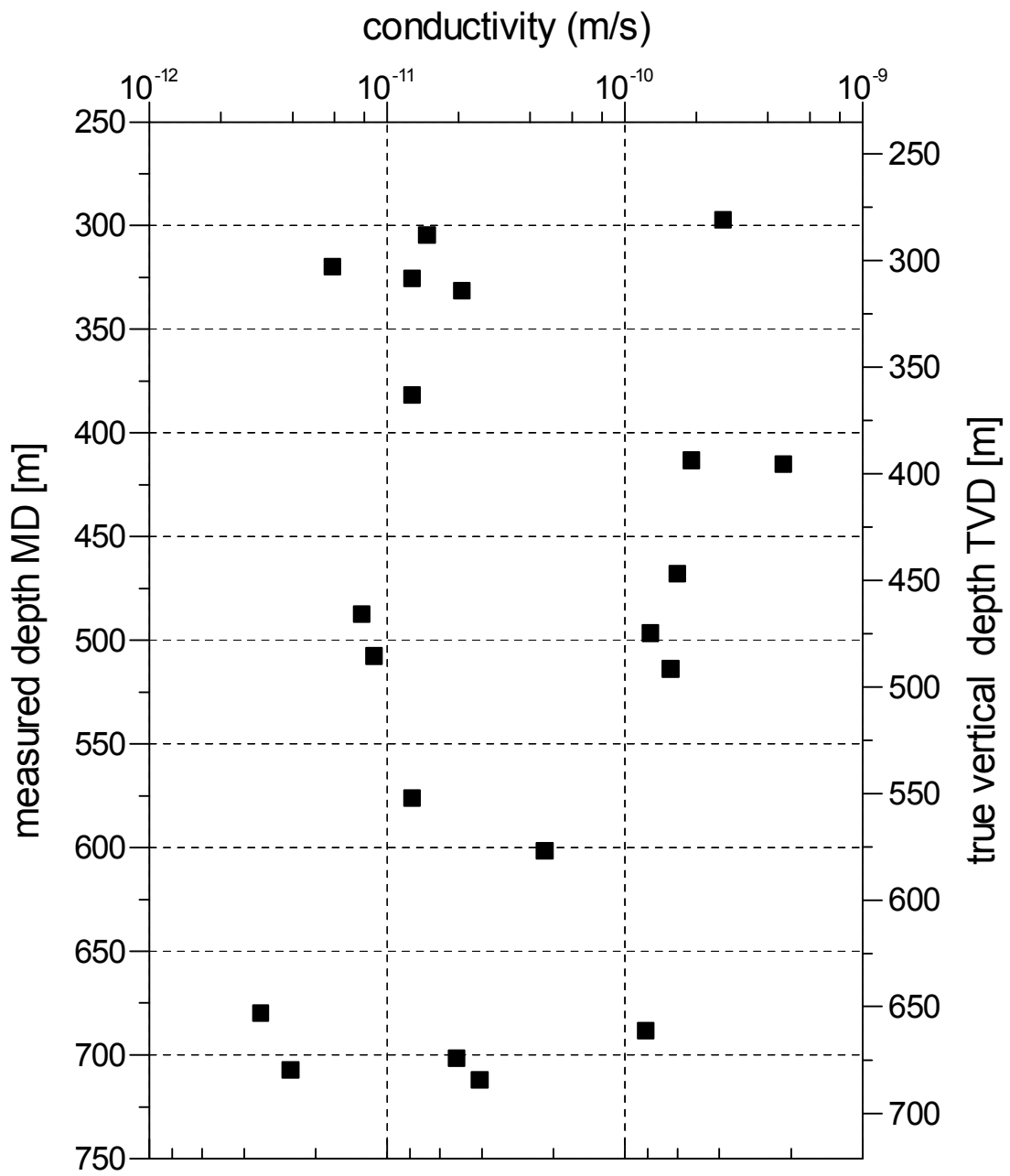


Figure 4-5. Test section conductivity.



## 5 Data Analysis

### 5.1 In - situ Stress Computation

As shown in chapter 4.1.3 in the majority of tests axial or vertical / sub-vertical fractures with a consistent NW - SE strike direction ( $145^\circ \pm 18^\circ$ ) were initiated or stimulated. Therefore, in-situ stresses can be calculated with the "classical" Hubbert & Willis (1957) concept (see Section 3.1):

- $S_v = \rho \cdot g \cdot z$
- $S_h = P_{si}$
- $S_H = 3 S_h - P_r$

The vertical stress  $S_v$  is calculated assuming a constant density for the overburden rock of  $\rho = 2.7 \text{ g/cm}^3$ . The relation  $S_h = P_{si}$  is based on the assumption that the induced fractures propagated in a direction perpendicular to the least principal stress  $S_h$  or the most favourable oriented pre-existing fractures were stimulated and reopened. In the estimate of the major horizontal stress  $S_H$  the pore pressure is neglected with respect to the low porosity / permeability of crystalline rocks.

The calculation is carried out using the Refrac- and Shut-in pressure data  $P_r$  and  $P_{si}$  derived by the routine analysis shown in APPENDIX A and given in Table 4.1. The stress data are listed in Table 5.1 together with the azimuths of the axial fractures as given in Table 4.2. Note, that we assumed the axial fracture was the "active" fracture in the cases where more than one fracture trace was observed, and that the minimum horizontal stress  $S_h$  for the inclined fractures in the test interval at 280.9, 288.0 and 314.2 m TVD were derived by consideration of the dip angle  $\alpha$  of the fractures. The stress data are shown graphically in Figure 5.1.

Due to the large variations in  $P_{si}$  - data and the narrow range for the fracture azimuths the stress calculation by an inversion method does not lead to any meaningful result. Thus, at present, the significant scatter in stress must be accepted, although a more homogeneous stress profile is expected for the homogeneous rock mass in the Oskarshamn - Laxemar - Äspö region.

**Table 5-1. Magnitudes of vertical Stress  $S_v$ , the minor horizontal stress  $S_h$ , the major horizontal stress  $S_H$ , and the direction of  $S_H$ ,  $\theta_{SH}$ .**

TVD (m)	$S_v$ (MPa)	$S_h$ (MPa)	$S_H$ (MPa)	$\theta_{SH}$ , N over E (deg)
280.9	7.4	9.0 <sup>2)</sup>	---	161
288.0	7.6	13.9 <sup>2)</sup>	---	137
302.8	8.0	11.5	21.0	144
308.5	8.2	12.4	23.7	141
314.2	8.3	10.9 <sup>2)</sup>	---	176
363.0	9.6	10.1	17.9	132
393.8	10.4	8.2	13.6	144
446.9	11.8	9.4	16.2	158
465.8	12.3	10.3	19.0 <sup>1)</sup>	102
474.8	12.6	11.1	26.7	158
485.6	12.9	11.5	23.8	168
491.6	13.0	11.6	25.2	161
552.2	14.6	14.7	30.7	179
577.0	15.3	12.1	26.5	119
652.9	17.3	13.7	26.8	147
661.3	17.5	13.7	26.6	124
674.2	17.9	20.7	44.9	133
679.6	18.0	16.7	28.9	127
684.4	18.1	19.7	39.4	133

1) the  $P_r$  - value is derived from the Step-Rate test, 12.5 MPa is used

2)  $S_h$  is calculated with the  $P_{si}$ -value and the value of  $S_v$  for an inclined fracture plane with dip  $\alpha$  and a strike direction of  $\theta_{SH}$  by

$$S_h = \frac{P_{si} - \frac{1}{2} S_v (1 + \cos 2\alpha)}{\frac{1}{2} (1 - \cos 2\alpha)}$$

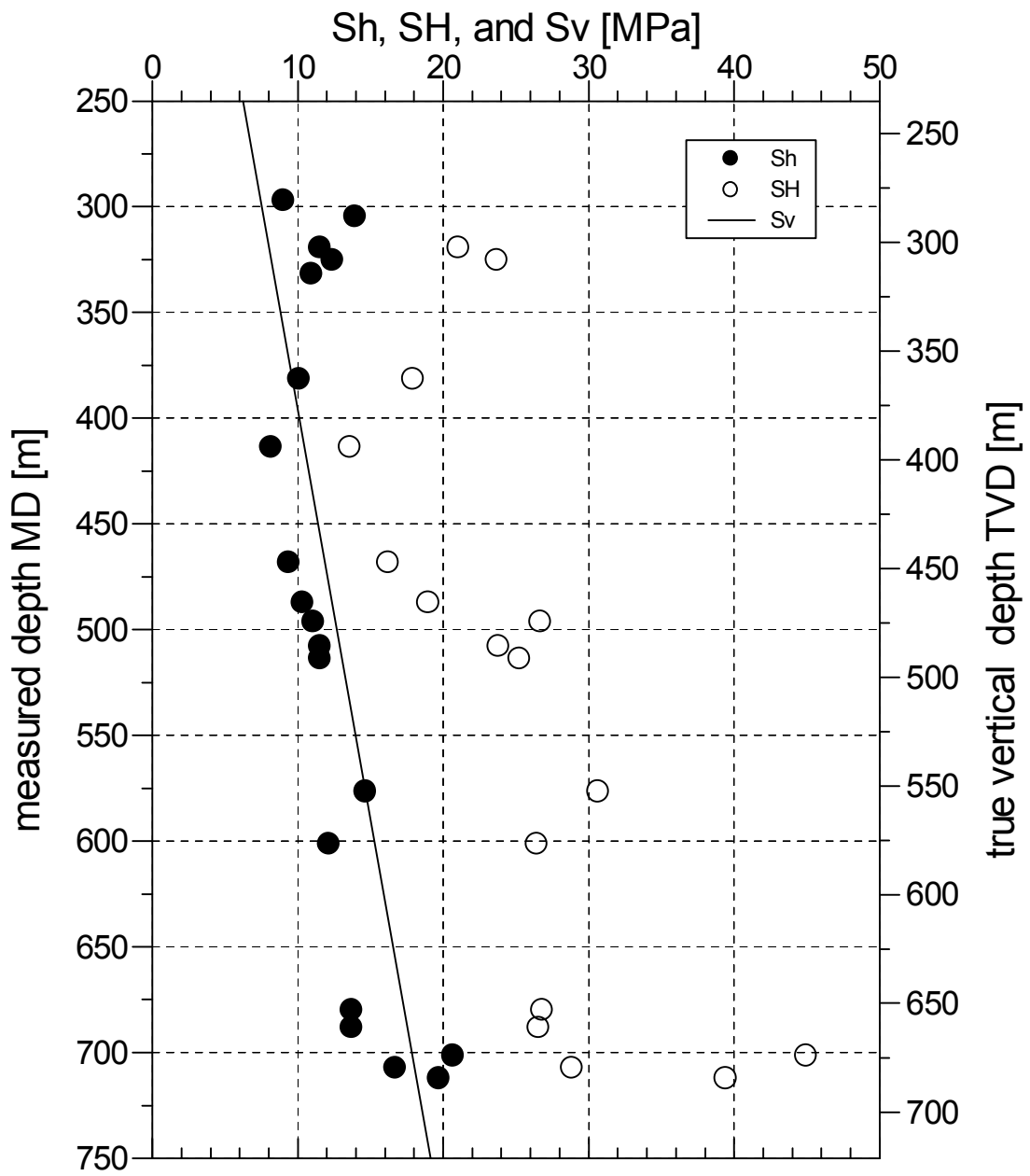


Figure 5-1. Stress Magnitudes  $S_v$ ,  $S_h$  and  $S_H$  in MPa for borehole KOV01.

## 5.2 Quality Control by Laboratory Test Results

### 5.2.1 Core Characterization

The core material was first characterized by density and ultrasonic velocity testing. The rock density values are given in Table 5.2. The rock can be characterized by a mean density value of  $(2.69 \pm 0.023) \text{ g/cm}^3$ . The density test data are given in APPENDIX D.

**Table 5-2. Rock density of the core material.**

core piece - no.	mean depth (m)	density ( $\text{g/cm}^3$ )
1	333.79	$2.705 \pm .018$
2	383.16	$2.690 \pm .031$
3	438.33	$2.680 \pm .015$
4	463.85	$2.700 \pm .008$
5	500.40	$2.694 \pm .010$
G1 - G3	app. 500	$2.66 \pm .04$
6	559.90	$2.722 \pm .027$
7	596.65	$2.709 \pm .012$
8	648.89	$2.701 \pm .010$
9	692.78	$2.691 \pm .014$
10	709.55	$2.637 \pm .017$
<b>mean</b>		<b><math>2.690 \pm .023</math></b>

The ultrasonic test results are given in Table 5.3. The mean data are:

P - wave velocity  $v_p$  : 5.43 km/s ( $\pm 0.18$ )

S - wave velocity  $v_s$  : 3.05 km/s ( $\pm 0.12$ )

Young's modulus E : 64 GPa ( $\pm 4$ )

Poisson's ratio  $\nu$  : 0.27 ( $\pm 0.01$ )

Concerning elasticity the rock can be described as isotropic. It may be of interest that the core from app. 700 m depth shows low velocities and consequently a low Young's modulus ( $E = 55 \text{ GPa}$ ). Otherwise the rock can be described also as homogenous with respect to elasticity. For details see also the Ultrasonic Velocity Data and Elasticity Data Sheets in APPENDIX E and APPENDIX F.

**Table 5-3. Ultrasonic velocities  $v_p$  and  $v_s$  and dynamic elastic parameters Young's modulus E and Poisson's ratio  $\nu$ .**

ultrasonic velocities		radial		axial	
MeSy core-no./ sample-no	depth (m)	$v_p$ (km/s)	$v_s$ (km/s)	$v_p$ (km/s)	$v_s$ (km/s)
1	333.79	5.47±.08	3.14±.07	5.47±.12	2.72±.48
2	383.16	5.41	3.07	5.58±.02	3.15±.08
3	438.33	5.41±.09	3.08±.02	5.44±.08	3.07±.02
4	463.85	5.47±.10	3.11±.01	5.55±.01	3.17±.08
5	500.40	5.31	3.01	5.50±.05	3.12±.06
6	559.90	5.36	2.99	5.42±.11	3.12±.09
7	596.65	5.83	3.05	5.64±.06	3.20±.07
8	648.89	5.26±.09	3.01±.03	5.52±.02	3.16±.02
9	692.78	5.38±.11	3.01±.04	5.44±.12	3.07±.10
10	709.55	5.05±.21	2.86±.16	5.05±.04	2.92±.04
<b>mean</b>		<b>5.40±.19</b>	<b>3.03±.07</b>	<b>5.46±.15</b>	<b>3.07±.14</b>

elastic parameters		radial		axial	
MeSy core-no./ sample-no	depth (m)	$E_{dyn}$ (GPa)	$\nu_{dyn}$	$E_{dyn}$ (GPa)	$\nu_{dyn}$
1	333.79	67±3	0.26±.01	62±4	0.29±.01
2	383.16	64	0.26	68±3	0.27±.02
3	438.33	65±1	0.26±.01	64±1	0.27±.01
4	463.85	66±1	0.26±.01	68±3	0.26±.01
5	500.40	62	0.26	67±2	0.26±.01
6	559.90	62	0.28	66±4	0.26±.01
7	596.65	64	0.26	70±3	0.27±.01
8	648.89	62±1	0.26±.01	68±1	0.26
9	692.78	63±2	0.28±.01	65±3	0.26
10	709.55	55±6	0.27	56±1	0.25
<b>mean</b>		<b>63±3</b>	<b>0.265</b>	<b>65±4</b>	<b>0.265</b>

## 5.2.2 Fracture Toughness

The results of the fracture toughness tests on 20 core samples are given in Table 5.4. The  $K_{IC}$  Test Data Sheets are given in APPENDIX G. The tests yield a mean fracture toughness  $K_{IC} = (2.54 \pm 0.27) \text{ MN/m}^{3/2}$ .

**Table 5-4. Fracture toughness  $K_{IC}$  of the core material.**

sample no.	mean depth (m)	diameter D (mm)	Chevron notch length $a_o$ (mm)	$F_{max}$ (N)	$K_{IC}$ ( $\text{MN/m}^{3/2}$ )
1A1-2	333.79	47.6	7.0	2927	2.90
1A2-3	333.79	47.6	7.0	2547	2.53
1B1-2	333.79	47.6	7.0	2617	2.60
2A1-2	383.16	47.4	6.9	2736	2.73
2A2-3	383.16	47.4	6.0	3023	2.83
4A1-2	463.85	47.4	6.8	2700	2.67
4B1-2	463.85	47.4	7.2	2665	2.71
5A1-2	500.40	51.7	7.3	2672	2.30
5A2-3	500.40	51.7	7.1	2698	2.30
G1 <sup>1)</sup>	app. 500				2.18
G2 <sup>1)</sup>	app. 500				2.37
G3 <sup>1)</sup>	app. 500				2.41
6A1-2	596.65	47.2	7.0	2228	2.25
6A2-3	596.65	47.2	7.0	2900	2.93
7A1-2	578.16	47.2	7.1	1866	1.90
7A2-3	578.16	47.2	7.3	2585	2.67
8B1-2	648.89	47.3	7.2	2500	2.55
8B2-3	648.89	47.3	6.6	2416	2.37
9A1-2	692.78	51.9	7.0	2789	2.34
10A1-2	709.55	51.9	5.2	3385	2.53
<b>mean</b>					<b>2.54±.27</b>

<sup>1)</sup> no Test Data Sheet exists in App. G (preliminary test sample)

### 5.2.3 Hydrofrac Tests on Minicores

The results of the hydrofrac tests on core pieces of 47 mm diameter or on mini-plugs of 30 mm diameter are given in Table 5.5 and are shown graphically in Figure 5.2. The corresponding Hydrofrac Data sheets are given in APPENDIX H.

The breakdown pressure  $p_c$  as a function of confining pressure  $p_m$  can be described by a linear relation

$$p_c = p_{co} + k \cdot p_m$$

$$\text{with } p_{co} = (29.4 \pm 2) \text{ MPa}$$

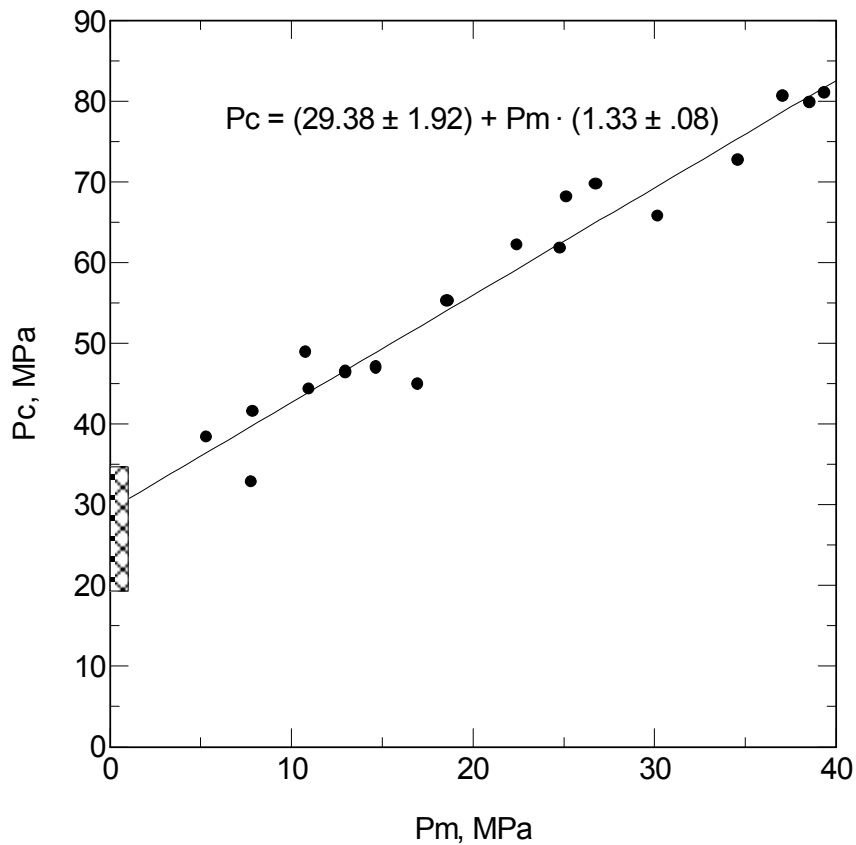
$$k = 1.33 \pm 0.08.$$

The hydraulic tensile strength  $p_{co} = 29.4$  MPa derived from samples subjected to confining pressure is confirmed by the unconfined test on sample G2B ( $p_{co} = 34.7$  MPa, 3 mm ID, 1 ml/s). Unconfined tests on samples with a larger injection hole diameter (8 mm ID) and conducted at lower injection rates (0.5 / 0.1 ml/s) yield a significantly lower hydraulic tensile strength value. This indicates the size and injection rate dependence of hydraulic strength.

**Table 5-5. Hydrofrac tests on mini-cores ( $\sigma_1$ : axial stress,  $p_m$ : confining pressure,  $p_c$ : breakdown pressure)**

sample - no.	$\sigma_1$ (MPa)	$p_m$ (MPa)	$p_c$ (MPa)	remark
1A2a	22.5	18.6	55.3	3 mm ID / 1 ml/s
1A2b	66.6	39.4	81.1	dto.
2A3a	32.4	30.2	65.8	dto.
2A3b	21.8	10.8	48.9	dto.
3Ca	32.6	11.0	44.4	dto.
3Cb	61.4	34.6	72.7	dto.
4B1a	53.3	24.8	61.8	3 mm ID / 1 ml/s
4B1b	20.1	14.7	47.0	dto.
5A1a	11.0	5.3	38.4	dto.
5A1b	67.6	38.6	79.8	dto.
6A1a	32.5	25.2	68.2	dto.
6A1b	36.2	22.4	62.2	dto.
7A1a	30.1	26.8	69.7	dto.
7A1b	67.6	37.1	80.6	dto.
8B2a	12.7	7.9	41.6	dto.
9A1a	17.9	13.0	46.4	dto.
9A1b	10.6	7.8	32.8	dto.
10A1a	38.4	17.0	45.0	dto.
G1A <sup>1)</sup>	8.6	0	19.3	8 mm ID / 0.1 ml/s
G1B <sup>1)</sup>	7.9	0	20.4	dto.
G2A <sup>1)</sup>	8.0	0	27.2	8 mm ID / 0.5 ml/s
G2B <sup>1)</sup>	9.0	0	34.7	3 mm ID / 1 ml/s
G3A <sup>1)</sup>	4.6	0	24.6	3 mm ID / 0.1 ml/s
G3B <sup>1)</sup>	5.3	0	24.4	3 mm ID / 0.1 ml/s

<sup>1)</sup> no Hydrofrac Data Sheet exists in App. H (preliminary test sample)



**Figure 5-2.** Breakdown pressure  $p_c$  as a function of confining pressure  $p_m$  for hydrofrac tests on mini-cores.

#### 5.2.4 Application of Laboratory Test Results

The results of the laboratory tests on the core material from borehole KOV01 can be summarized as follows :

- rock density  $\rho$ ,  $\text{g/cm}^3$  :  $2.69 \pm 0.023$

The mean rock density of  $2.7 \text{ g/cm}^3$  was used to estimate the overburden stress which is considered as a principal stress.

- Ultrasonic velocities  $v_p$  and  $v_s$  :

$v_p$ , radial, km/s :  $5.40 \pm 0.19$

$v_p$ , axial, km/s :  $5.46 \pm 0.15$

mean :  $5.43 \pm 0.18$

$v_s$ , radial, km/s :  $3.03 \pm 0.07$

$v_s$ , axial, km/s :  $3.07 \pm 0.14$

mean :  $3.05 \pm 0.12$



The ultrasonic velocity measurements served both, to characterize the rock with respect to homogeneity and isotropy, and to determine rock elasticity parameters. Considering the small scatter, the rock can be characterized as extremely homogeneous and isotropic with respect to elasticity.

- elasticity parameters Young's modulus  $E$  and Poisson's ratio  $\nu$ :

The dynamic elasticity parameters  $E_{\text{dyn}}$  and  $\nu_{\text{dyn}}$  were calculated by using the ultrasonic velocity data and the rock density data by the following relations :

$$E_{\text{dyn}} = 2 v_s^2 \cdot \rho (1 + \nu_{\text{dyn}})$$

$$\nu_{\text{dyn}} = \frac{1}{2} \cdot \frac{2 - (v_p / v_s)^2}{1 - (v_p / v_s)^2}$$

The following mean values were obtained and were subsequently used in the hydrofrac simulation calculations in Section 5.2.4.3:

$$E_{\text{dyn}}, \text{ GPa} : \quad 64 \pm 4$$

$$\nu_{\text{dyn}} : \quad 0.27 \pm 0.01$$

- fracture toughness  $K_{\text{IC}}$ ,  $\text{MN/m}^{3/2}$  :

The fracture toughness  $K_{\text{IC}}$  ranges between 1.9 and 2.9  $\text{MN/m}^{3/2}$  with a mean value of  $(2.54 \pm 0.27) \text{MN/m}^{3/2}$ , which is used in the fracture mechanics hydrofrac simulation calculation of Section 5.2.4.3. The values are rather high when compared to other granite rocks (Atkinson & Meredith, 1987).

- tensile strength  $p_{\text{co}}$  and hydrofrac coefficient  $k$  :

$$p_{\text{co}}, \text{ MPa} : \quad 29.4 \pm 2$$

$$k : \quad 1.33 \pm 0.08$$

The mean hydrofrac tensile strength of the rock is high when compared to other granites (e.g. Rummel 1987). Similarly, the value of  $k = 1.33$  is high when compared to other granites (e.g.  $k = 1.04$  for the coarse grained Falkenberg granite, Rummel 1987), but is much smaller than  $k = 2$  for a flawless material. Thus, the  $k$  value observed which only depends on the crack size is an expression for the importance of a fracture mechanics analysis of hydraulic fracturing tests in crystalline rock as already indicated at the end of Section 3.1 and therefore such an approach can be considered as a necessary quality control particularly for hydrofrac tests in so-called "intact" rock sections - (again we refer to Rummel 1987 or Rummel 2001).

#### 5.2.4.1 Comparison of In - Situ and Laboratory Tensile Rock Strength

Following a fracture mechanics approach to hydraulic fracturing (Rummel, 1987) a simple scaling relation between the in-situ and laboratory rock strength  $P_{\text{co}}$  and  $p_{\text{co}}$  exists :

$$P_{\text{co}} = p_{\text{co}} \sqrt{r/R}$$

where R and r are the radii of the injection boreholes for the in-situ and laboratory tests, respectively. Using the laboratory results of tests in 3 mm and 8 mm diameter boreholes, the in-situ tensile strength for tests in 76 mm diameter boreholes can thus easily be estimated:

r or R (mm)	1.5	4	38
p <sub>co</sub> or P <sub>co</sub> (MPa)	29.4	≈ 20	5.8

The in-situ measured hydraulic tensile strength was (5.2 ± 3) MPa and thus is in good agreement with the estimated strength value of 5.8 MPa. The result will not vary dramatically if we also consider that the laboratory tests were conducted with oil (viscosity: 25 cPoise) while the in-situ tests were conducted with water (1cPoise) as injection fluid. The injection rates of 1 ml/s (laboratory) and some l/min (in-situ) were appropriate with respect of the system stiffness.

#### 5.2.4.2 Estimation of Crack Size

Again following Rummel (1987) the characteristic size for the in-situ rock at "intact" test intervals and for laboratory test specimens, the crack length A<sub>o</sub> and a<sub>o</sub>, may be estimated using the fracture mechanics definition of hydraulic tensile strength:

$$\text{laboratory : } p_{co} = \frac{K_{IC}}{(h_o + h_a) \cdot \sqrt{r}}$$

$$\text{in-situ : } P_{co} = \frac{K_{IC}}{(H_o + H_A) \cdot \sqrt{R}}$$

The functions h<sub>o</sub>, h<sub>a</sub>, H<sub>o</sub> and H<sub>A</sub> are known dimensionless stress intensity factors which only depend on the crack size (crack length a and A, respectively) and on the fluid pressure distribution within the cracks (h<sub>a</sub>, H<sub>A</sub>) (for reference see Rummel, 1987).

Using above relations and the measured values of K<sub>IC</sub>, p<sub>co</sub>, P<sub>co</sub>, r and R, the values of (h<sub>o</sub> + h<sub>a</sub>) and (H<sub>o</sub> + H<sub>A</sub>) are obtained which then yield a<sub>o</sub> and A<sub>o</sub>. The calculation shows the following intrinsic microcrack sizes for fluid filled cracks:

laboratory rock sample : a<sub>o</sub> ≈ 1 mm

in-situ rock : A<sub>o</sub> ≈ 30 mm.

Considering the granitic rock type and the experimental procedure (low injection rates) this result can be accepted particularly for the TBM boring damaged zone around the borehole. This means that during the in-situ hydrofrac tests such microcracks were first saturated and pressurized before they became unstable and rapidly propagated during the initial frac-cycle.

#### 5.2.4.3 Hydrofrac Simulation Calculation

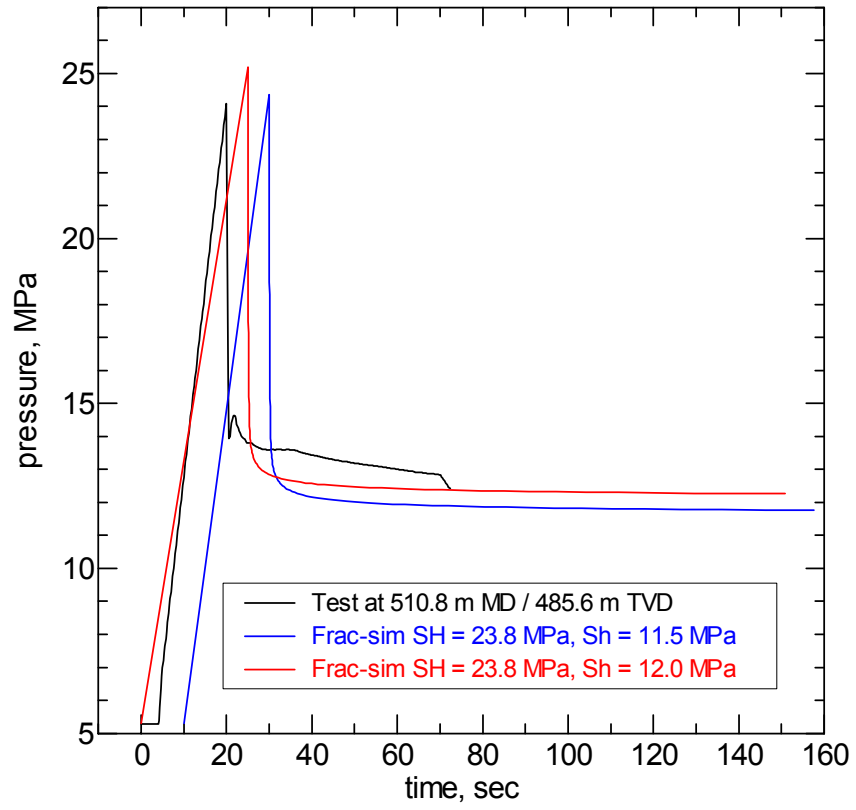
Various analytical hydrofrac simulation models on the basis of the fracture mechanics and fluid dynamics were developed by MeSy (e.g. Rummel & Hansen 1989, te Kamp 1991). The models are used for sensitivity analysis of input parameters and to match simulated hydrofrac records with in-situ measured hydrofrac test records. The present analysis is conducted with the MeSy code HSI MU which contains the following input variables:

- borehole and test interval dimensions
- hydraulic system stiffness
- fluid viscosity and fluid injection rate
- rock elasticity and fracture toughness
- initial micro-crack size (length)
- principle stresses
- pressure loss at the fracture inlet.

Here only one simulation calculation was carried out as an example to simulate the hydrofrac test record obtained for the test section at 485.6 m (TVD). The input parameters are given in Table 5.6. The comparison is shown in Figure 5.3 and shows that the calculated pressure record for the frac cycle matches quite well with the frac cycle record observed during the in-situ test.

**Table 5-6. Input parameter for the hydrofrac simulation at test interval at 485.6 m (TVD).**

Number of simulation model: 3		
borehole:	radius	3.8 cm
	length of injection interval	0.7 m
rock:	permeability	1 $\mu$ Darcy
	porosity	0 %
	Youngs's modulus	60 GPa
	Poisson's ratio	0.26
	fracture toughness	2.54 MN/m <sup>3/2</sup>
stresses:	Sigma 1	23.8 MPa
	Sigma 3	11.5 / 12.0 MPa
frac-fluid:	density	1 g/cm <sup>3</sup>
	viscosity	1 cPoise
	injection rate	1.1 lpm
	pressure loss (inlet)	0 %
	pressure loss (fracture)	0.3
crack characteristics:	microcrack length	30 mm
	final crack length	30 m
	number of add. cracks	0
	effectivness of cracks	0



**Figure 5-3.** Comparison of the simulated and measured hydrofrac record at the test section at 485.6 m (TVD).

## 6 Discussion

### 6.1 Rock Stress at Oskarshamn

In the Oskarshamn region hydrofrac stress testing was conducted in the Äspö island boreholes KAS02 (to 737 m) and KAS03 (to 963 m), and in the Laxemar borehole KLX02 to a depth of 1340 m in the mid-nineties (Ljunggren & Klasson 1997). The data are summarized by Ekman (2001) and were recently reinterpreted by Ask et al. (2001).

Unfortunately, MeSy has not access to the original hydrofrac records and the exact stress data from the tests in the Äspö island boreholes. Therefore, it is only possible to compare the stress data derived from the MeSy tests in borehole KOV01 with the stress data in the Laxemar borehole KLX02 given by Ekman (2001, page 88). The result is shown for the depth range up to 1000 m in Figure 6.1. Both test series show a significant scatter in the profile and, in particular, a steep stress gradient in the depth range between 650 and 700 m.

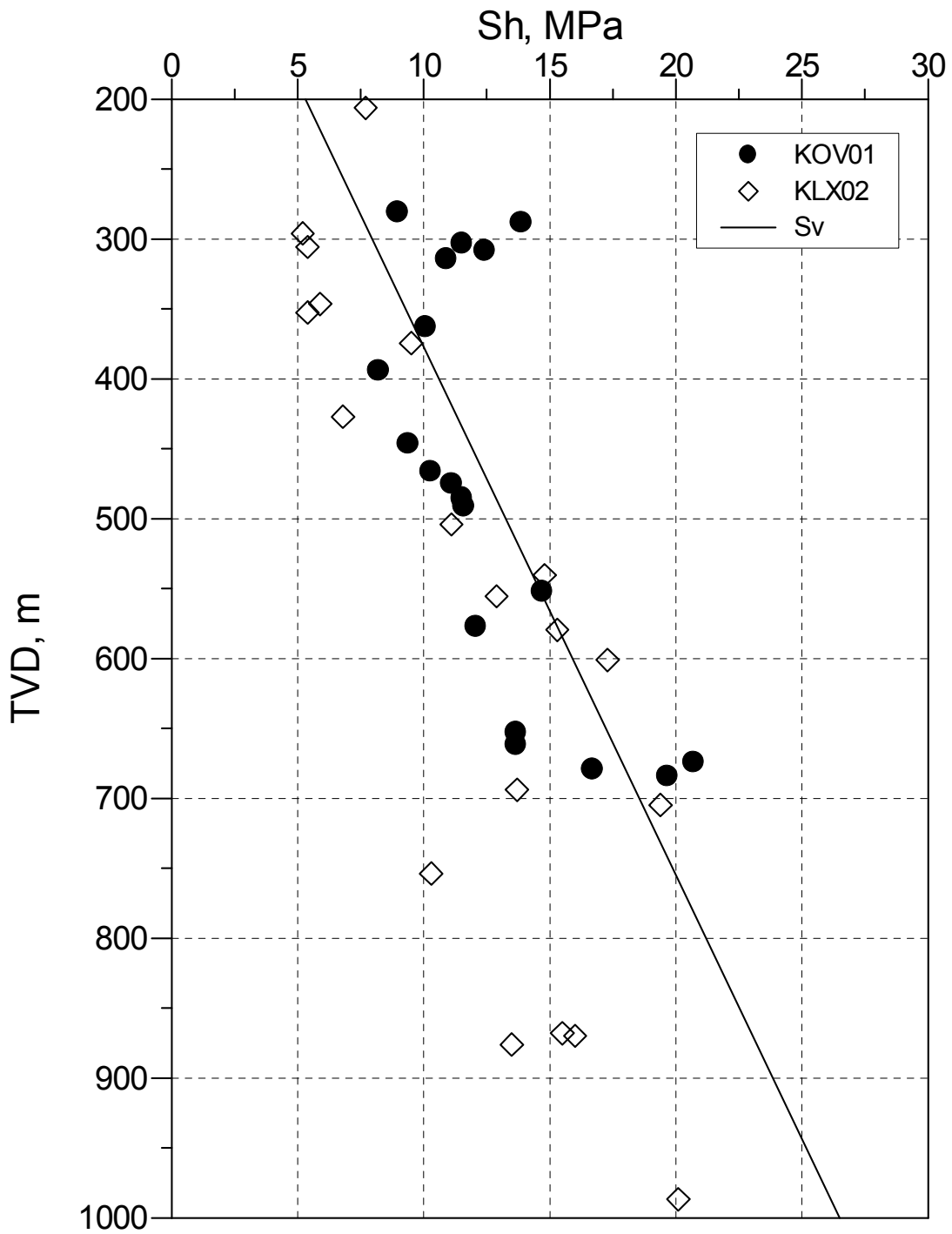
If we compare (Figure 6.2) the azimuths of the major horizontal stress data (strike of induced or stimulated fractures) we observe a similar scatter but a general agreement that the fracture strike varies around  $147 \pm 19$  degrees (if we neglect some odd data) :

$$\begin{aligned}\theta_{SH} &= 145^\circ \pm 18^\circ \text{ for KOV01} \\ \theta_{SH} &= 148^\circ \pm 20^\circ \text{ for KLX02} \\ &\text{with a mean of } \theta_{SH} = 147^\circ \pm 19^\circ.\end{aligned}$$

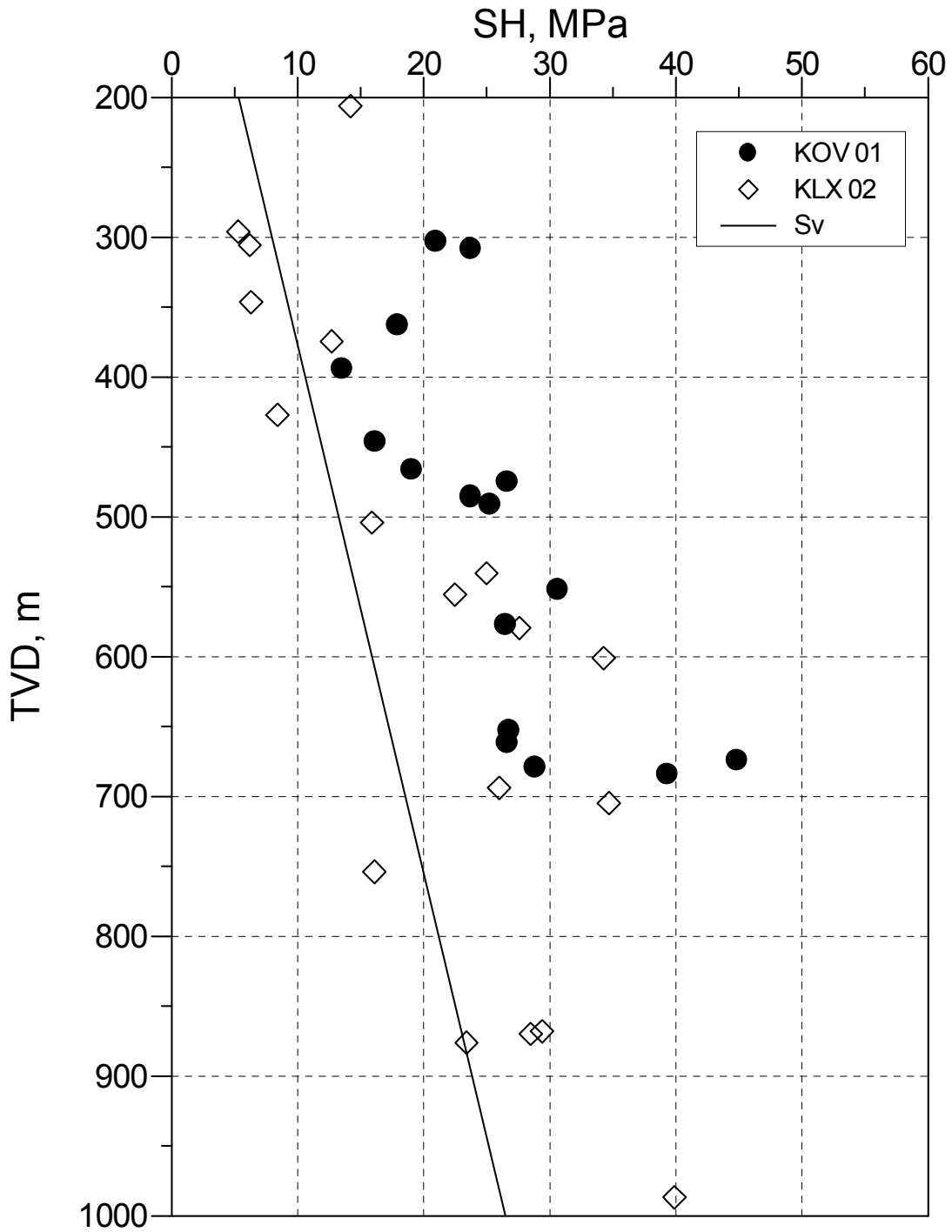
Thus, in summary: If we accept the reasonable agreement, the added information from both boreholes provides an almost gapless stress profile which may be representative for the Oskarshamn - Laxemar - Äspö region. The data from Äspö may further support this assumption.

### 6.2 Hydraulic Rock Mass Behaviour

From the pressure pulse tests we have derived permeability values  $k$  of app.  $1 \mu\text{D}$  ( $10^{-18} \text{ m}^2$ ) for intact test sections, and app.  $25 \mu\text{D}$  ( $25 \cdot 10^{-18} \text{ m}^2$ ) for test sections with pre-existing fractures. Considering water as the injection fluid we obtain hydraulic conductivity values of  $K \approx (1 - 25) \cdot 10^{-11} \text{ m/s}$ . Such values are acceptable for unfractured granites but are three orders of magnitude smaller than the values derived from the pumping tests in 300 m long test sections carried out in borehole KLX02 (Ekman 2001, page 129 ff). The discrepancy can easily be explained by assuming one or a few highly conductive fractures or fracture zones in the granite.



**Figure 6-1a.** Minimum horizontal stress  $S_h$  in borehole KOV01 and borehole KLX02 (Ekman, 2001).



**Figure 6-1b.** Major horizontal stress  $S_H$  in borehole KOV01 and borehole KLX02 (Ekman, 2001).

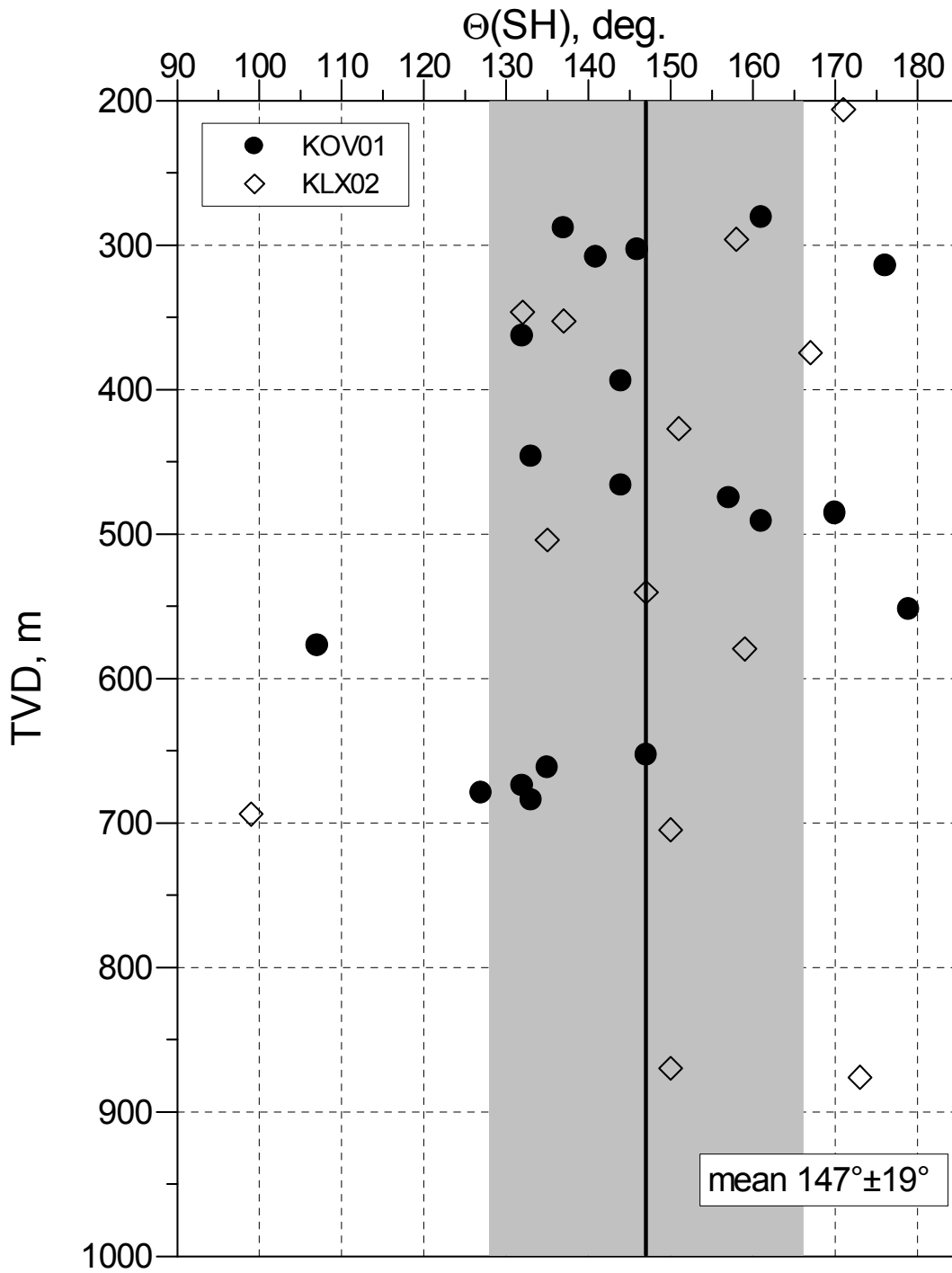


Figure 6-2. Strike direction  $\theta$  or azimuths  $\theta$  for  $S_H$  in boreholes KOV01 and KLX02.



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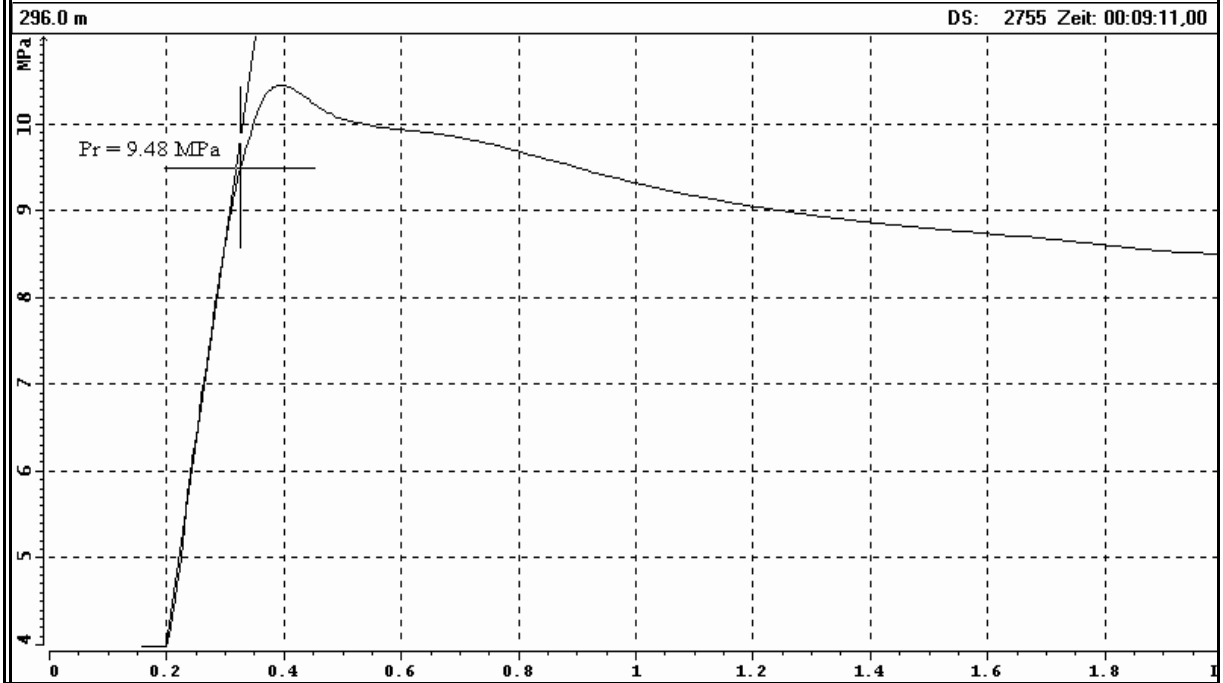
## **APPENDIX A**

**Records from in-situ hydrofrac tests together  
with evaluation of characteristic pressure data**

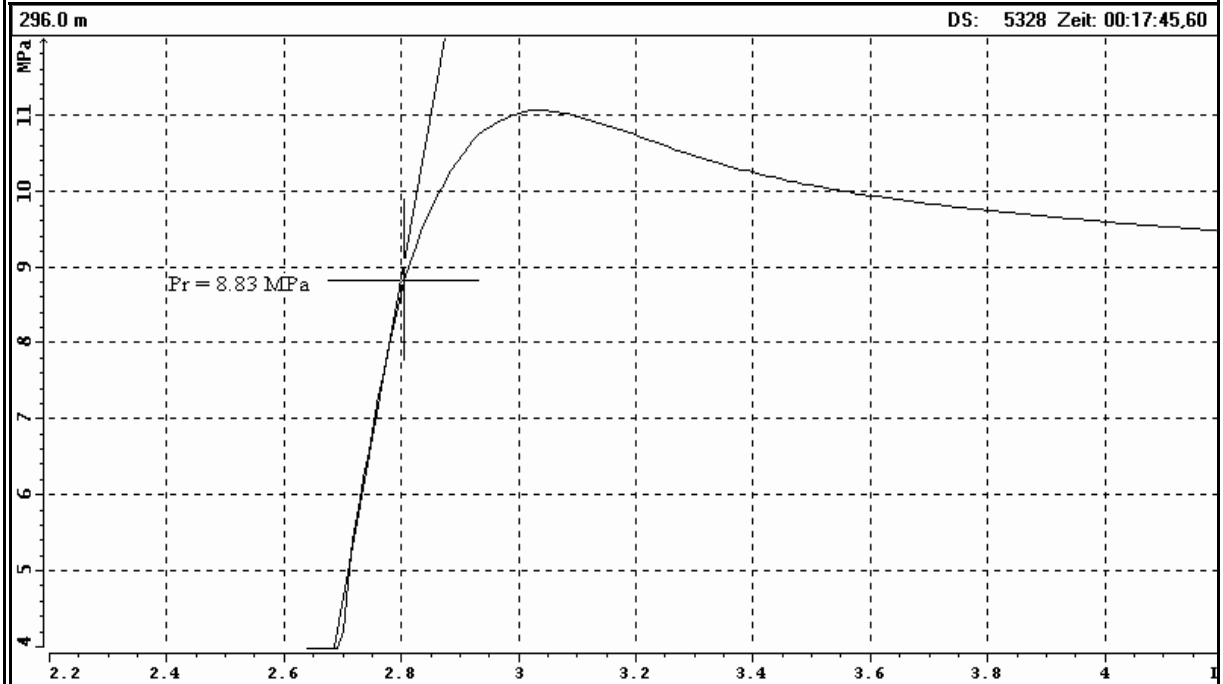




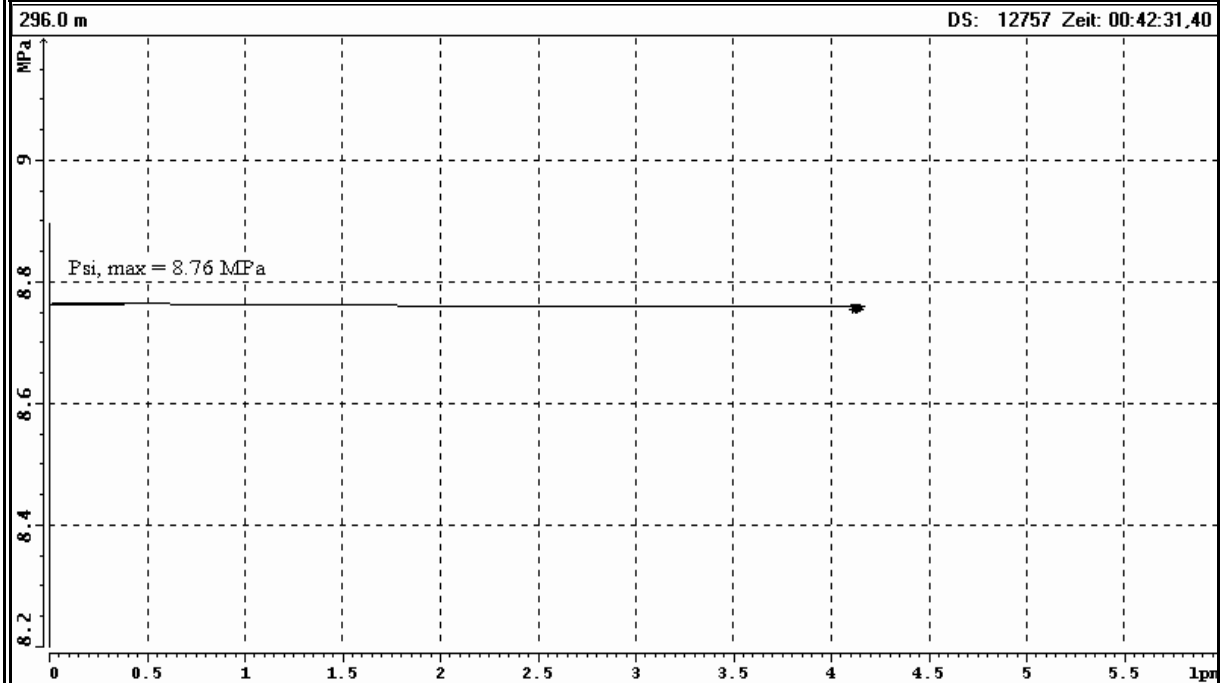
### Test at 296.0 m MD: Estimation of $P_r$ (Frac (1. Refrac) - Cycle)



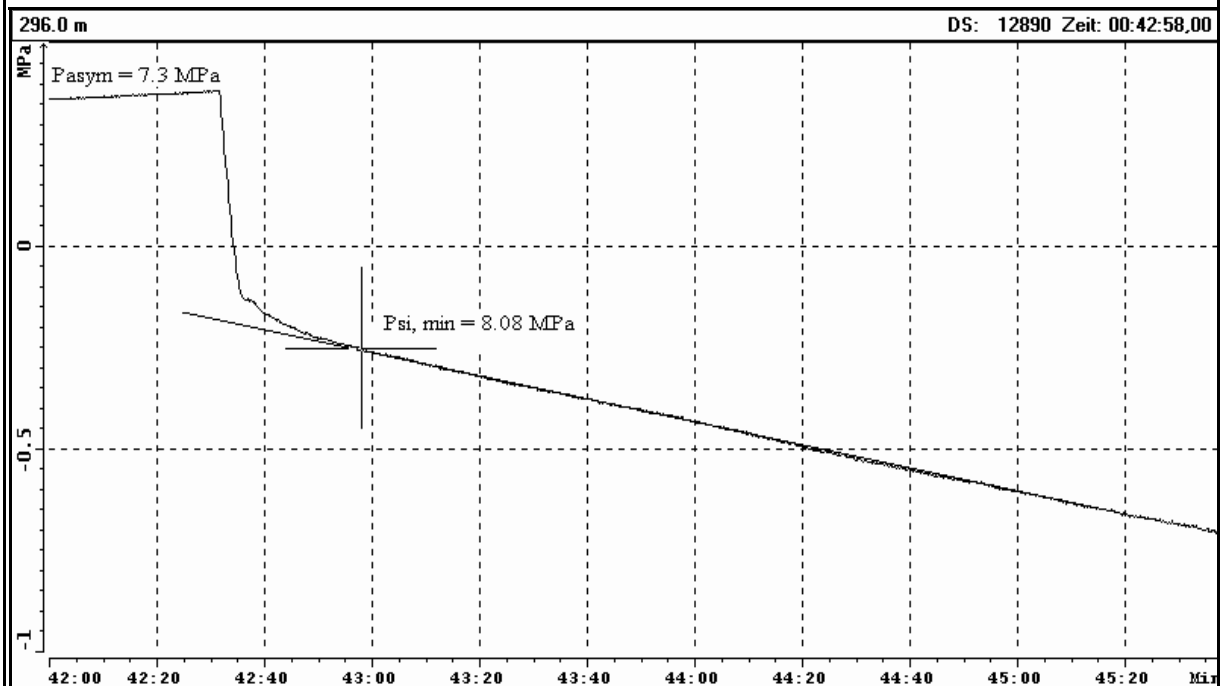
### Test at 296.0 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)



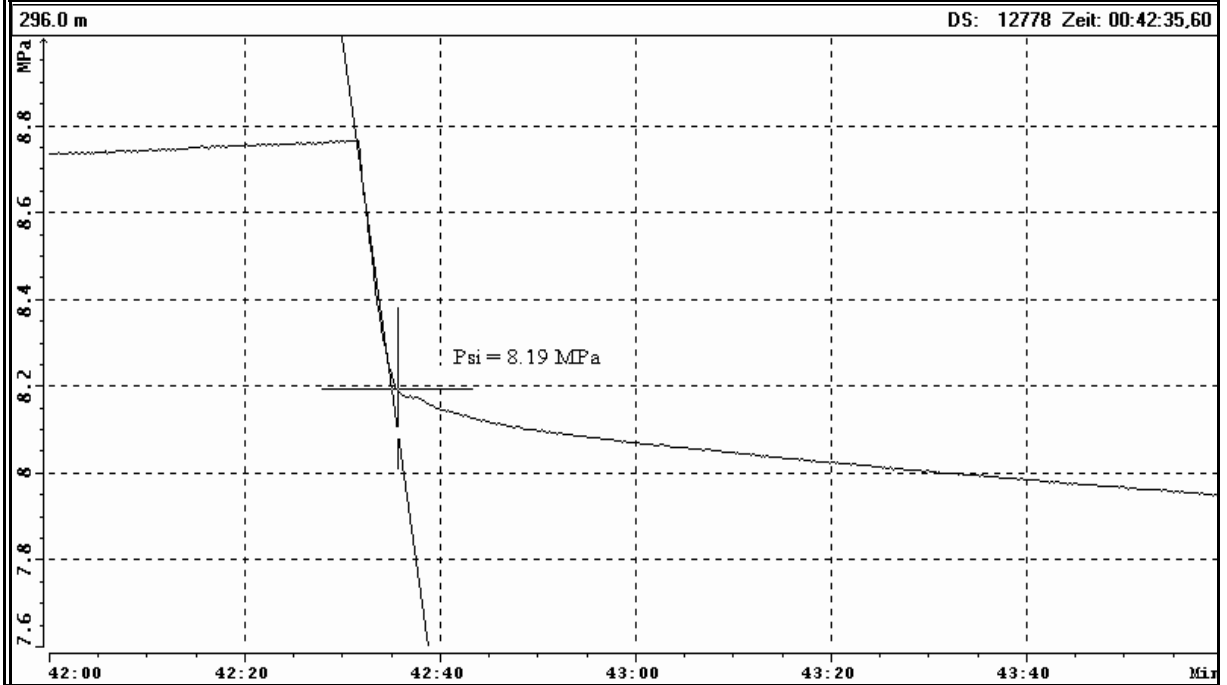
### Test at 296.0 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



### Test at 296.0 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)

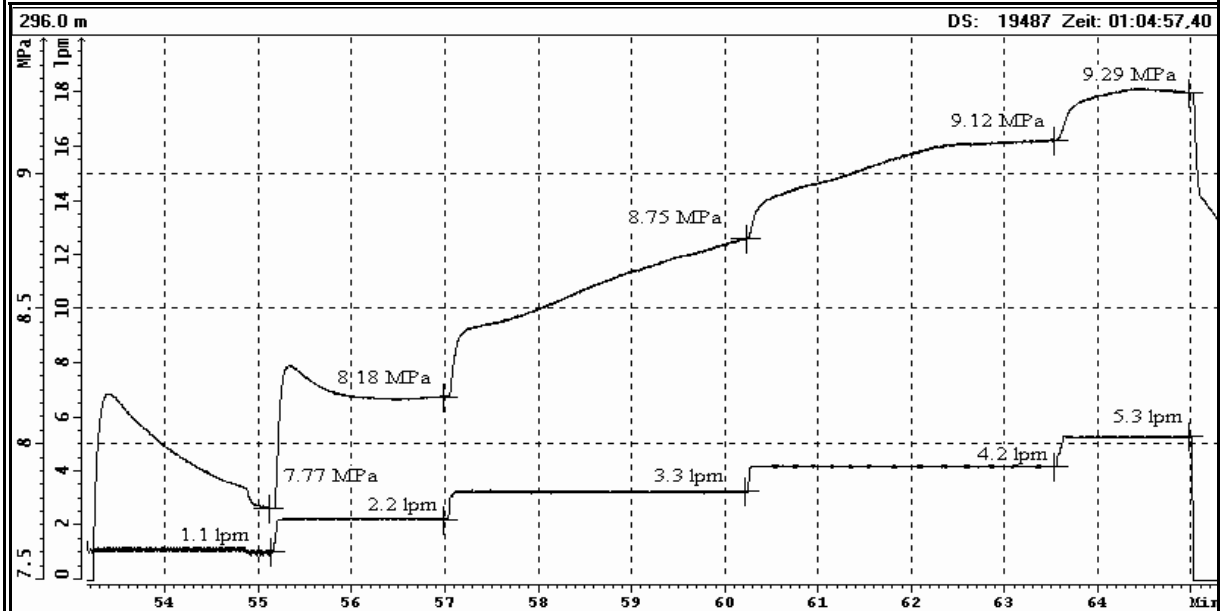


### Test at 296.0 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)

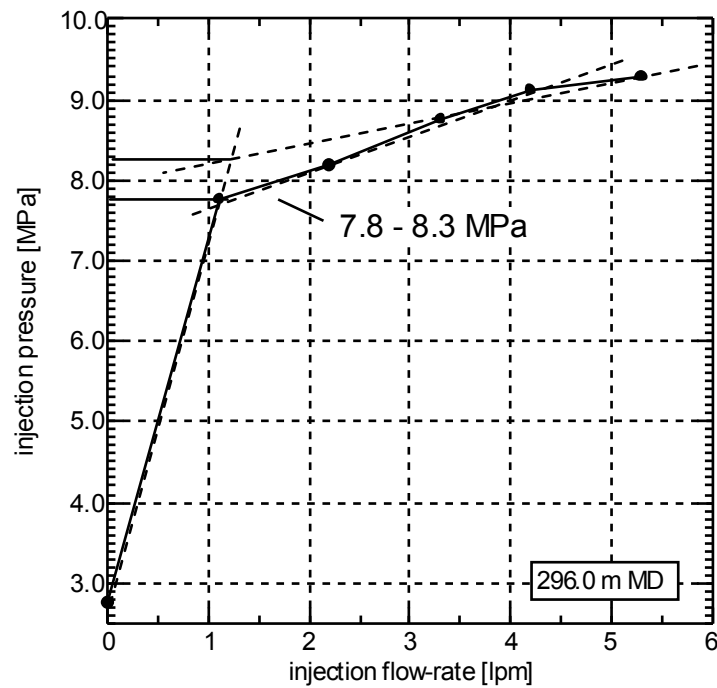




### Test at 296.0 m MD: Analysis of Slow - Pump / Step - Rate - Test

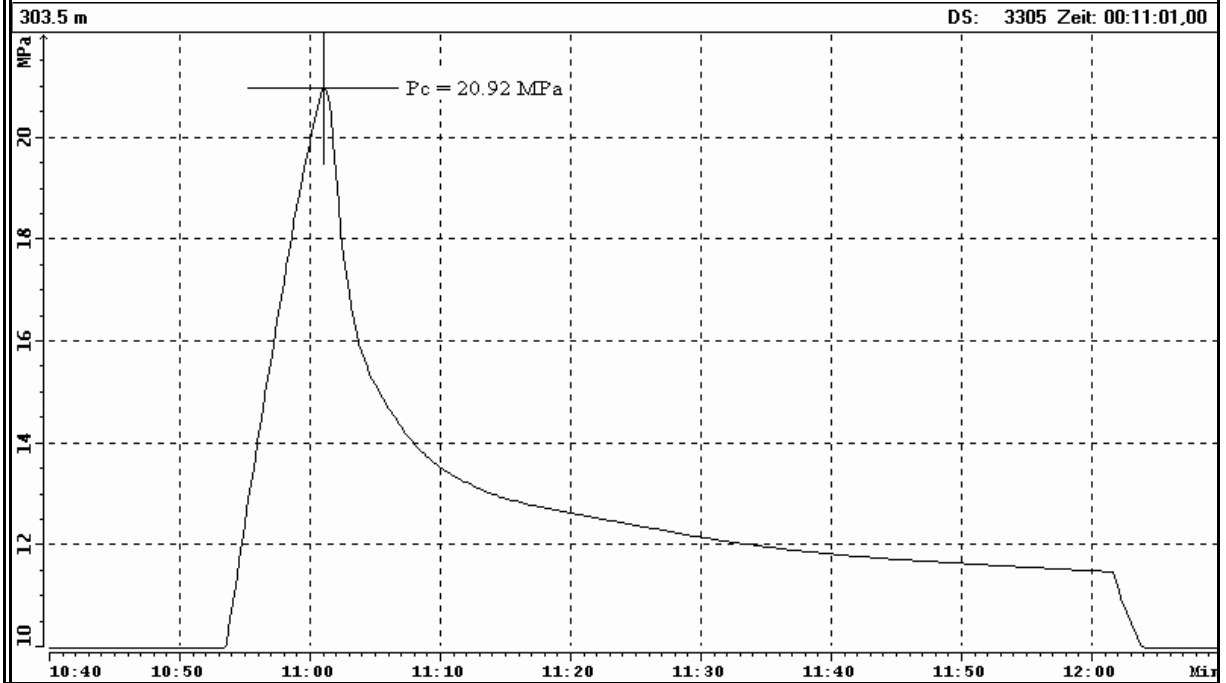


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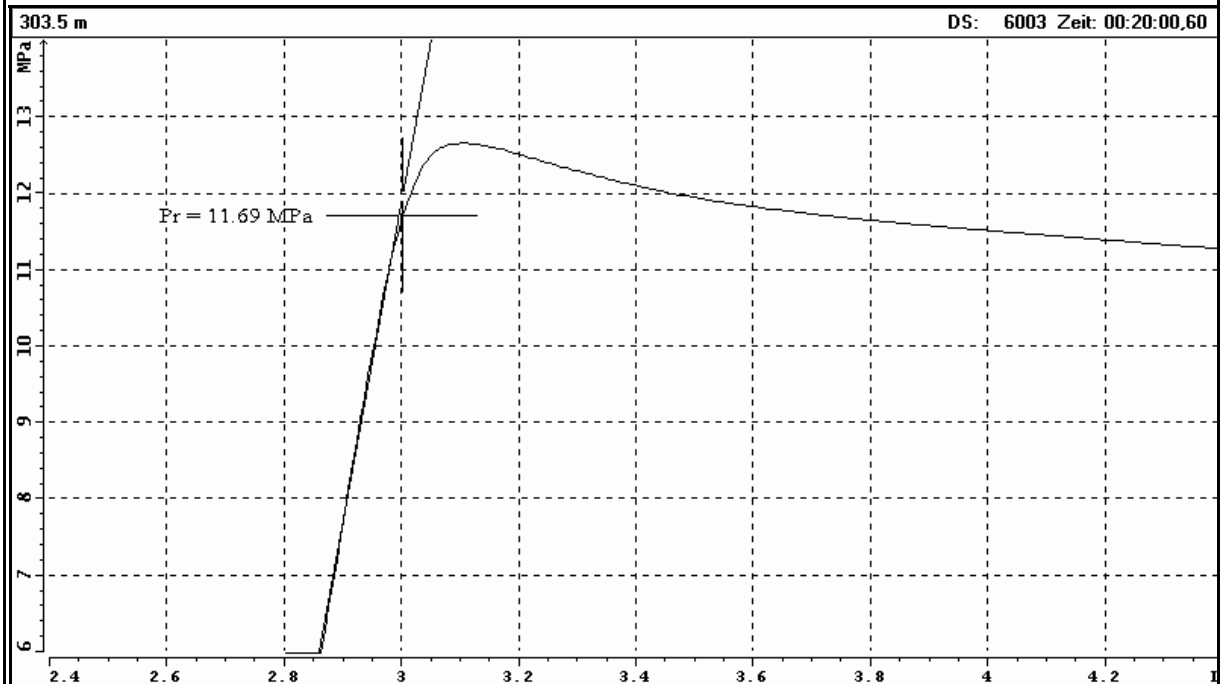




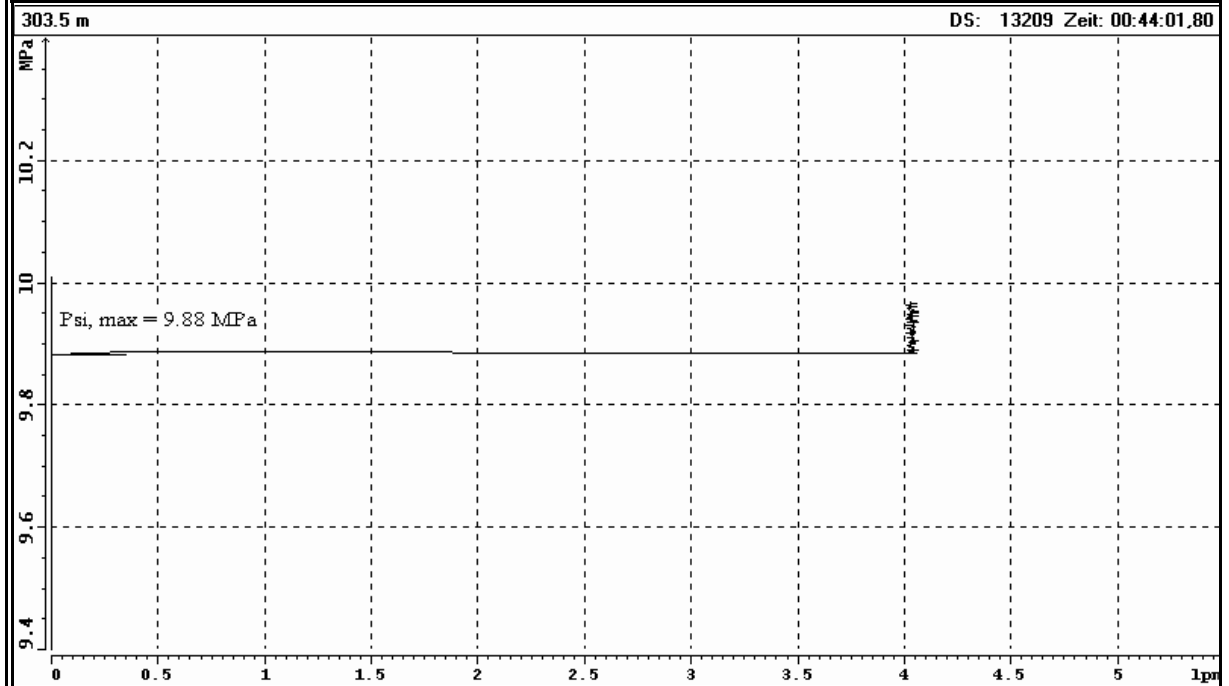
### Test at 303.5 m MD: Estimation of $P_c$ (Frac - Cycle)



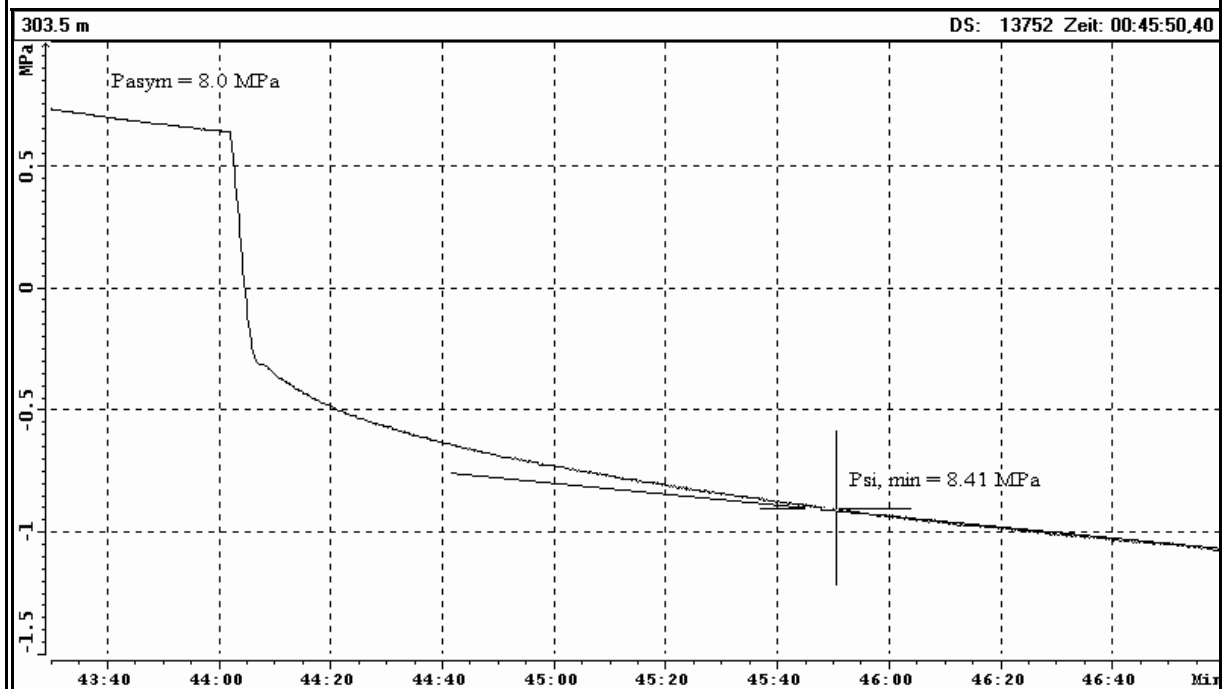
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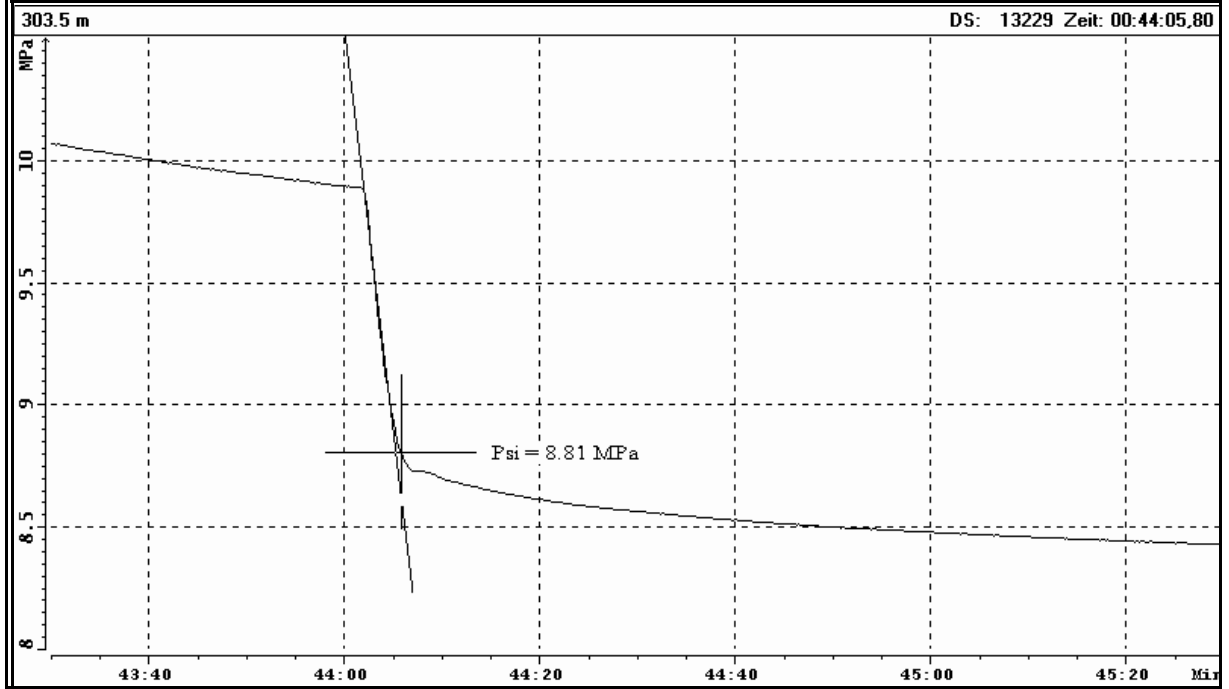
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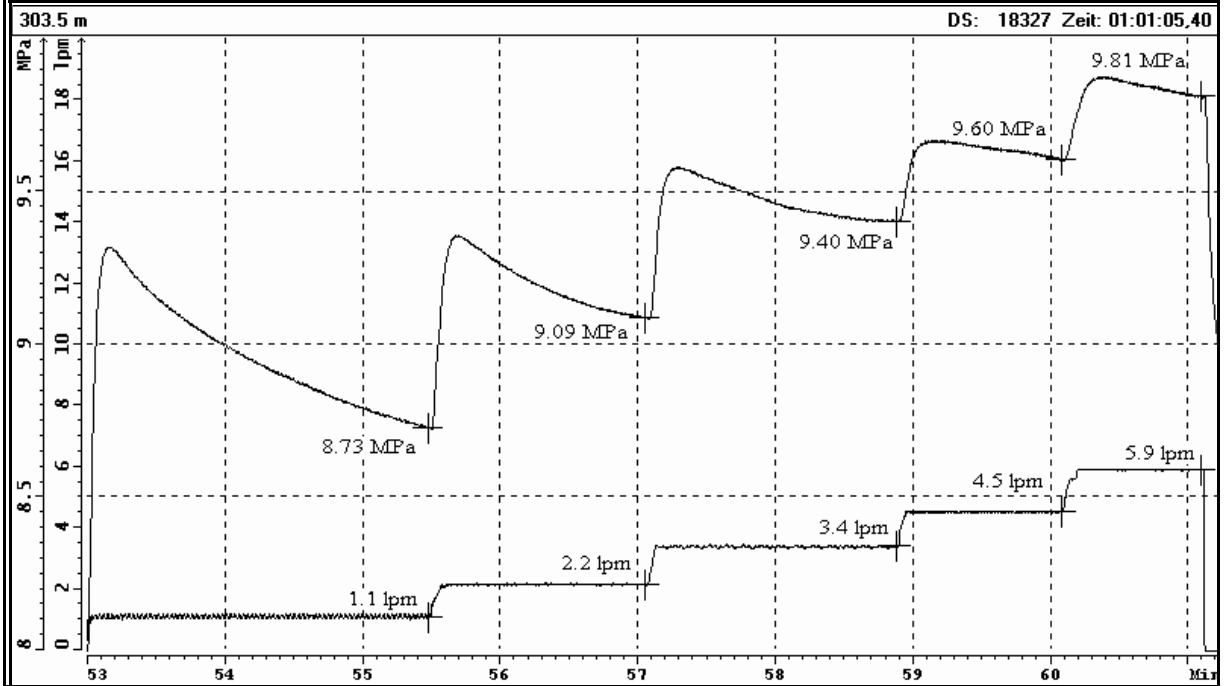
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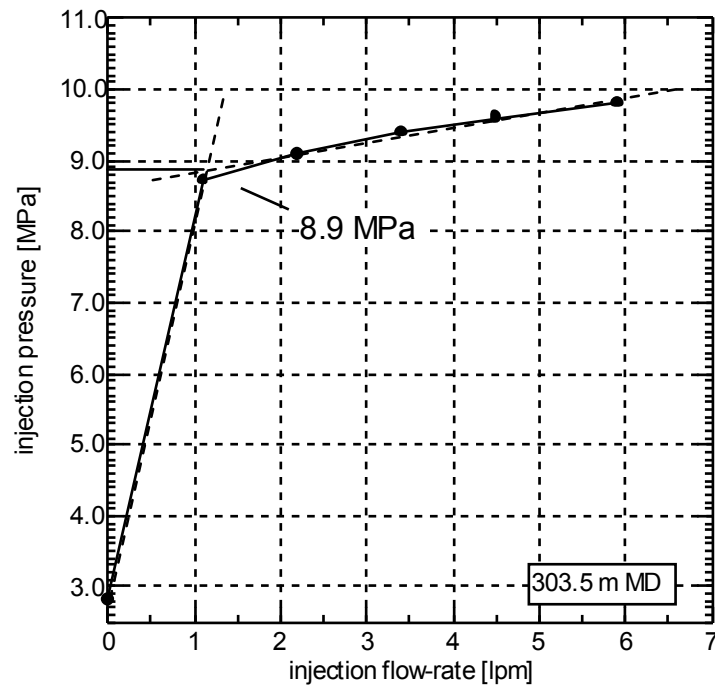
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### Test at 303.5 m MD: Analysis of Slow - Pump / Step - Rate - Test

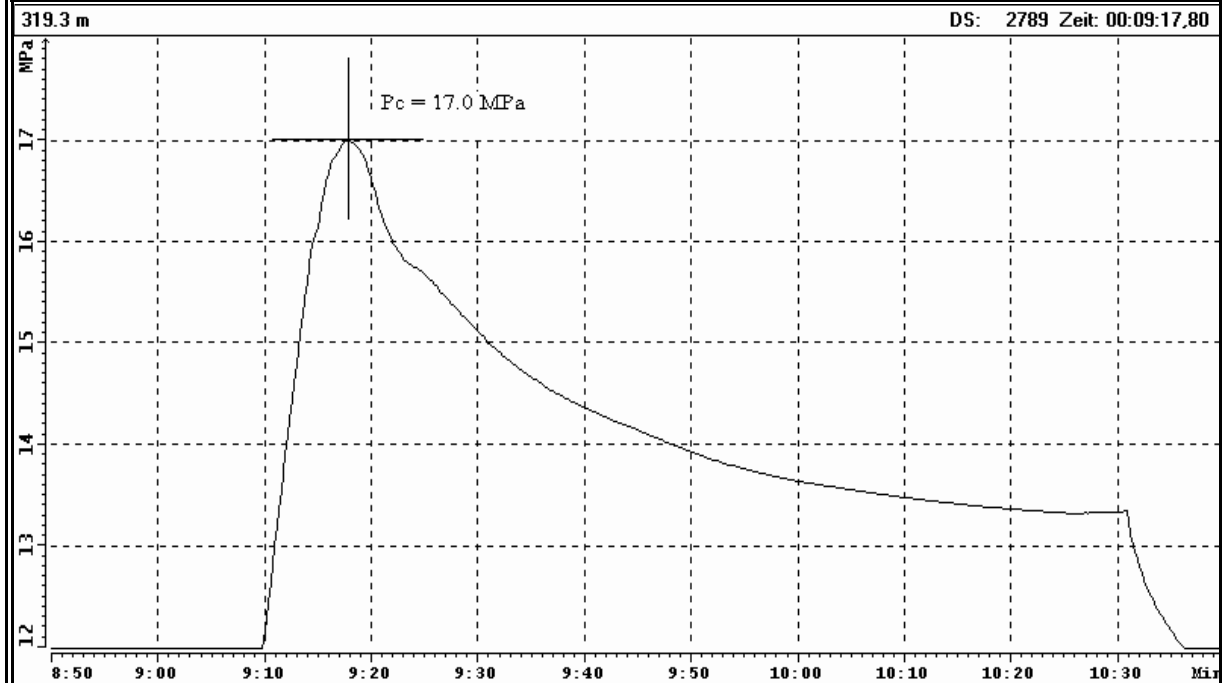


### Test at 303.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)

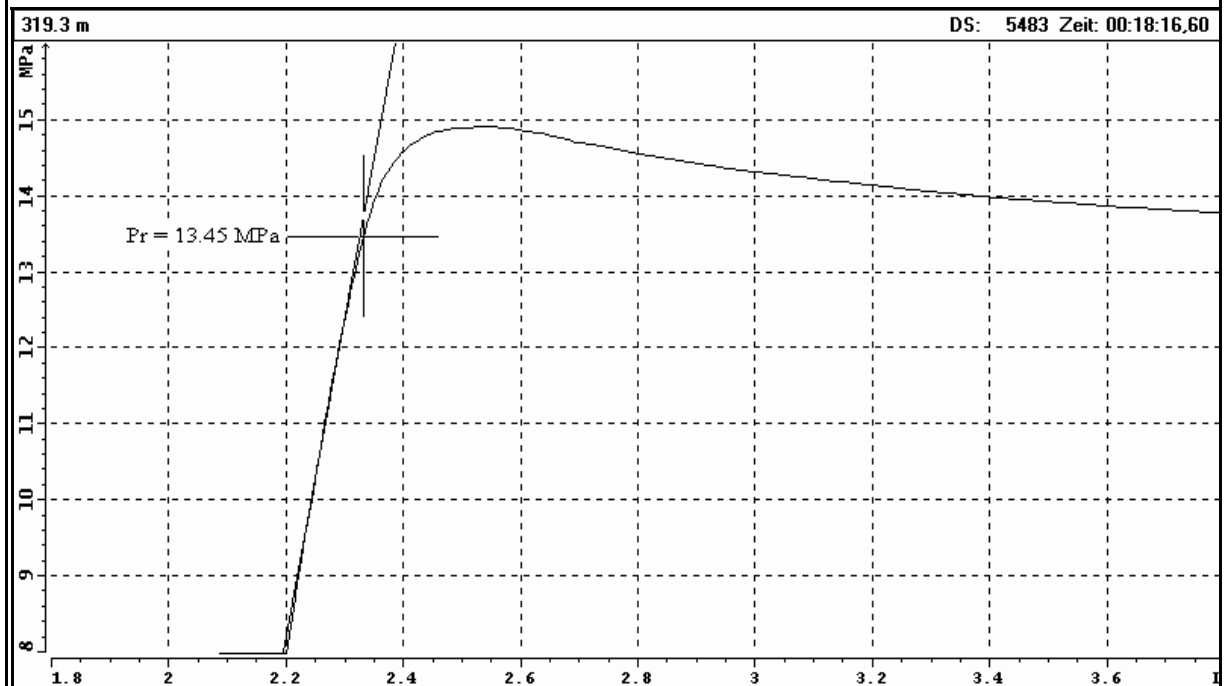




### Test at 319.3 m MD: Estimation of $P_c$ (Frac - Cycle)

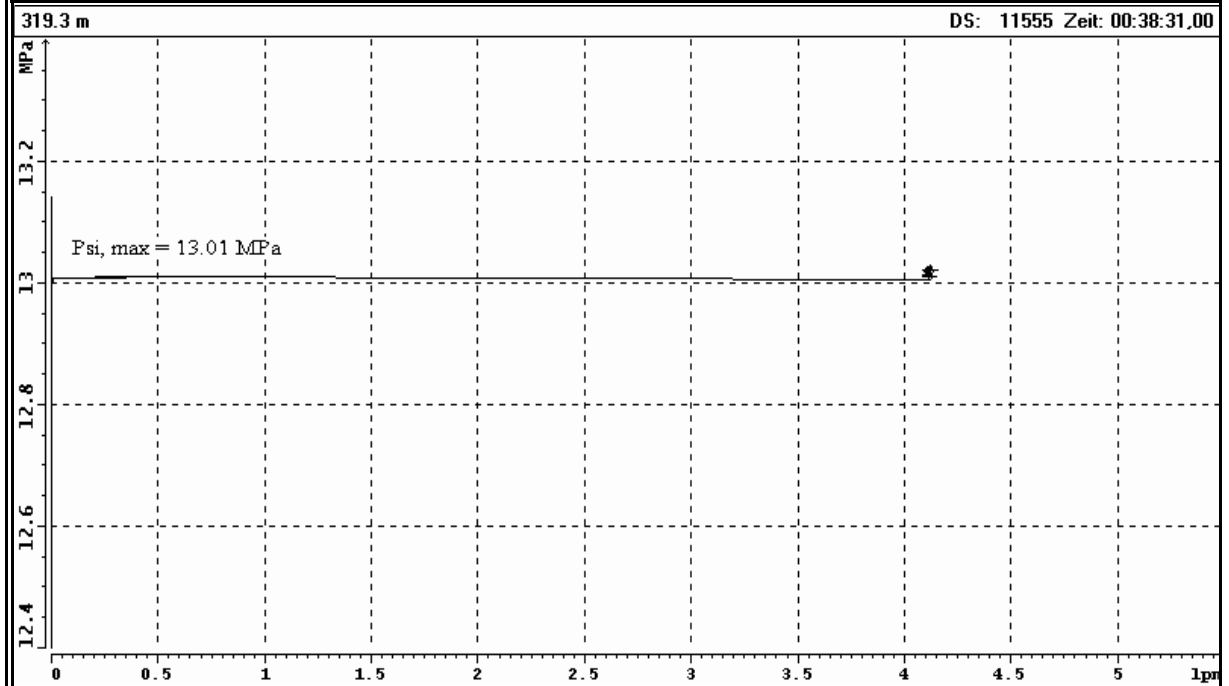


### Test at 319.3 m MD: Estimation of $P_r$ (1. Refrac - Cycle)

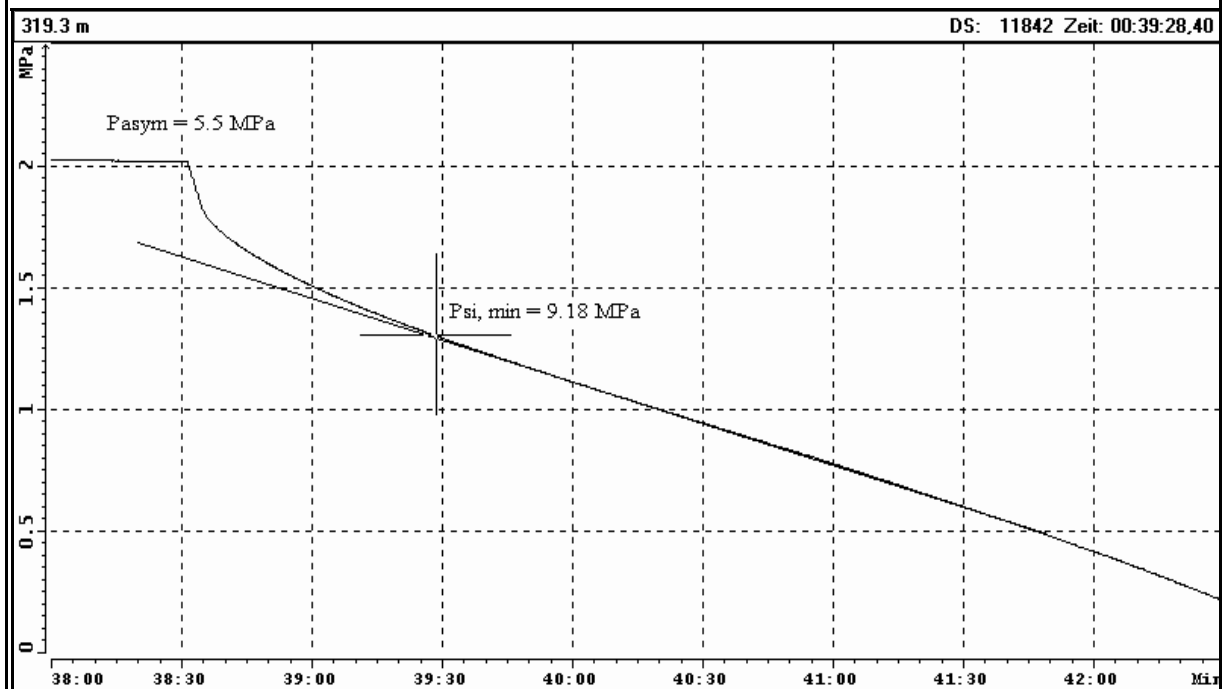




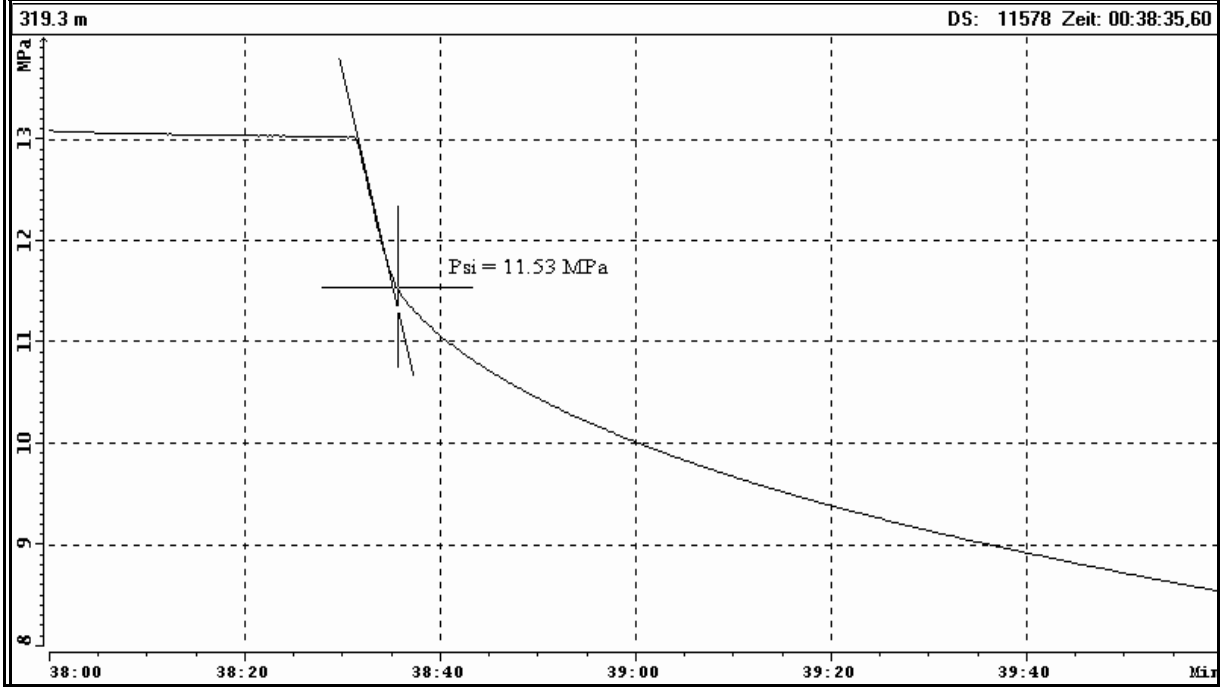
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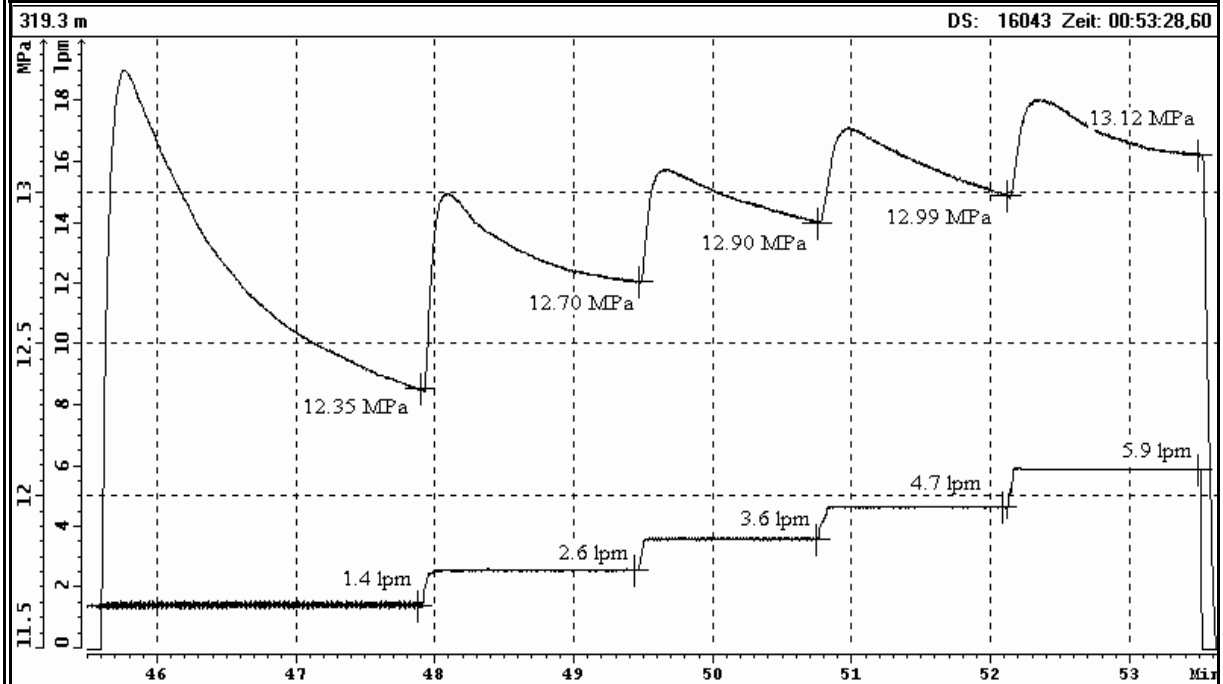
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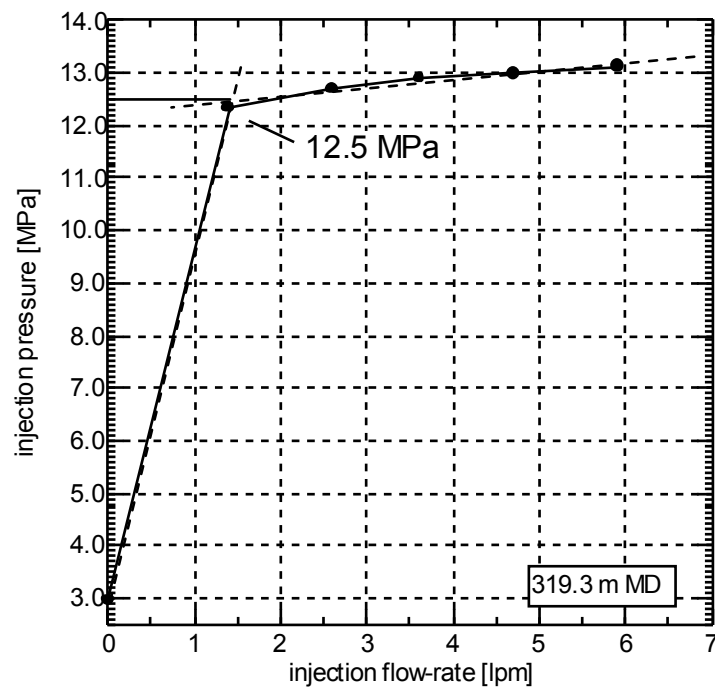
### Test at 319.3 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 319.3 m MD: Analysis of Slow - Pump / Step - Rate - Test

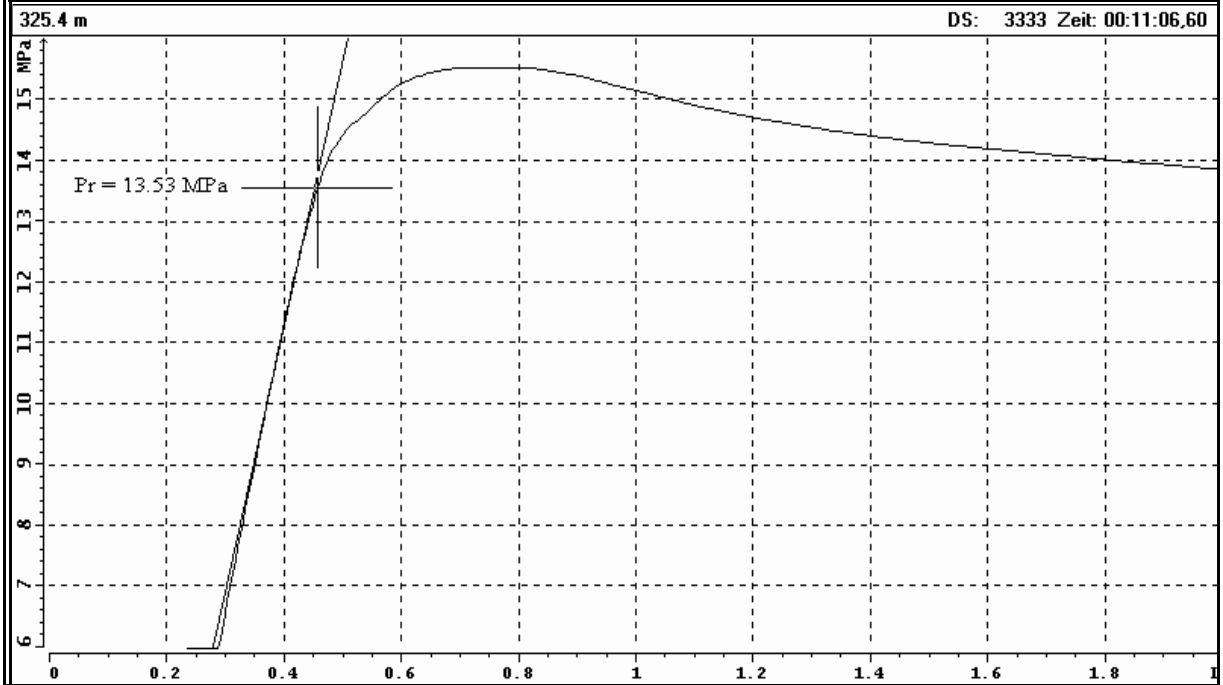


### Test at 319.3 m MD: Examination of $P_{si}$ (Step - Rate - Test)

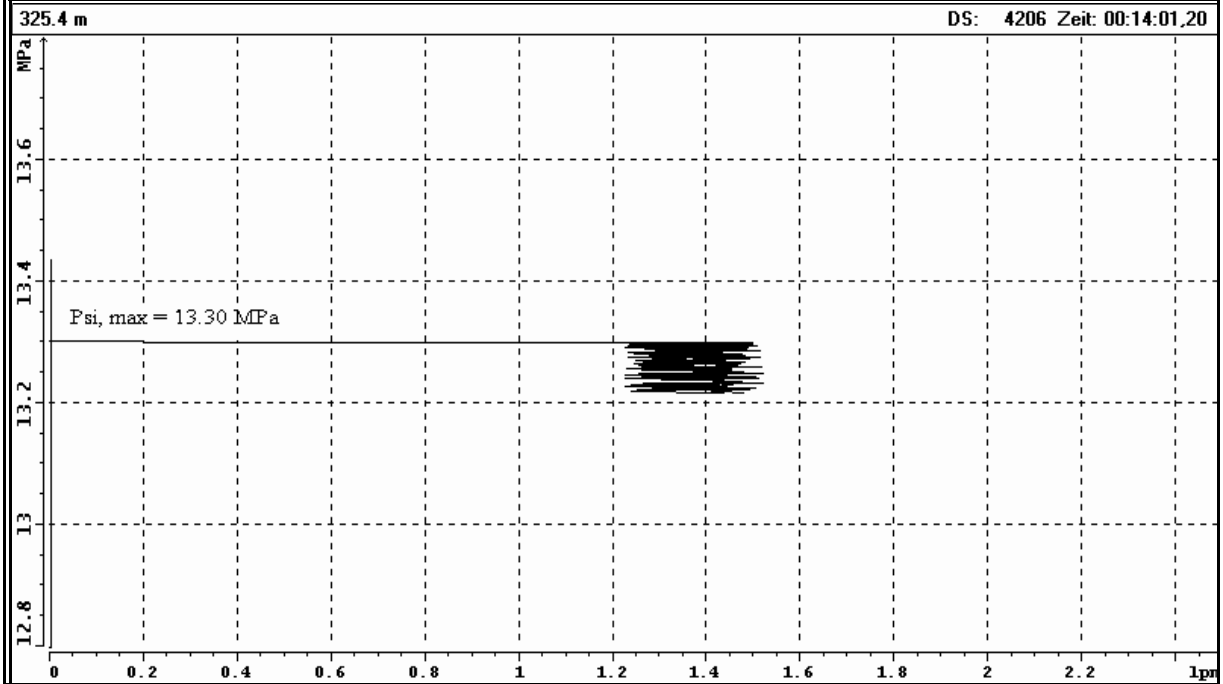




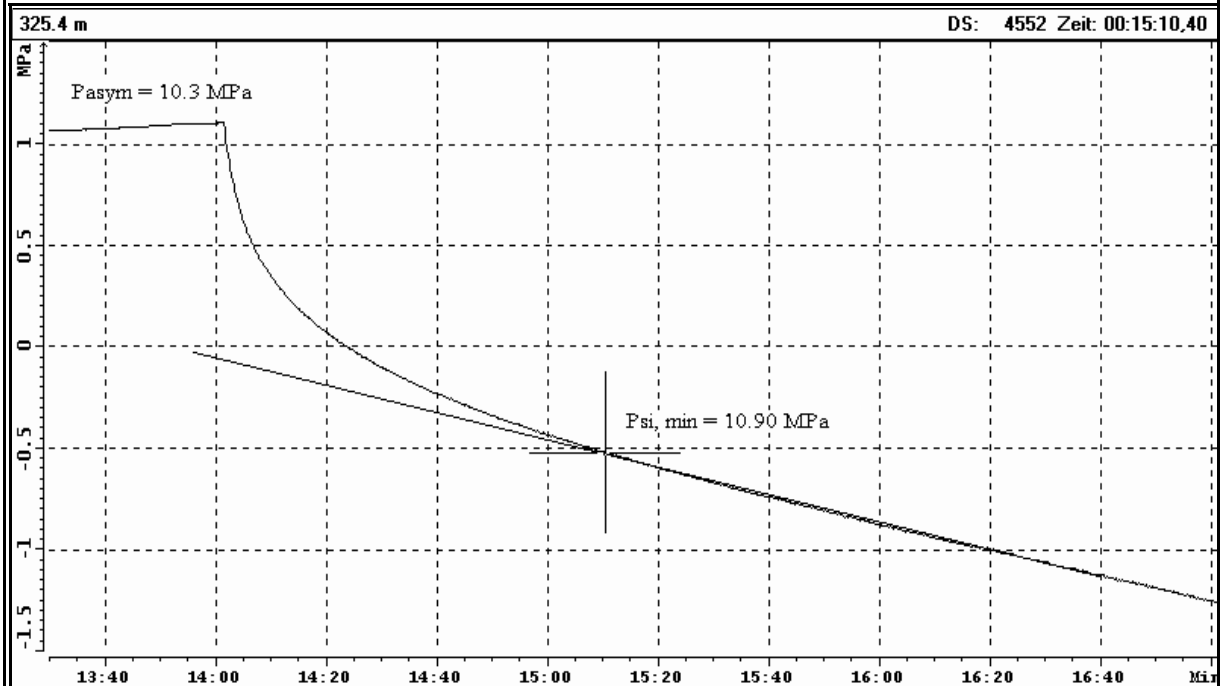
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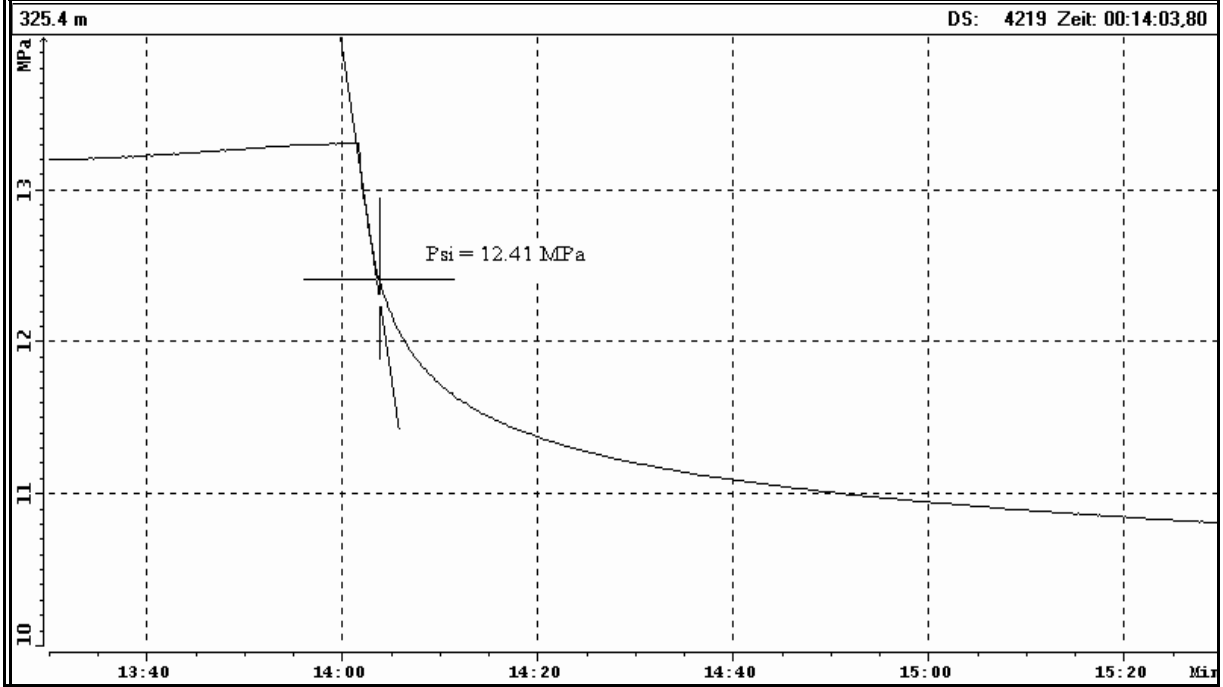
### Test at 325.4 m MD: Estimation of $P_{si, max}$ (1. Refrac - Cycle)



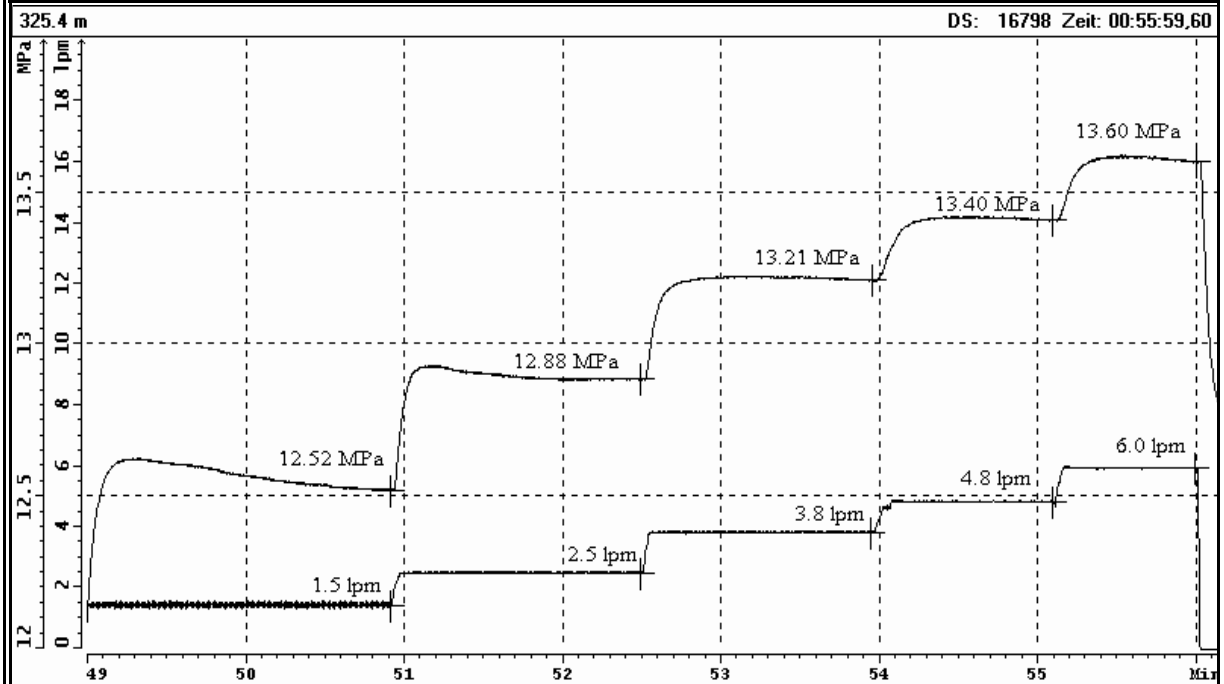
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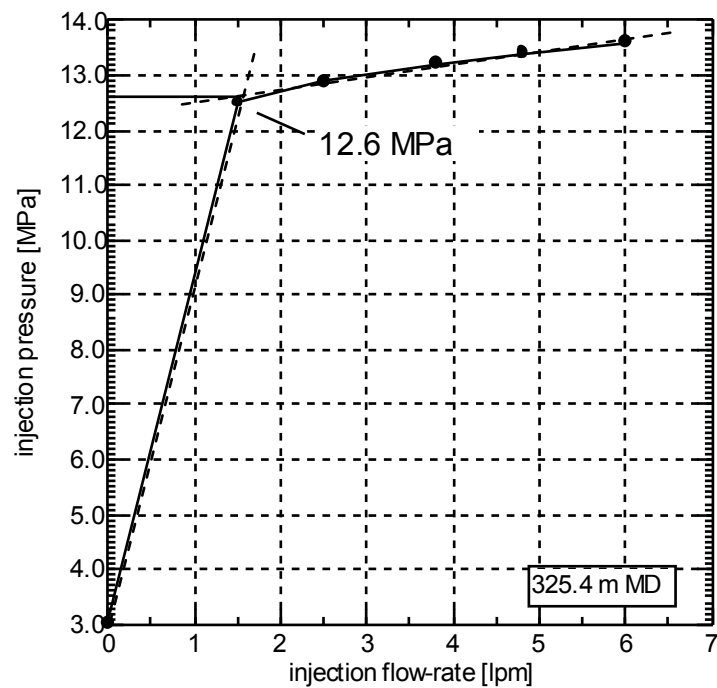
# Test at 325.4 m MD: Estimation of $P_{si}$ (1. Refrac - Cycle)



### Test at 325.4 m MD: Analysis of Slow - Pump / Step - Rate - Test



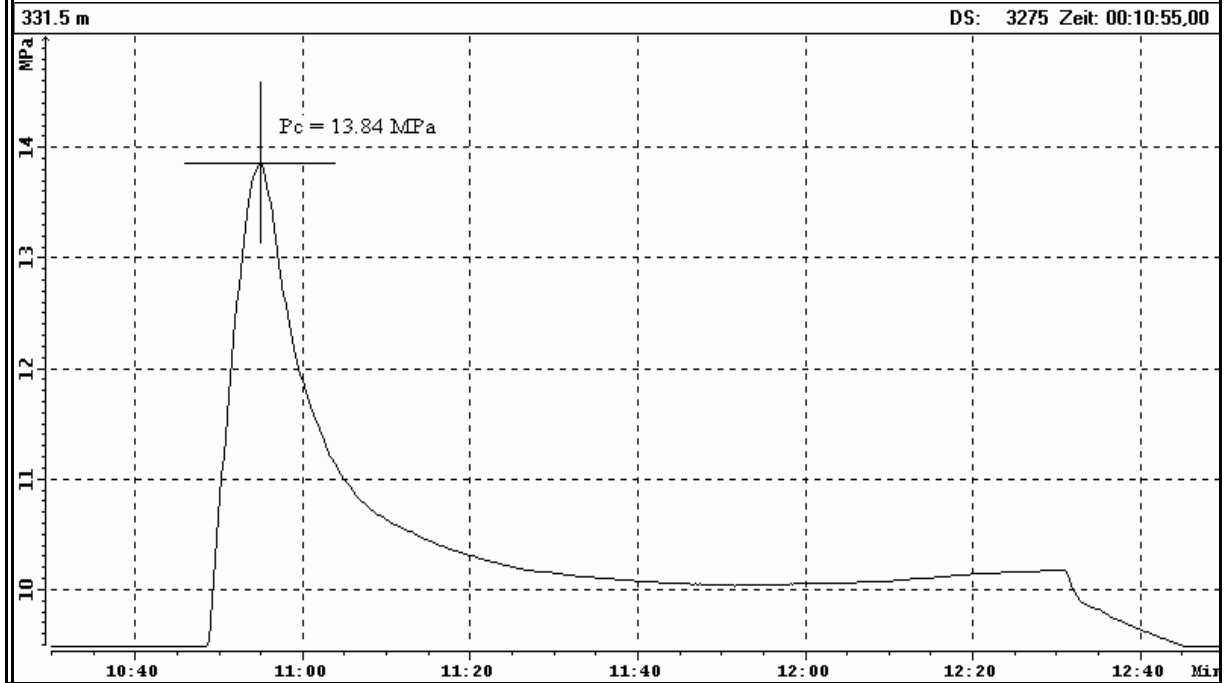
### Test at 325.4 m MD: Examination of $P_{si}$ (Step - Rate - Test)



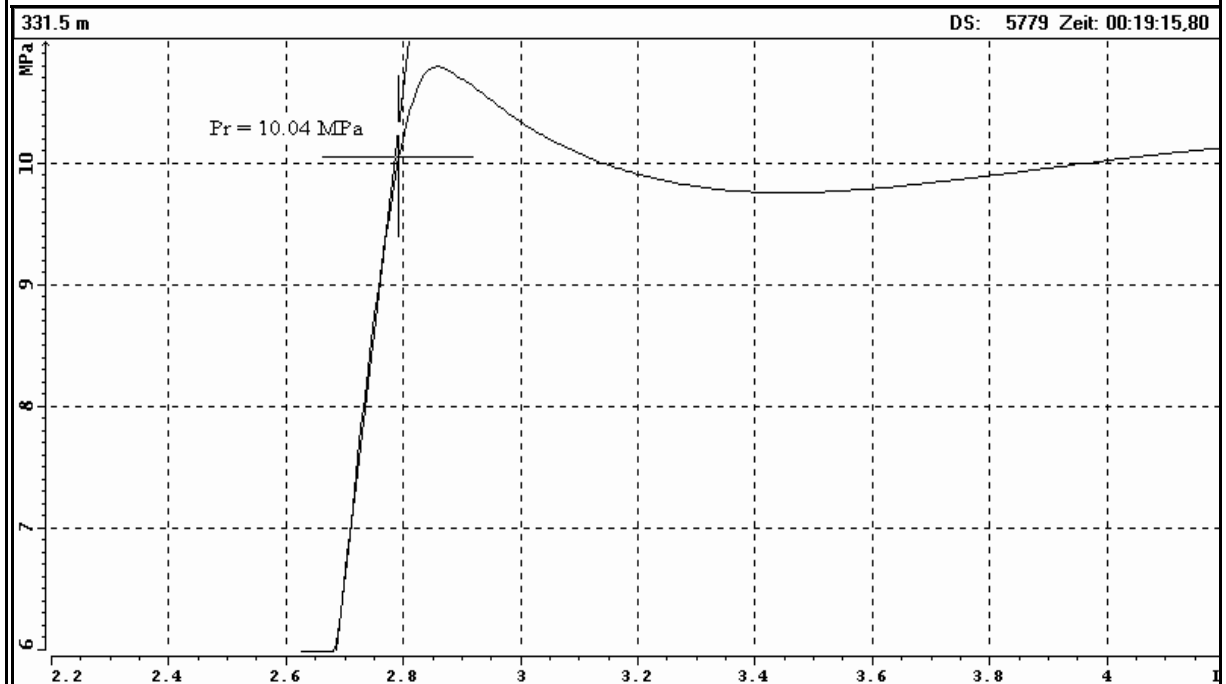




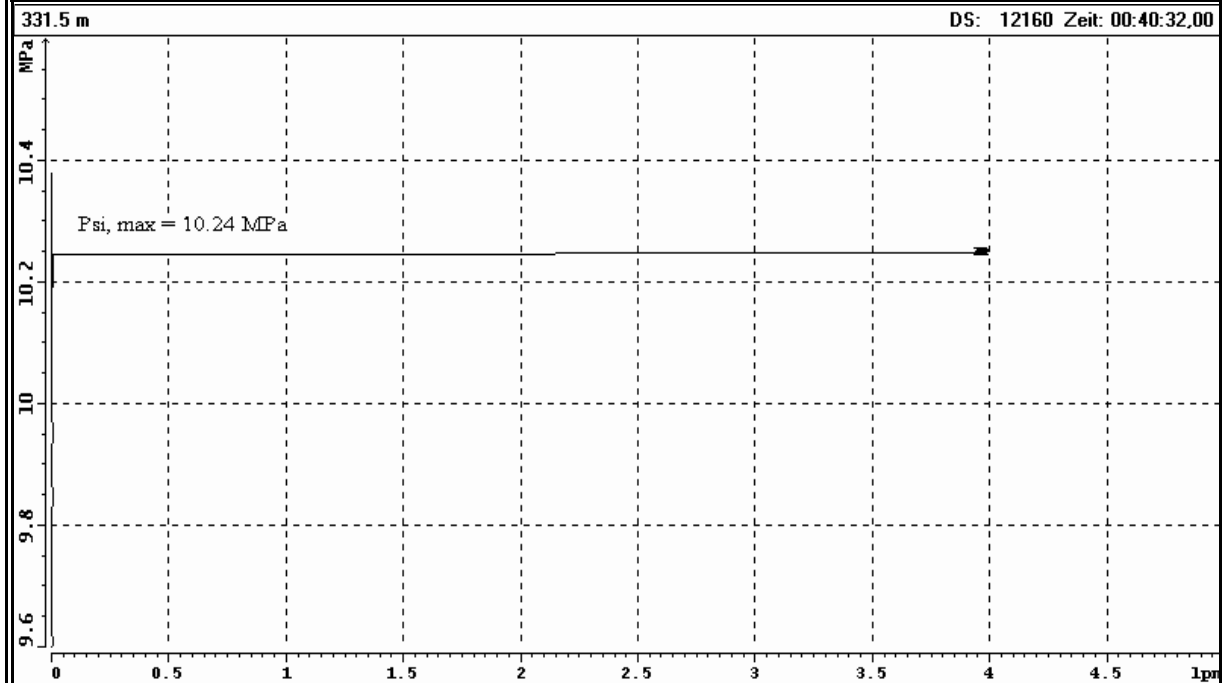
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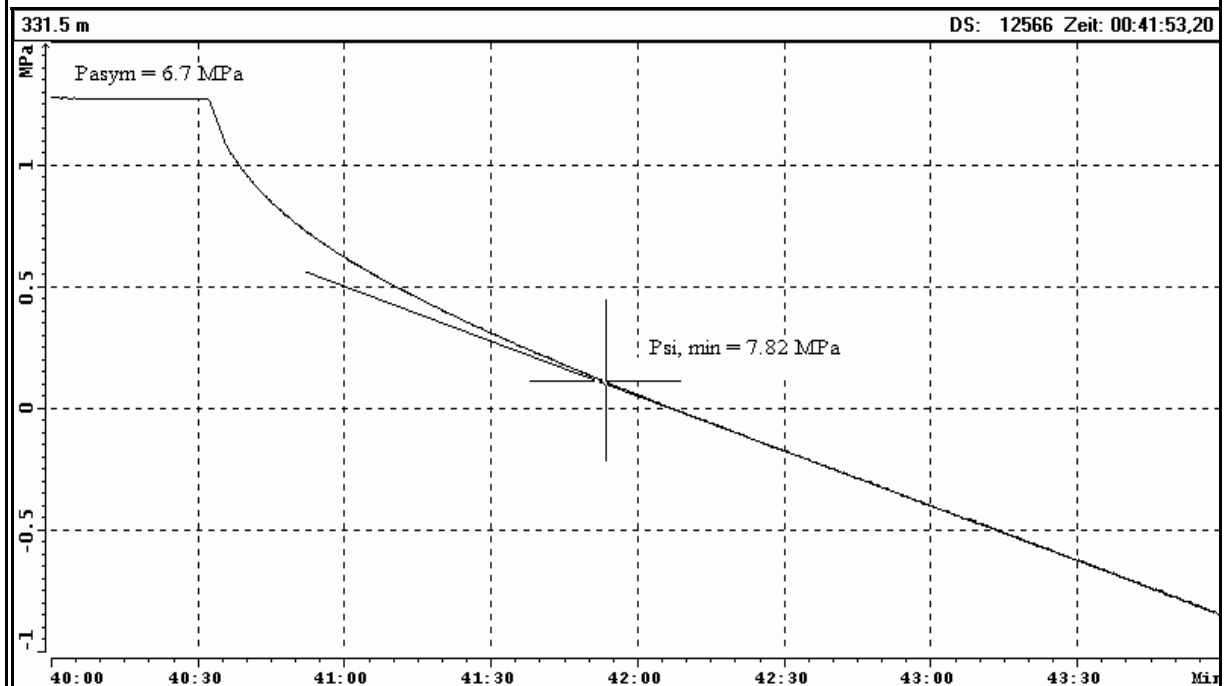
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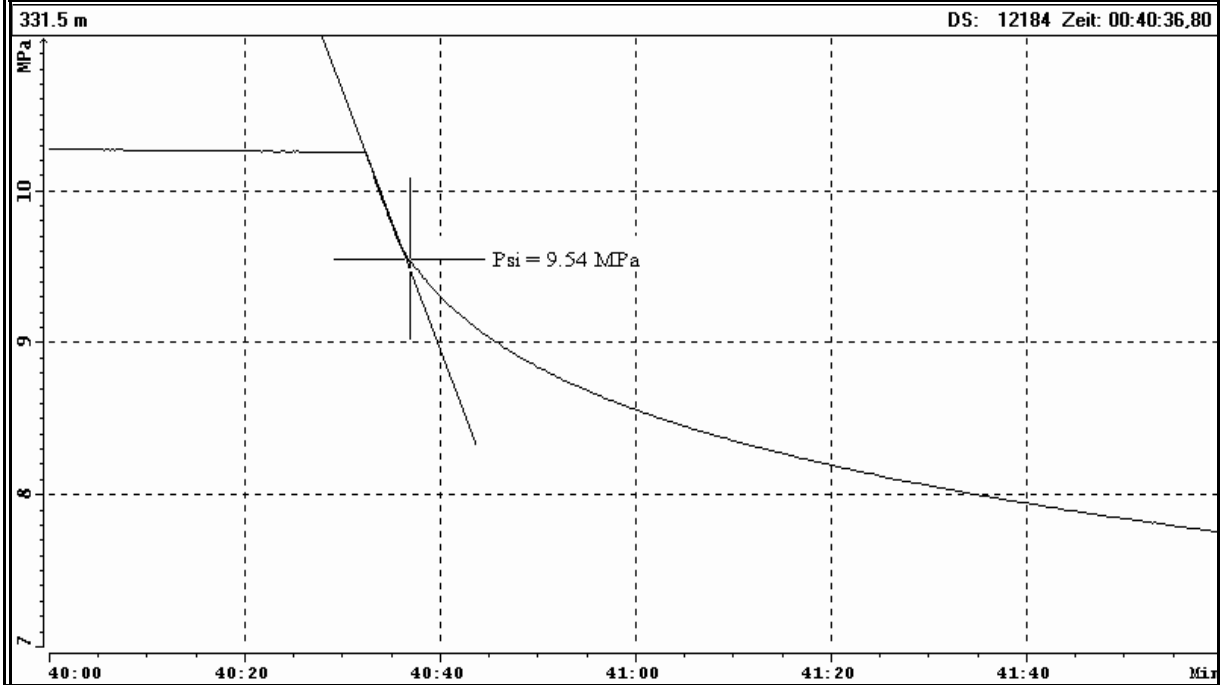
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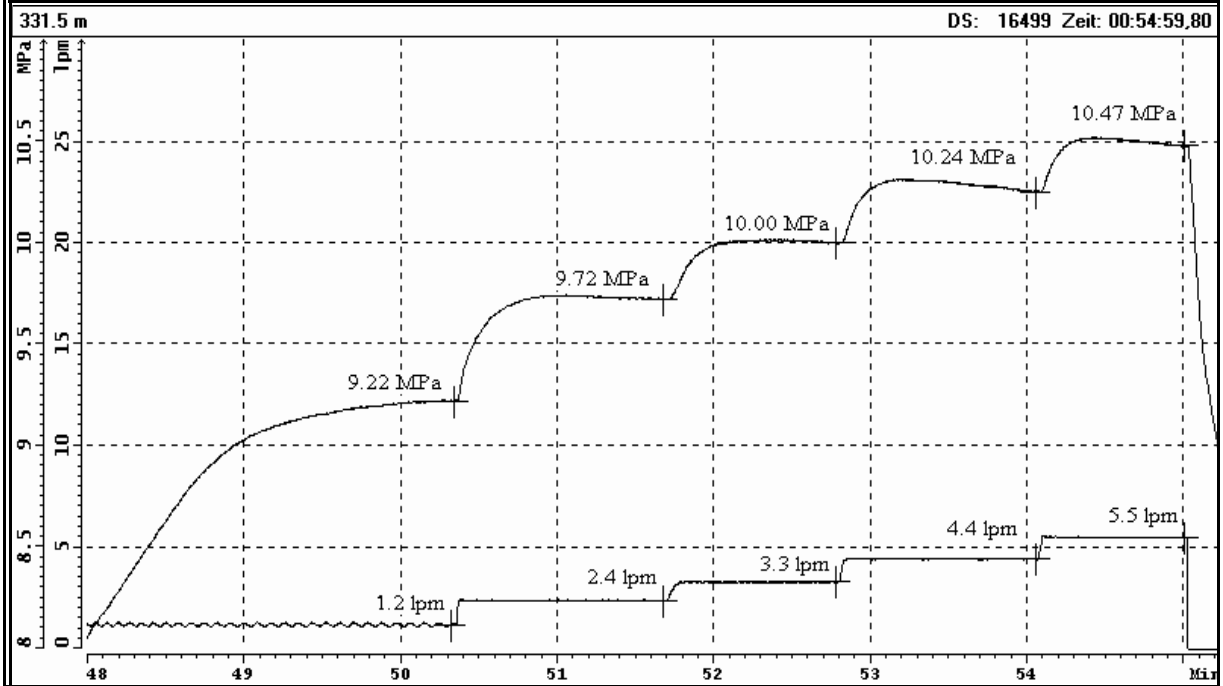
### Test at 331.5 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)



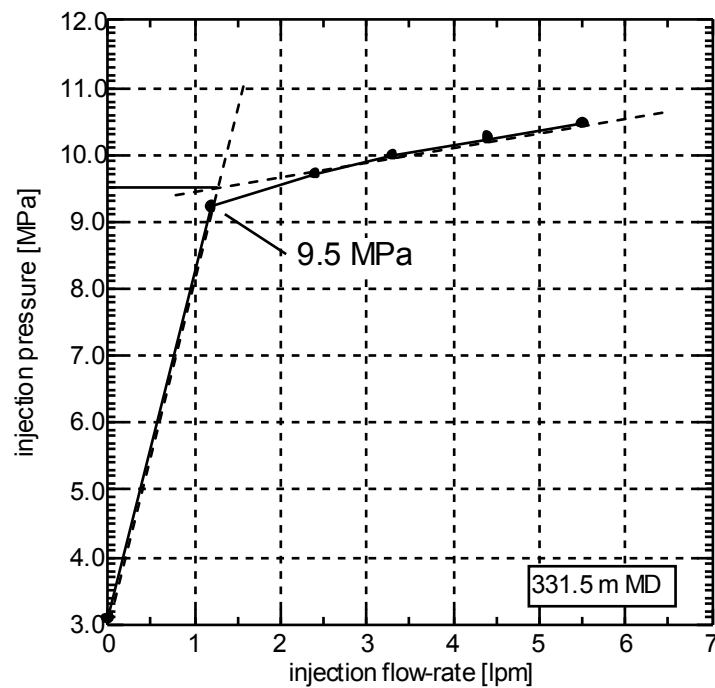
### Test at 331.5 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



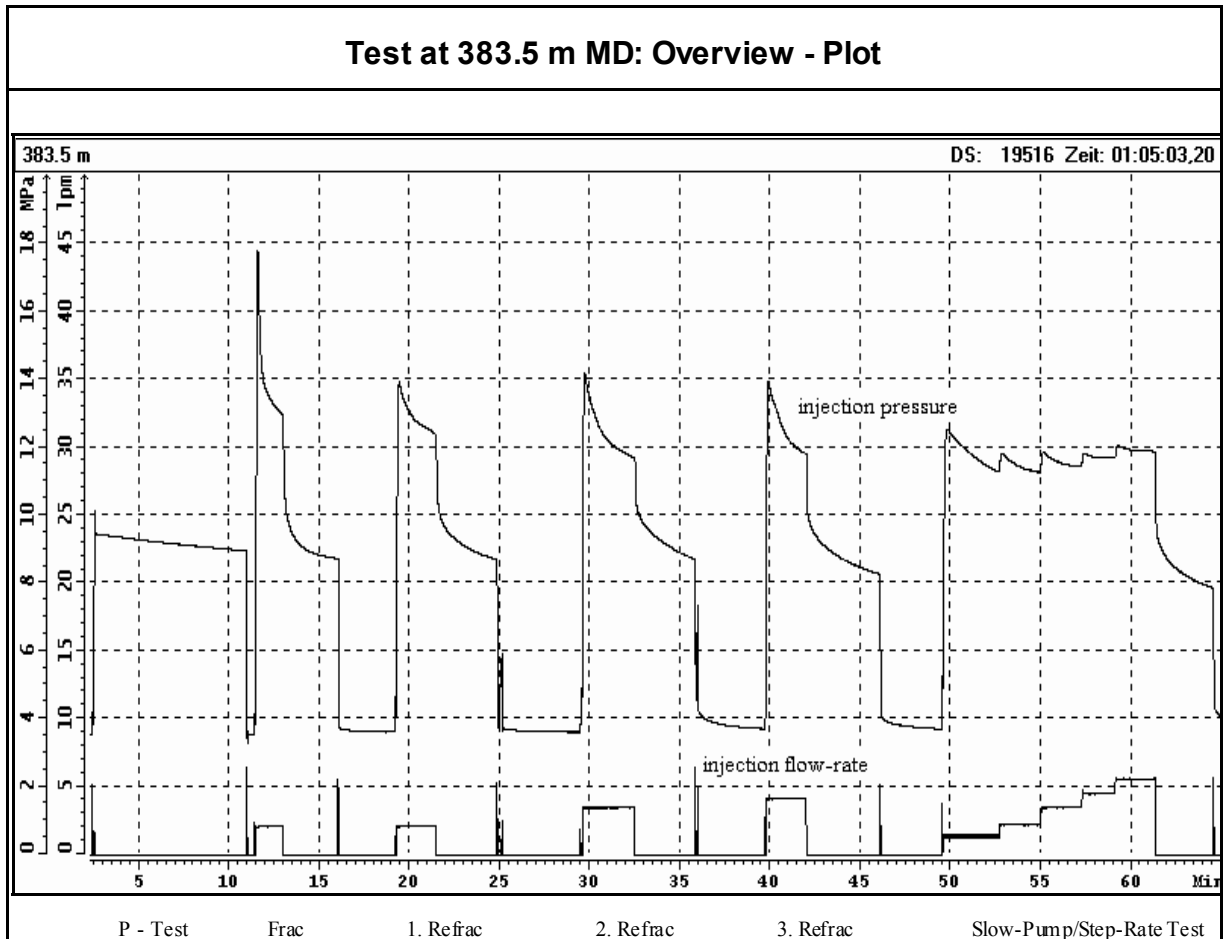
### Test at 331.5 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 331.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST AT 383.5 m MD / 363.0 m TVD

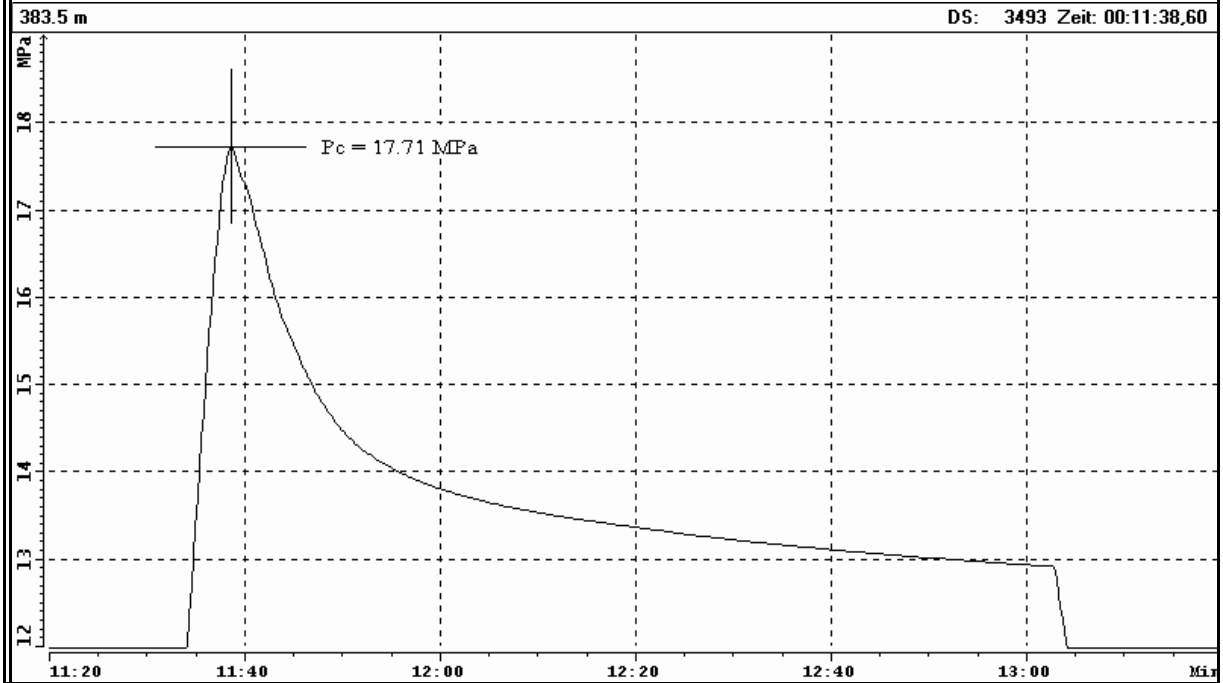


### TEST SUMMARY

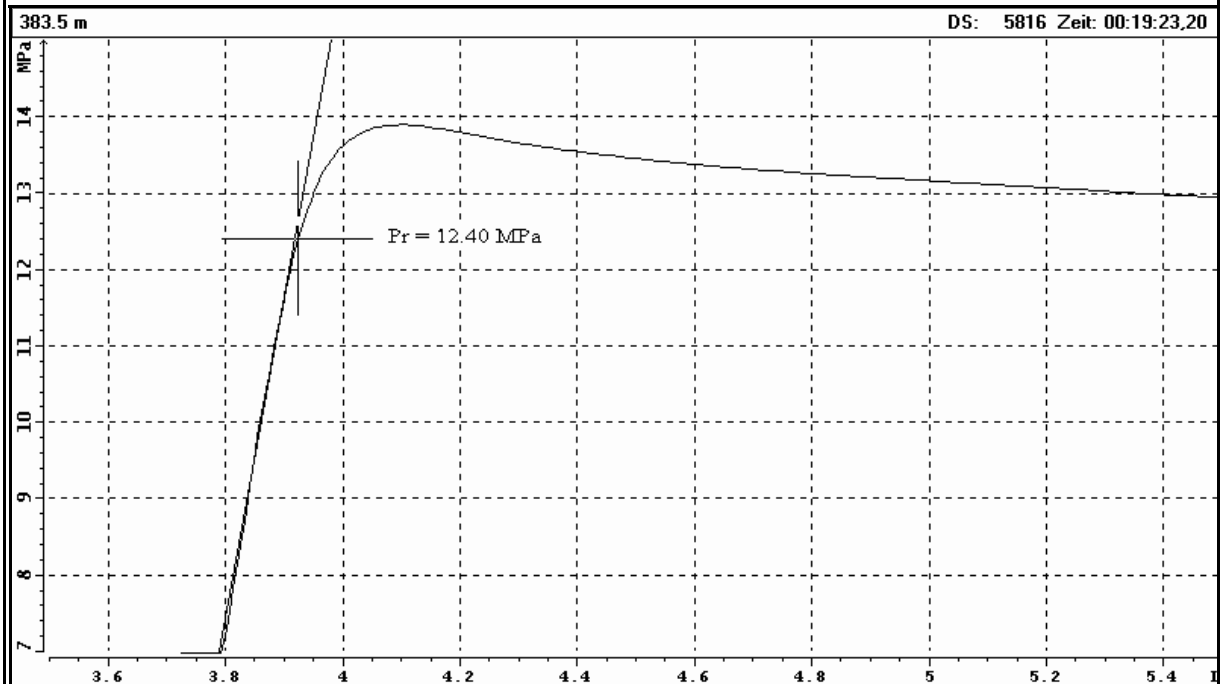
P - Test :	pressure decrease: 0.5 MPa in 8 min. 12 sec.
Frac - Cycle :	injection-rate $Q_i = 2.1$ lpm, injected volume $V_i = 3.3$ l, back-flow volume $V_r = 0.1$ l
	clear fracture initiation (breakdown event)
1. Refrac - Cycle :	$Q_i = 2.1$ lpm, $V_i = 3.3$ l, $V_r = 0.2$ l
2. Refrac - Cycle :	$Q_i = 3.5$ lpm, $V_i = 10.2$ l, $V_r = 0.2$ l
3. Refrac - Cycle :	$Q_i = 4.2$ lpm, $V_i = 9.3$ l, $V_r = 0.1$ l
Step-Rate Test :	$Q_i = 1.4-5.6$ lpm, $V_i = 38.2$ l, $V_r = 0.1$ l

total injected volume = 65.8 l, recovered volume = 0.7 l (1.1 %)

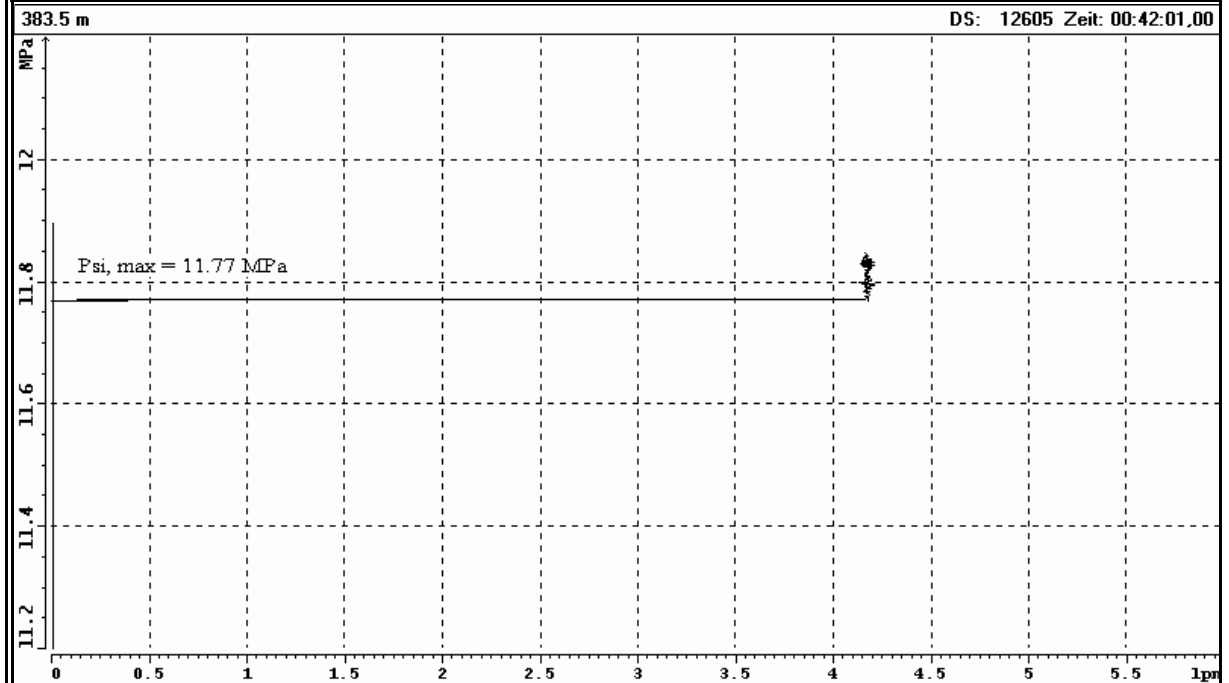
### Test at 383.5 m MD: Estimation of $P_c$ (Frac - Cycle)



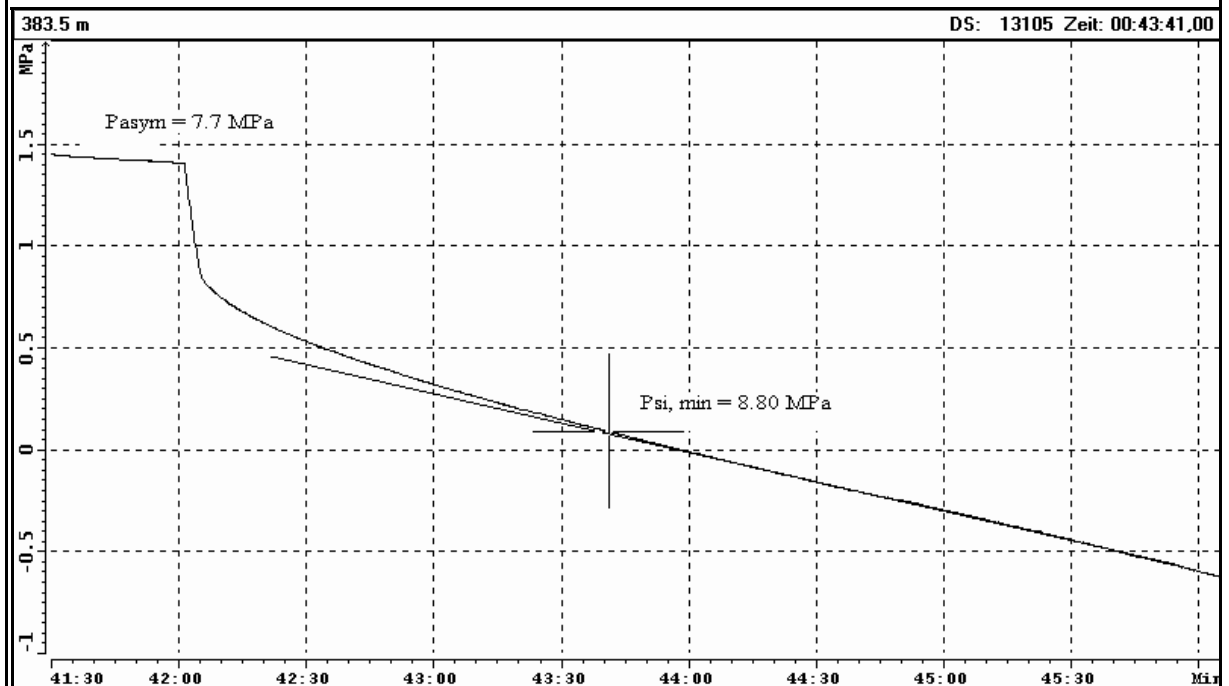
### Test at 383.5 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



### Test at 383.5 m MD: Estimation of $P_{si, max}$ (3. Refrac - Cycle)

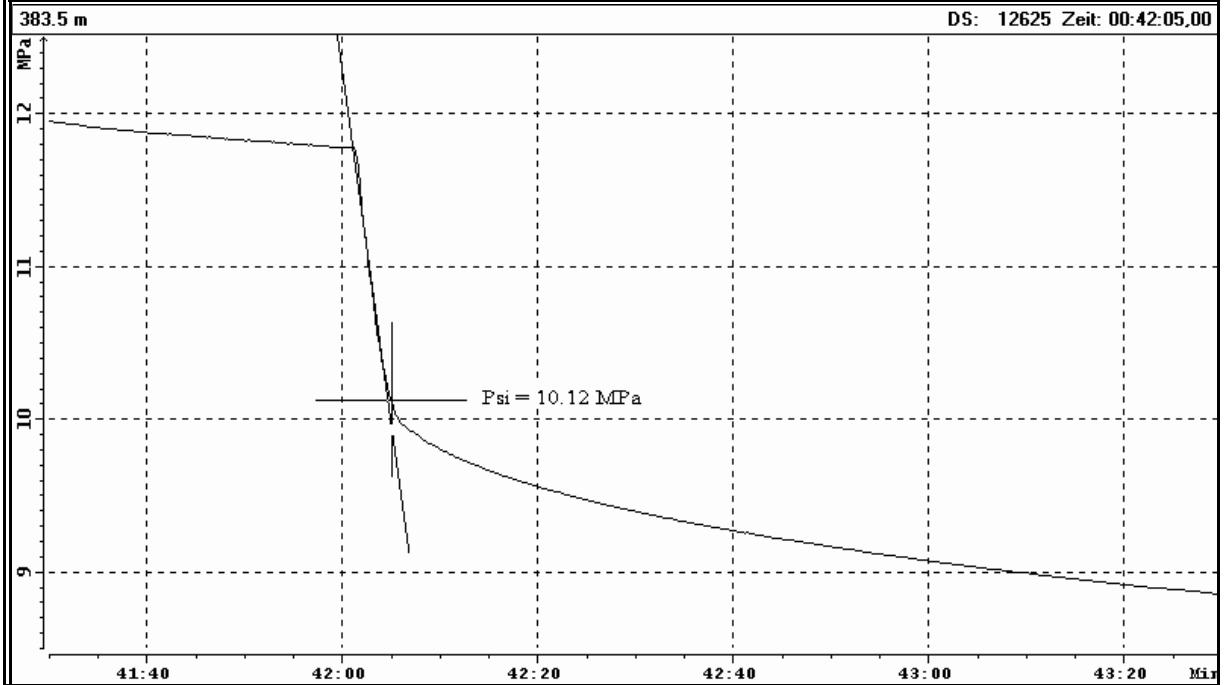


### Test at 383.5 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)

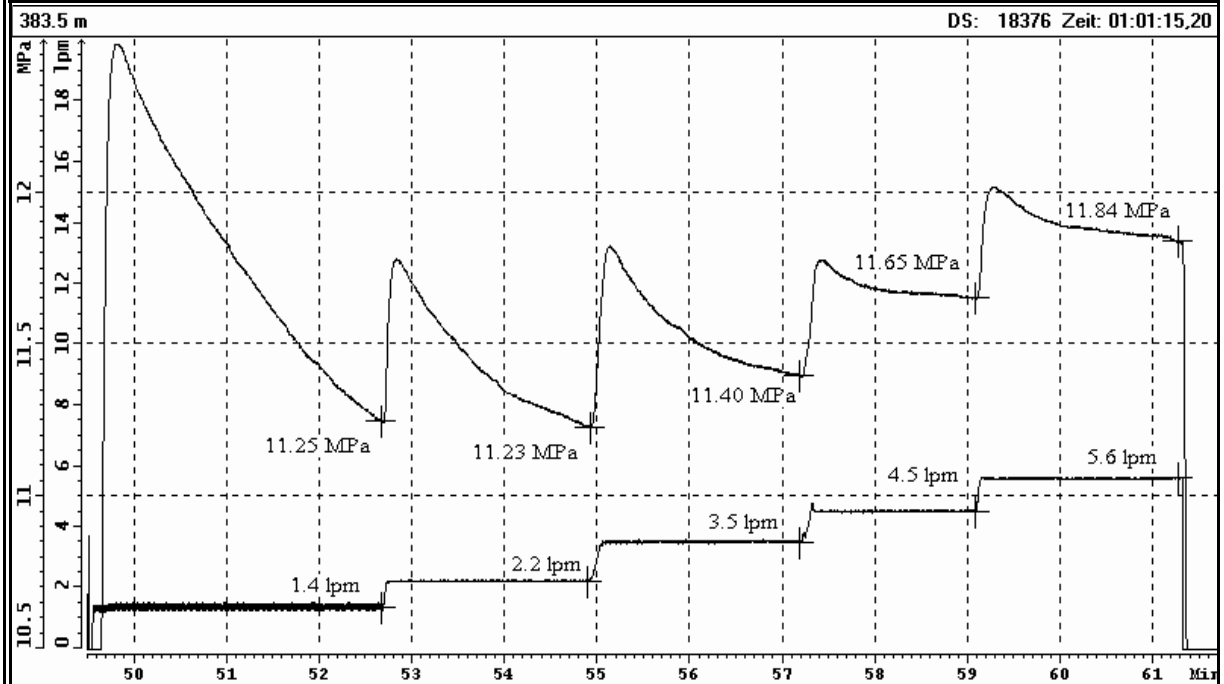




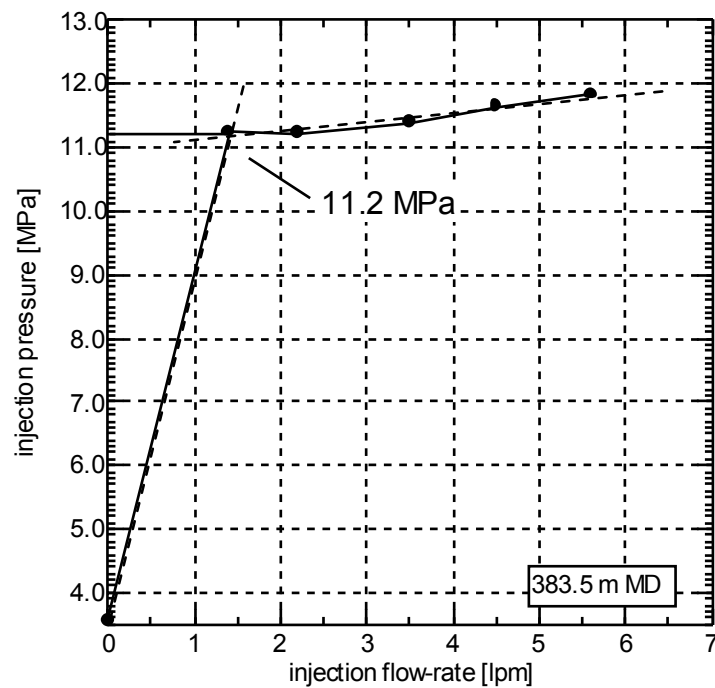
### Test at 383.5 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 383.5 m MD: Analysis of Slow - Pump / Step - Rate - Test

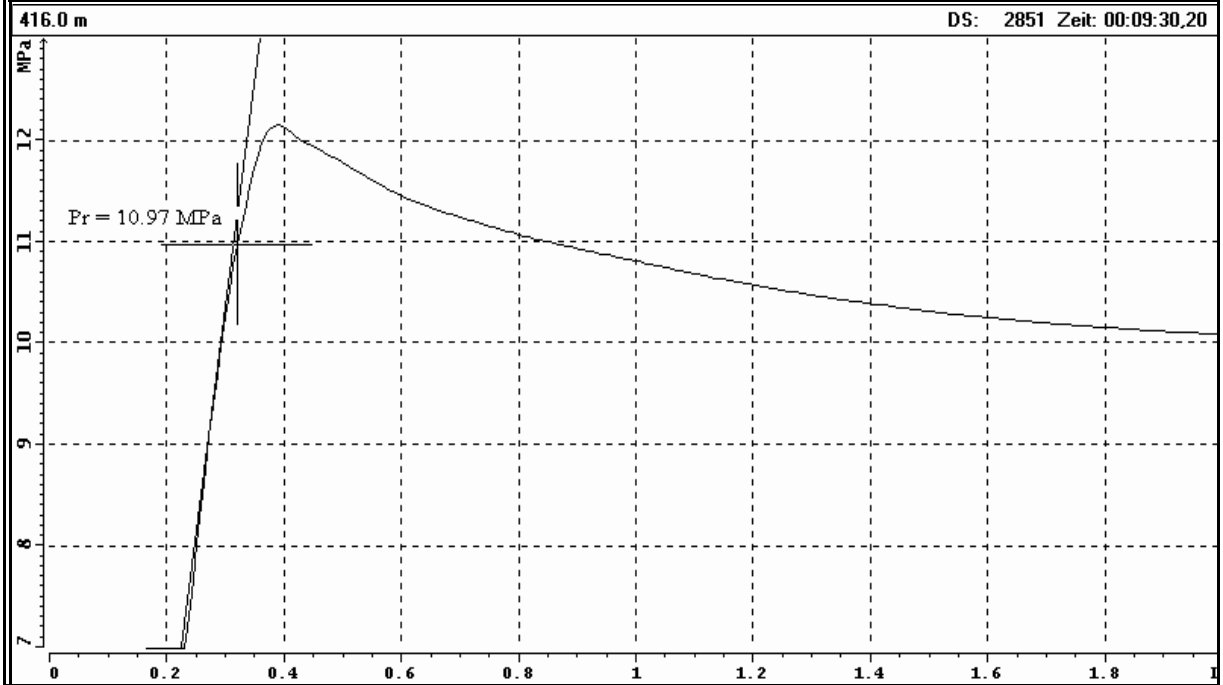


### Test at 383.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)

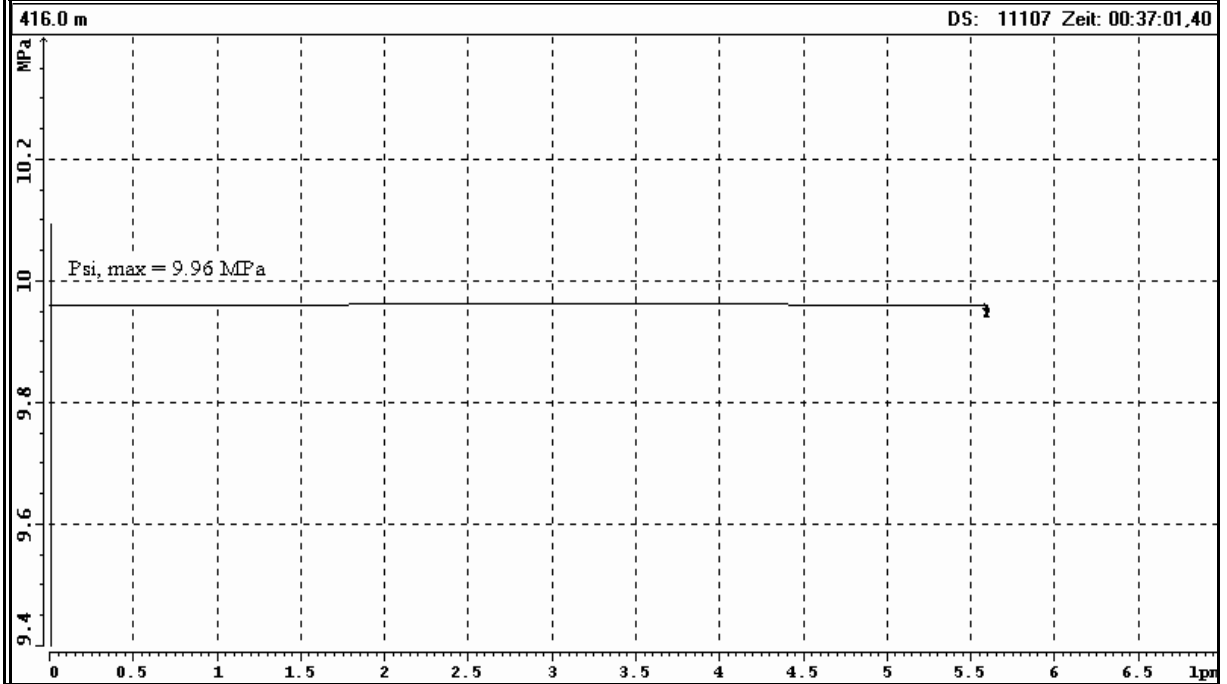




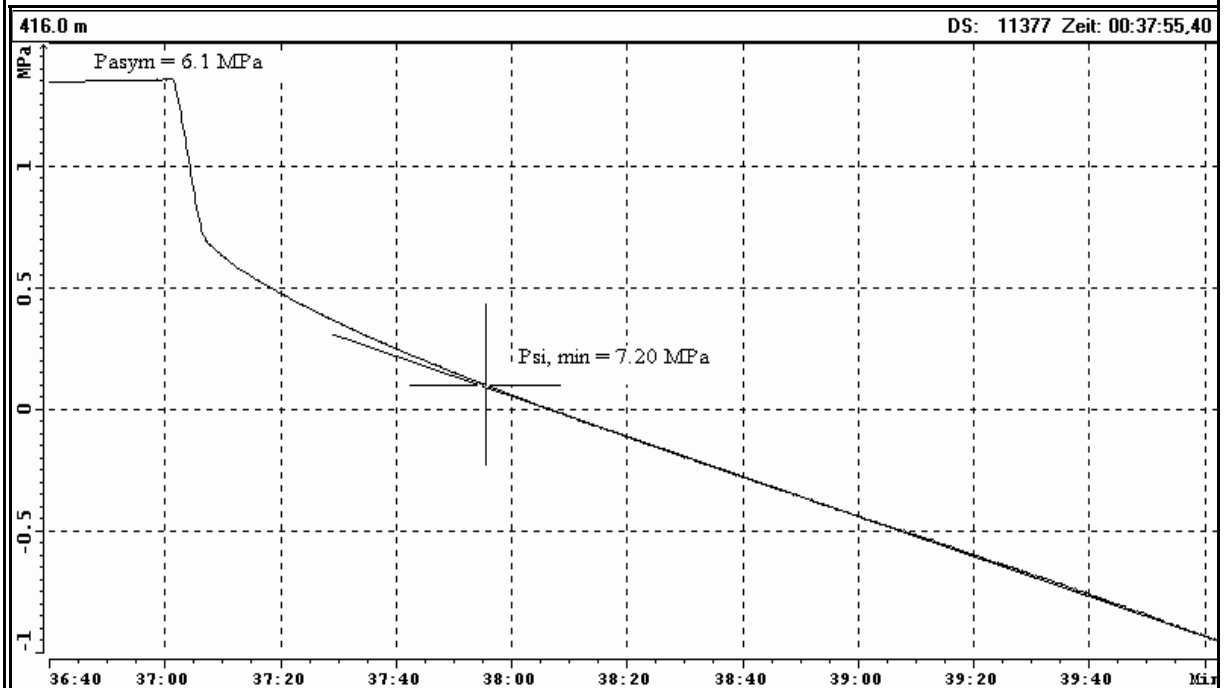
### Test at 416.0 m MD: Estimation of $P_r$ (Frac (1. Refrac) - Cycle)



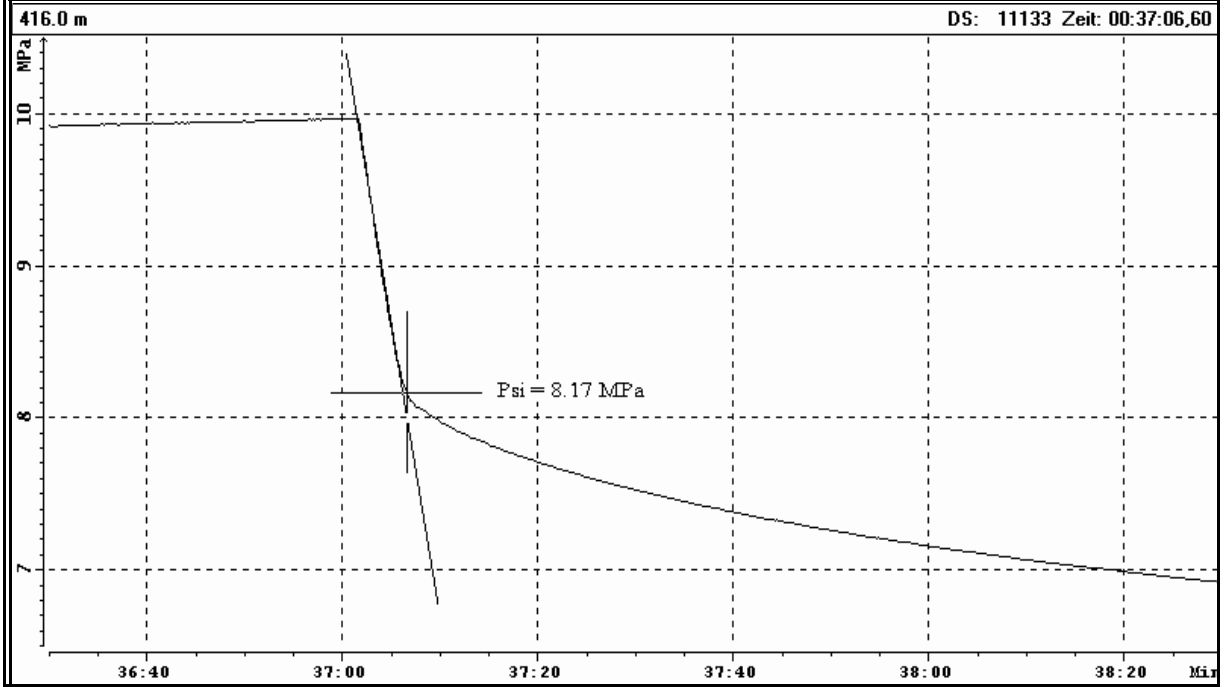
### Test at 416.0 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



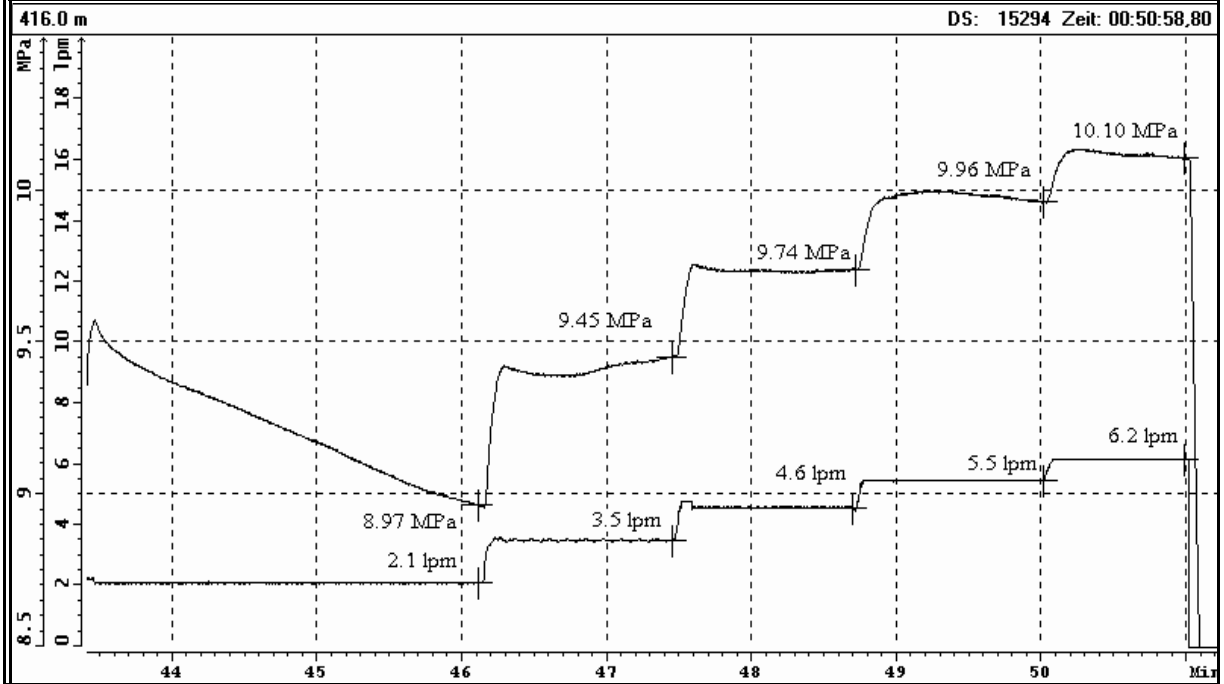
### Test at 416.0 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)



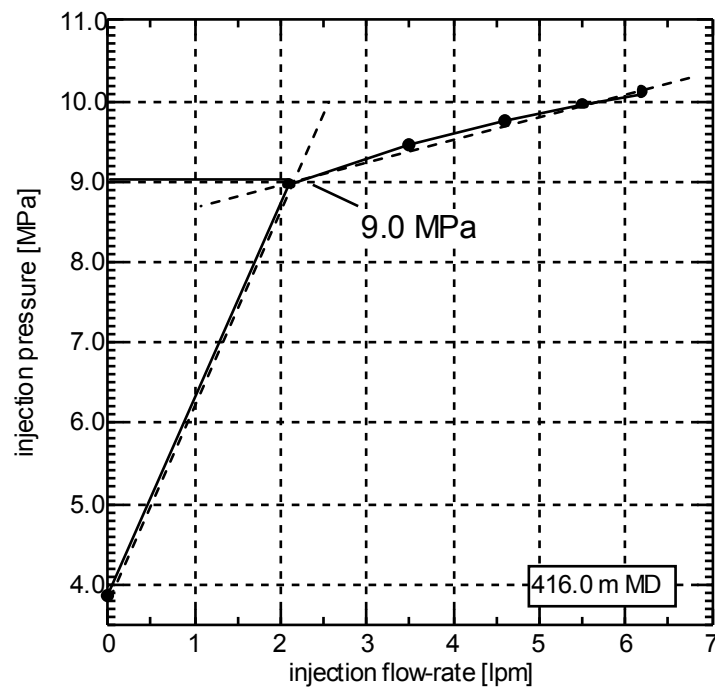
### Test at 416.0 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



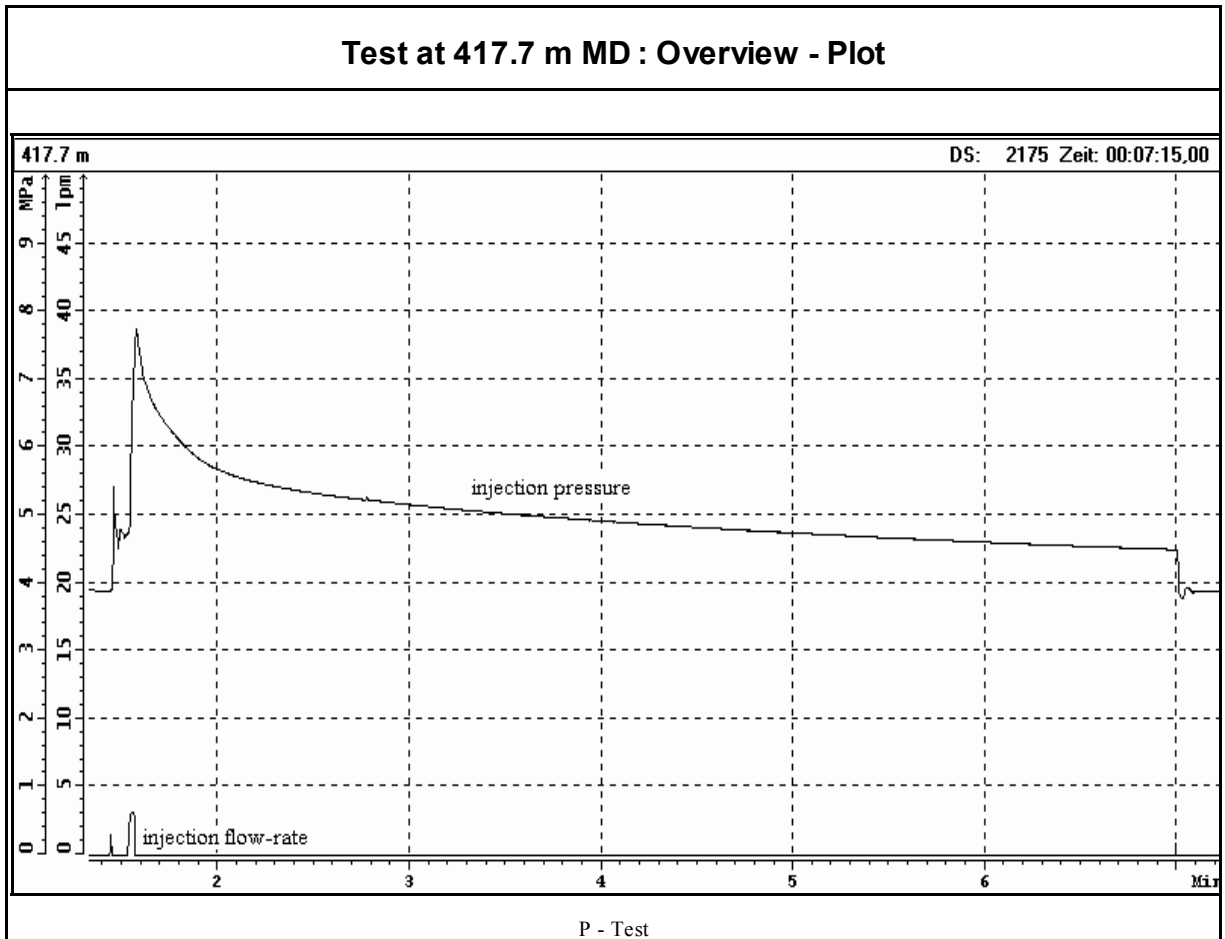
### Test at 416.0 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 416.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST ATTEMPT AT 417.7 m MD / 395.5 m TVD



### TEST SUMMARY

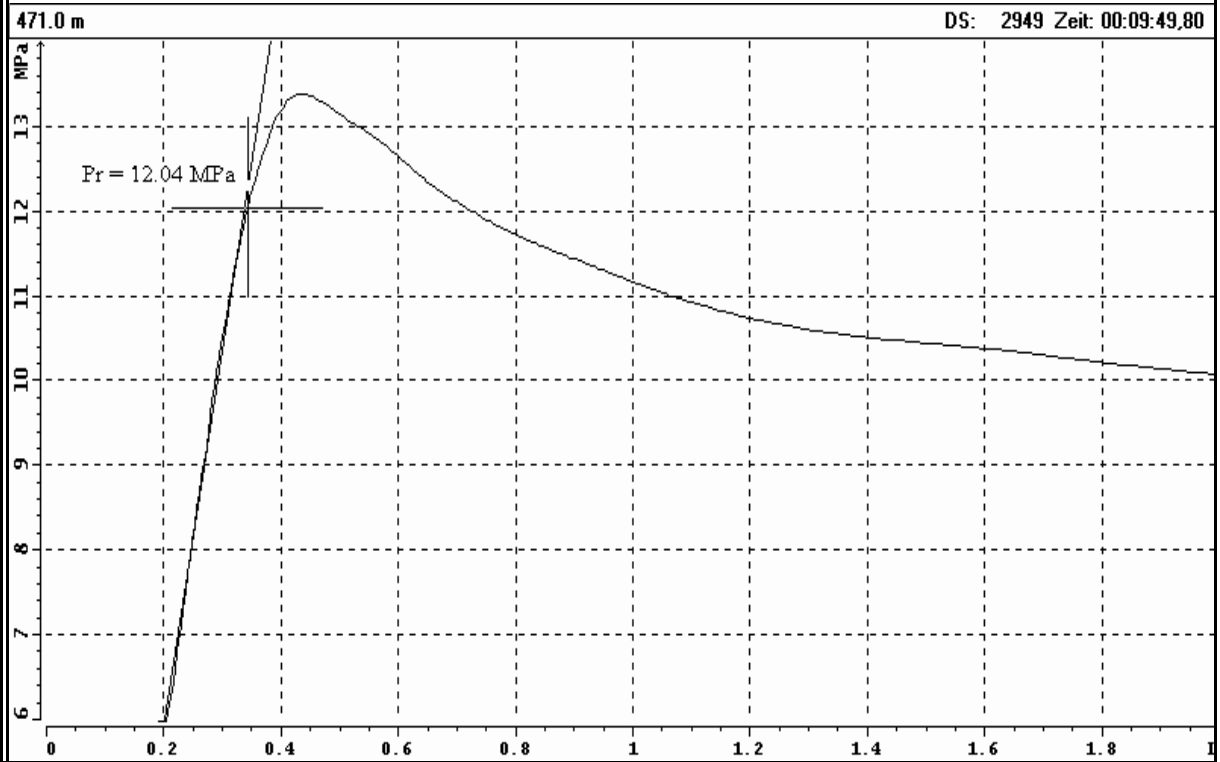
P - Test :                      pressure decrease: 3.2 MPa in 5 min. 24 sec.

Remark: *Test abandoned due to the high rock-mass permeability.*

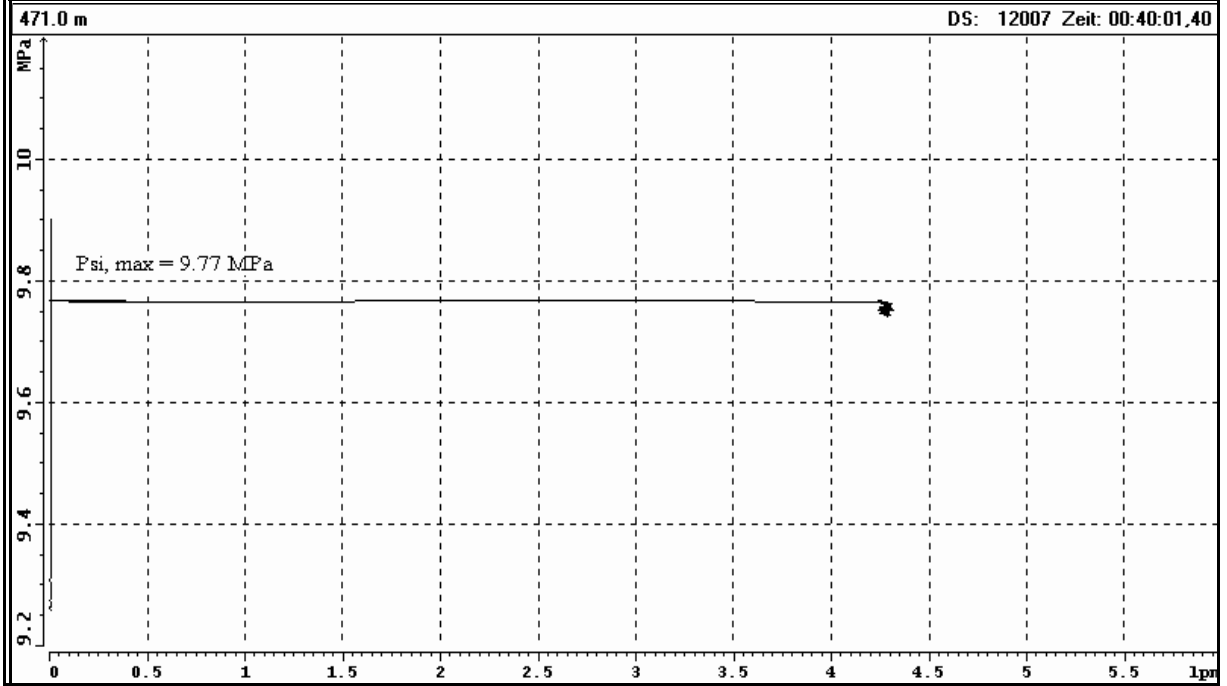




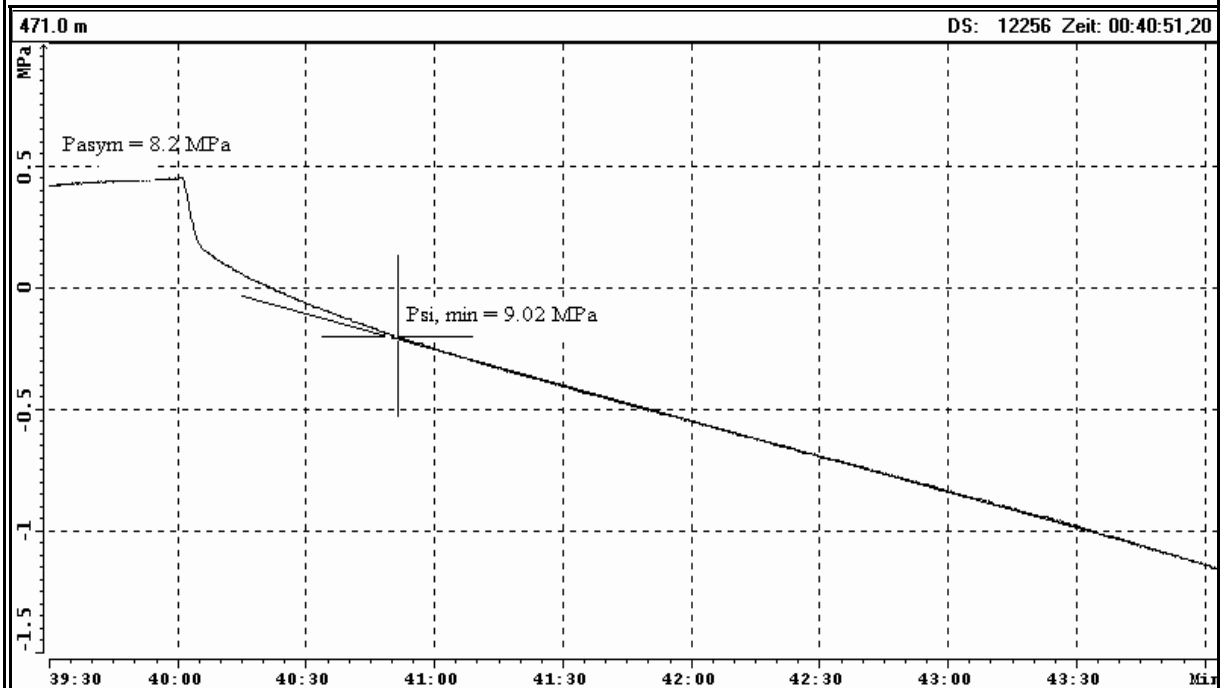
### Test at 471.0 m MD: Estimation of $P_r$ (Frac (1. Refrac) - Cycle)



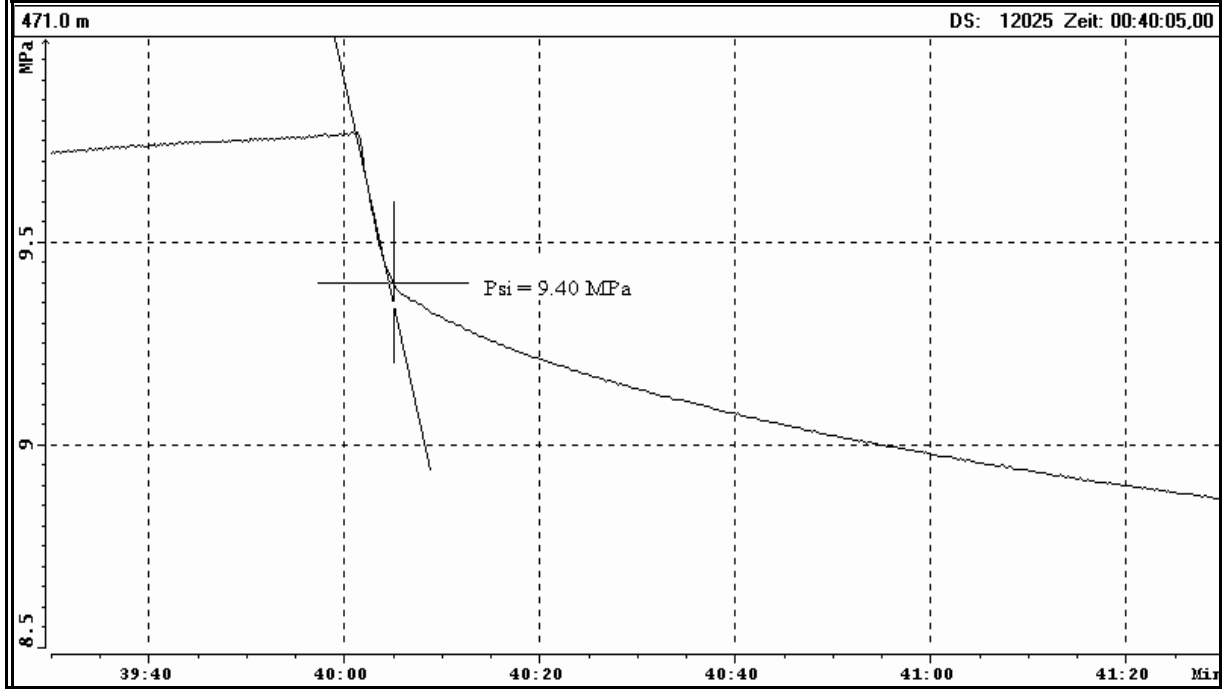
### Test at 471.0 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



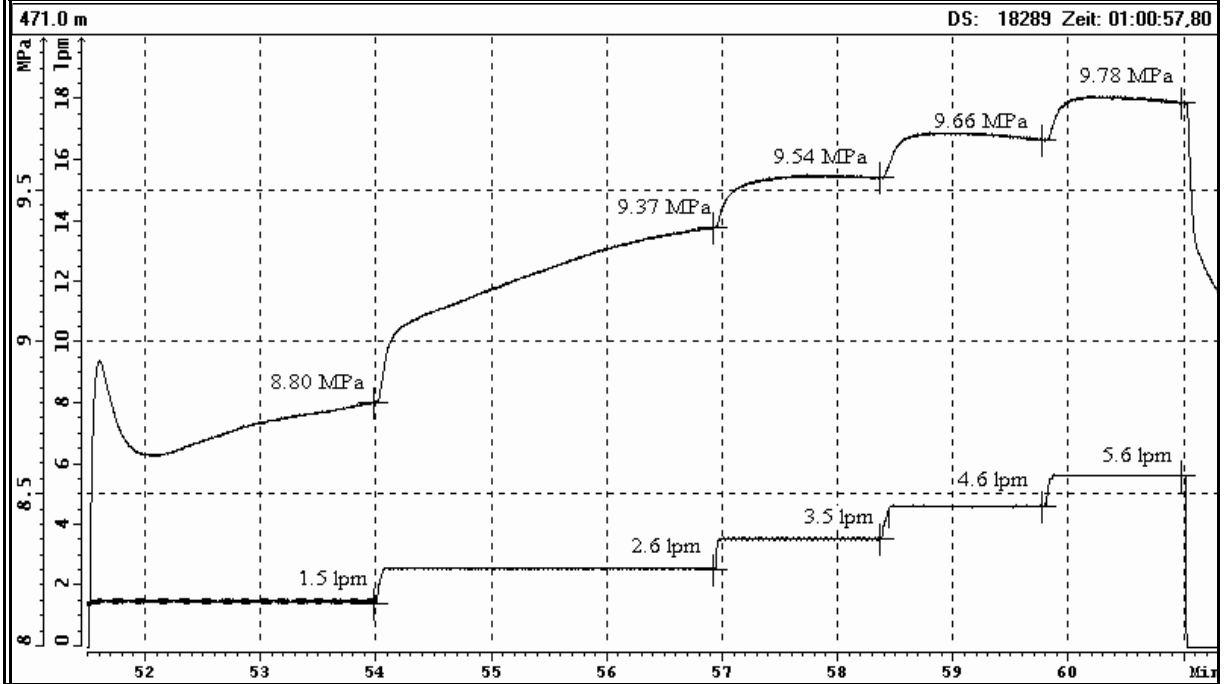
### Test at 471.0 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)



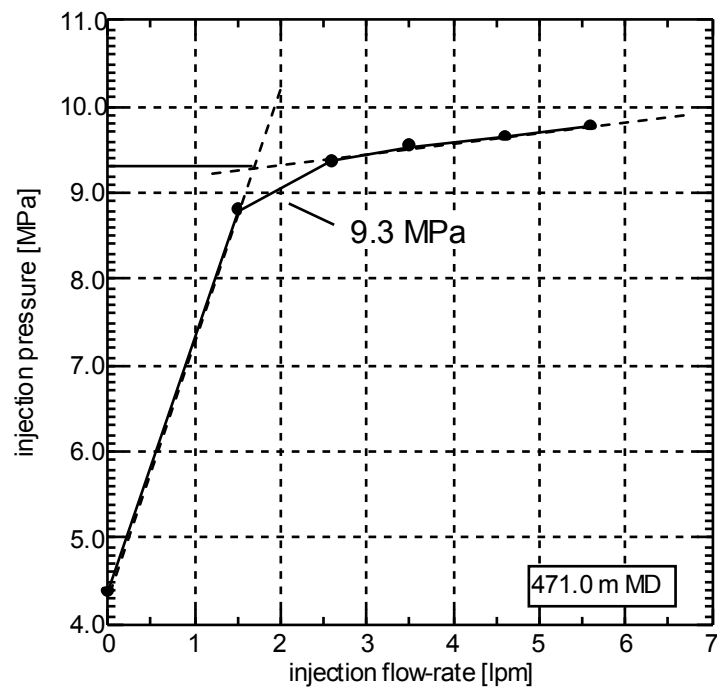
### Test at 471.0 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



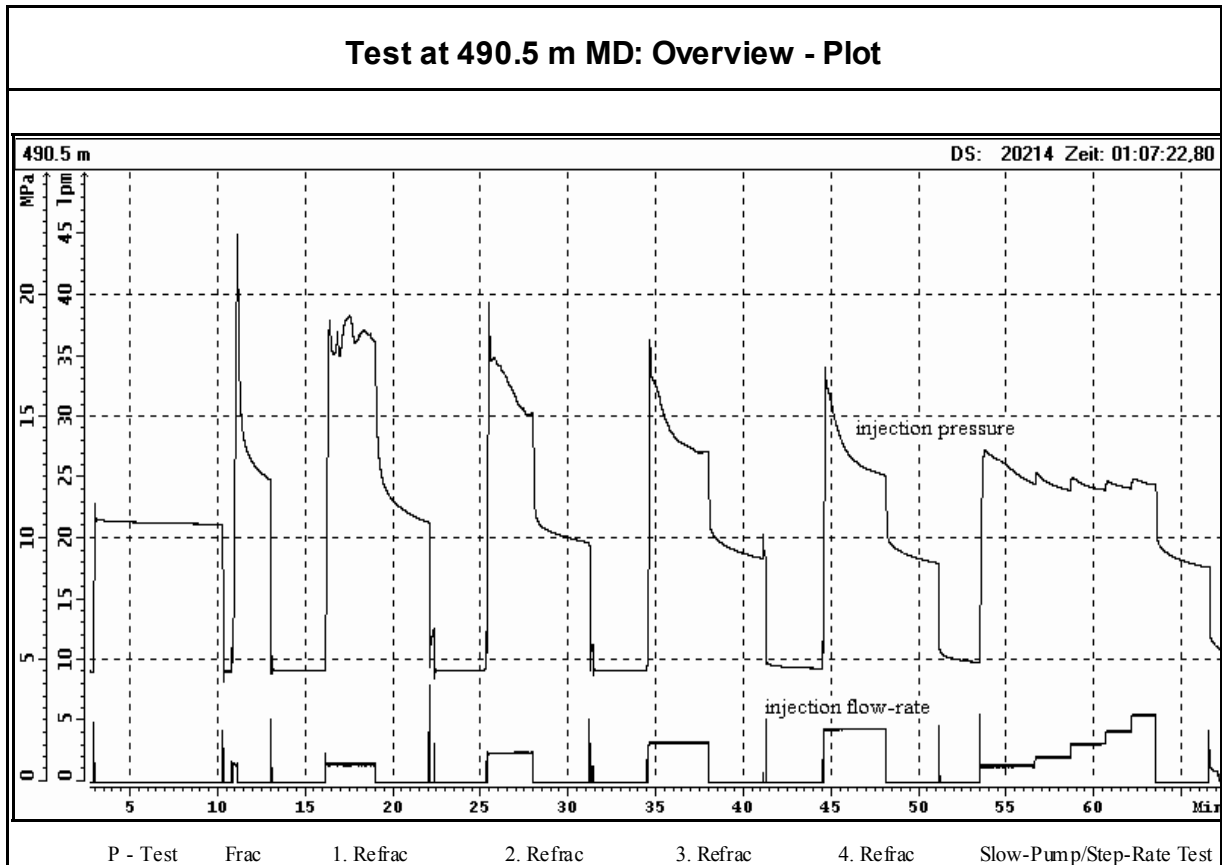
### Test at 471.0 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 471.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST AT 490.5 m MD / 465.8 m TVD



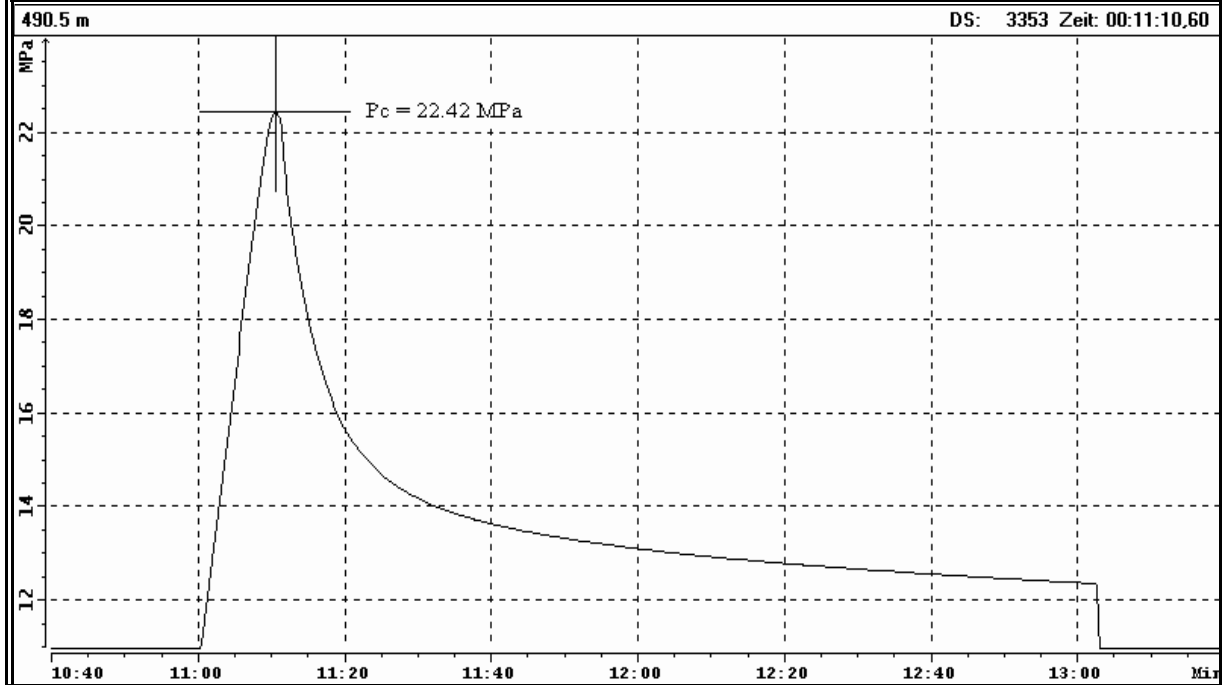
### TEST SUMMARY

P - Test :	pressure decrease: 0.2 MPa in 7 min. 10 sec.
Frac - Cycle :	injection-rate $Q_i = 1.5$ lpm, injected volume $V_i = 0.4$ l, back-flow volume $V_r = 0.1$ l clear fracture initiation (breakdown event)
1. Refrac - Cycle :	$Q_i = 1.5$ lpm, $V_i = 4.3$ l, $V_r = 0.1$ l
2. Refrac - Cycle :	$Q_i = 2.4$ lpm, $V_i = 6.4$ l, $V_r = 0.1$ l
3. Refrac - Cycle :	$Q_i = 3.3$ lpm, $V_i = 11.2$ l, $V_r = 0.1$ l
4. Refrac - Cycle :	$Q_i = 4.3$ lpm, $V_i = 15.4$ l, $V_r = 0.1$ l
Step-Rate Test :	$Q_i = 1.4$ - $5.5$ lpm, $V_i = 28.7$ l, $V_r = 0.6$ l

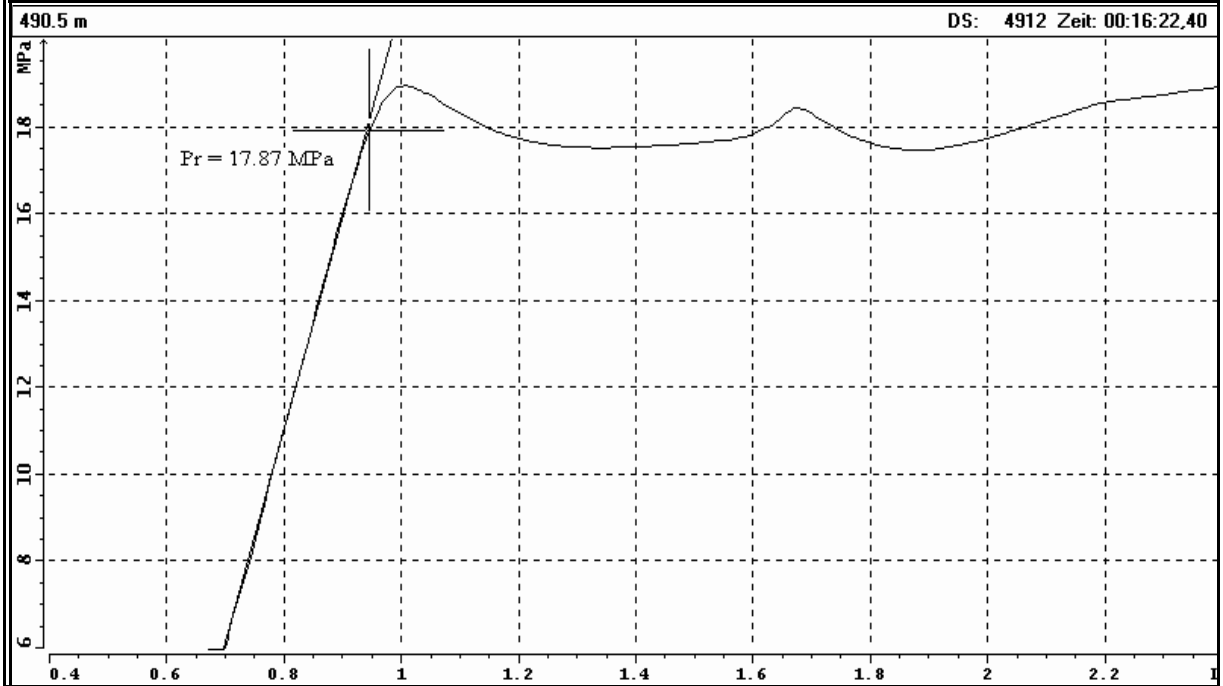
total injected volume = 66.4 l, recovered volume = 1.1 l (1.7 %)

*Remark: During the test, a significant decrease of the shut-in pressure from 15.3 MPa (1. refrac-cycle) to 10.3 MPa (4. refrac-cycle) was observed.*

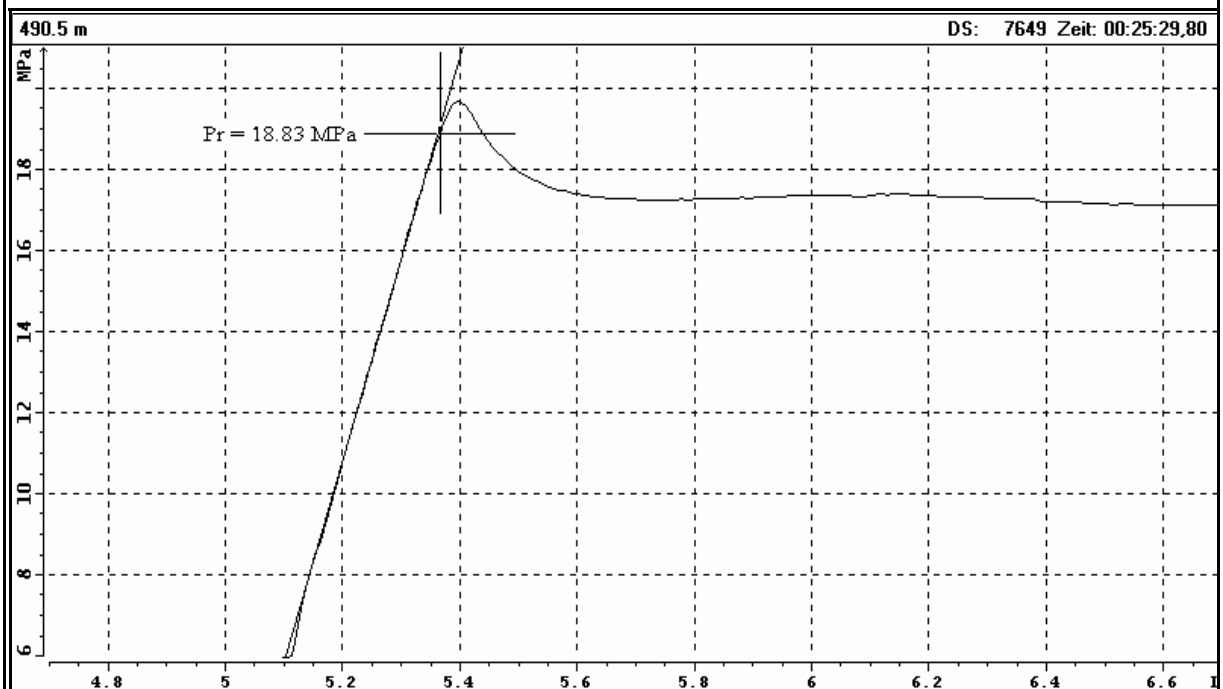
### Test at 490.5 m MD: Estimation of $P_c$ (Frac - Cycle)



### Test at 490.5 m MD: Estimation of $P_r$ (1. Refrac - Cycle)

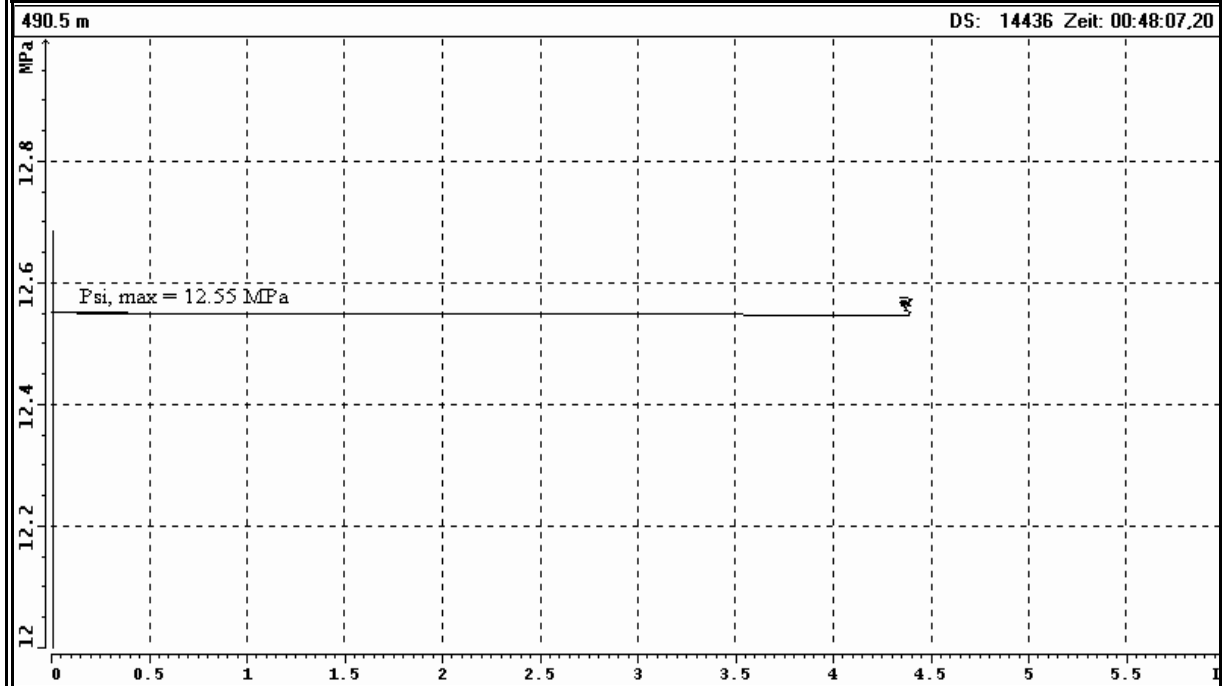


### Test at 490.5 m MD: Estimation of $P_r$ (2. Refrac - Cycle)

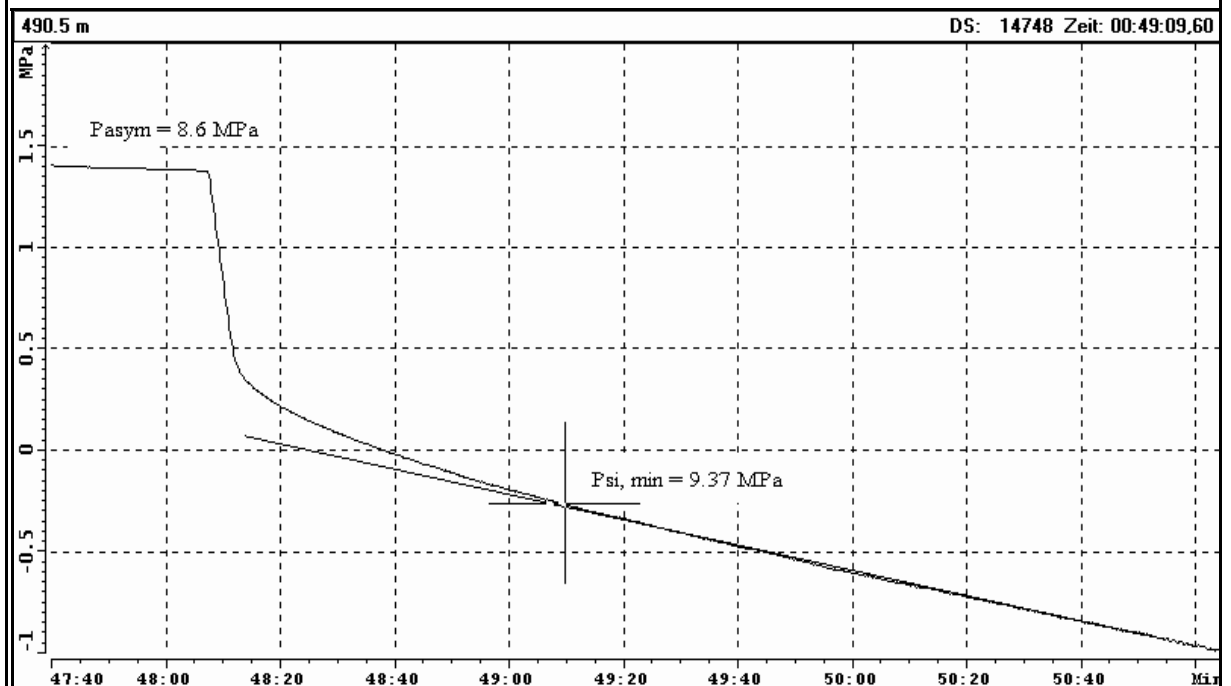




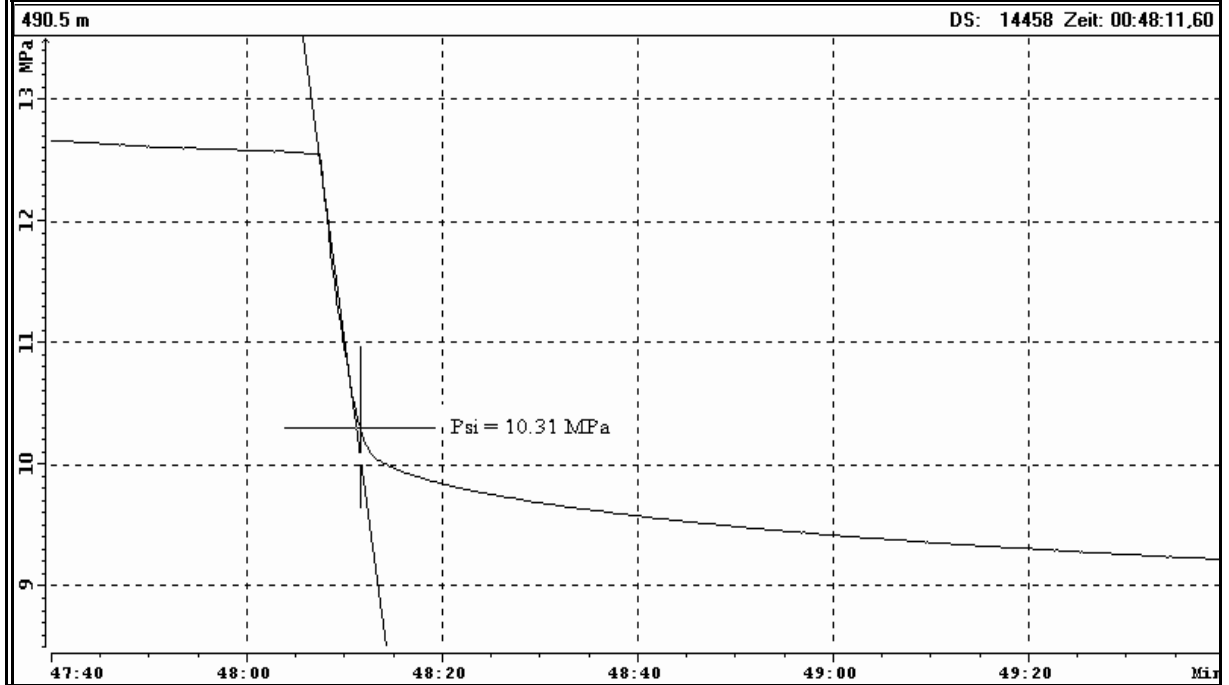
### Test at 490.5 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



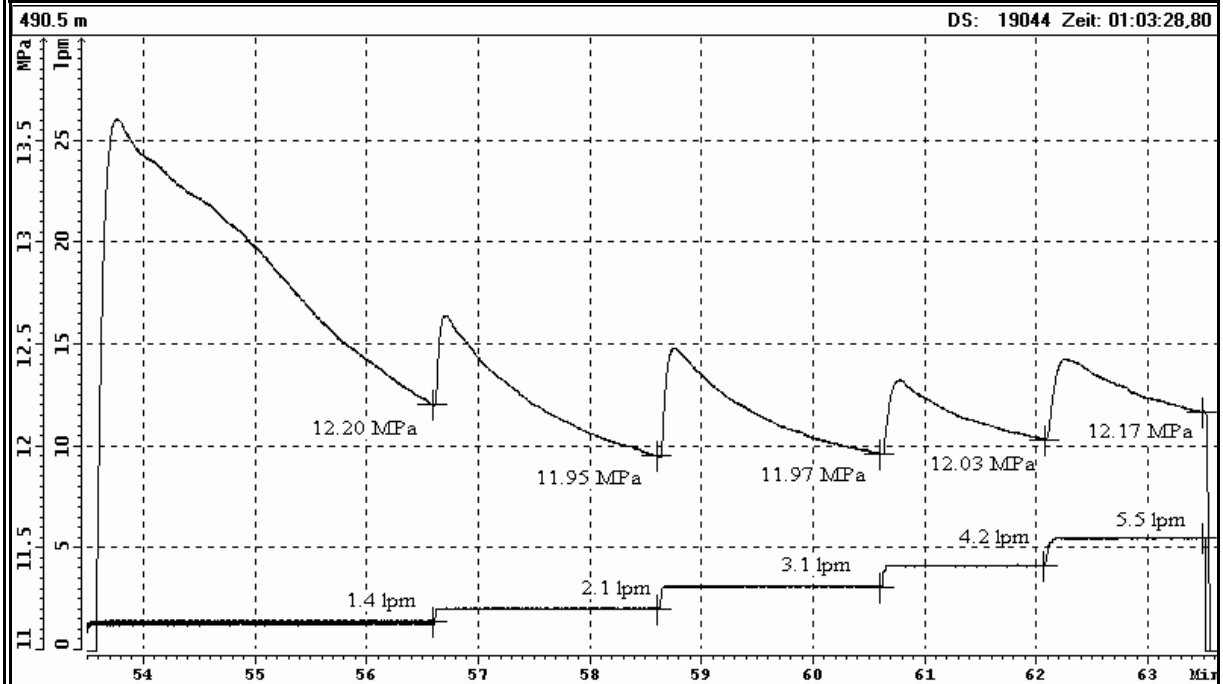
### Test at 490.5 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)



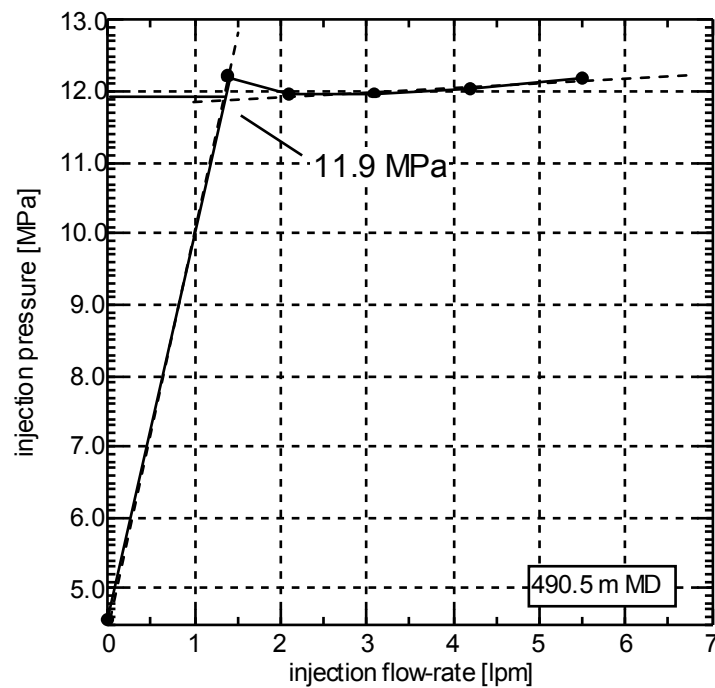
### Test at 490.5 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



### Test at 490.5 m MD: Analysis of Slow - Pump / Step - Rate - Test

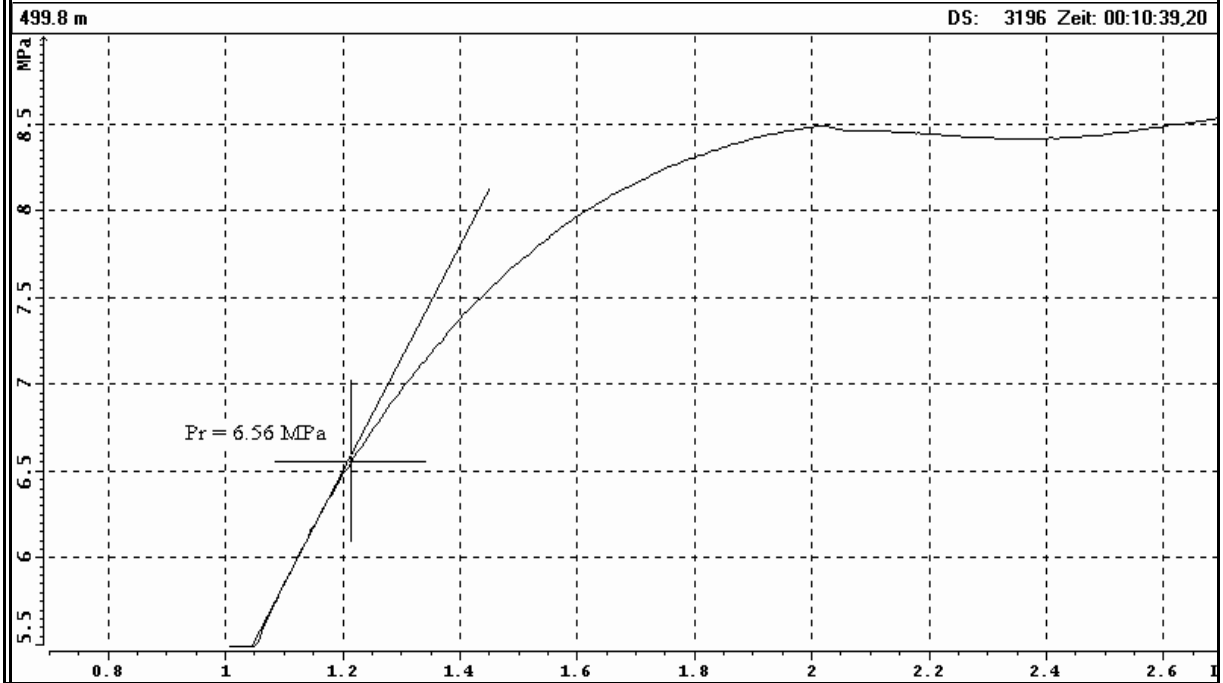


### Test at 490.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)

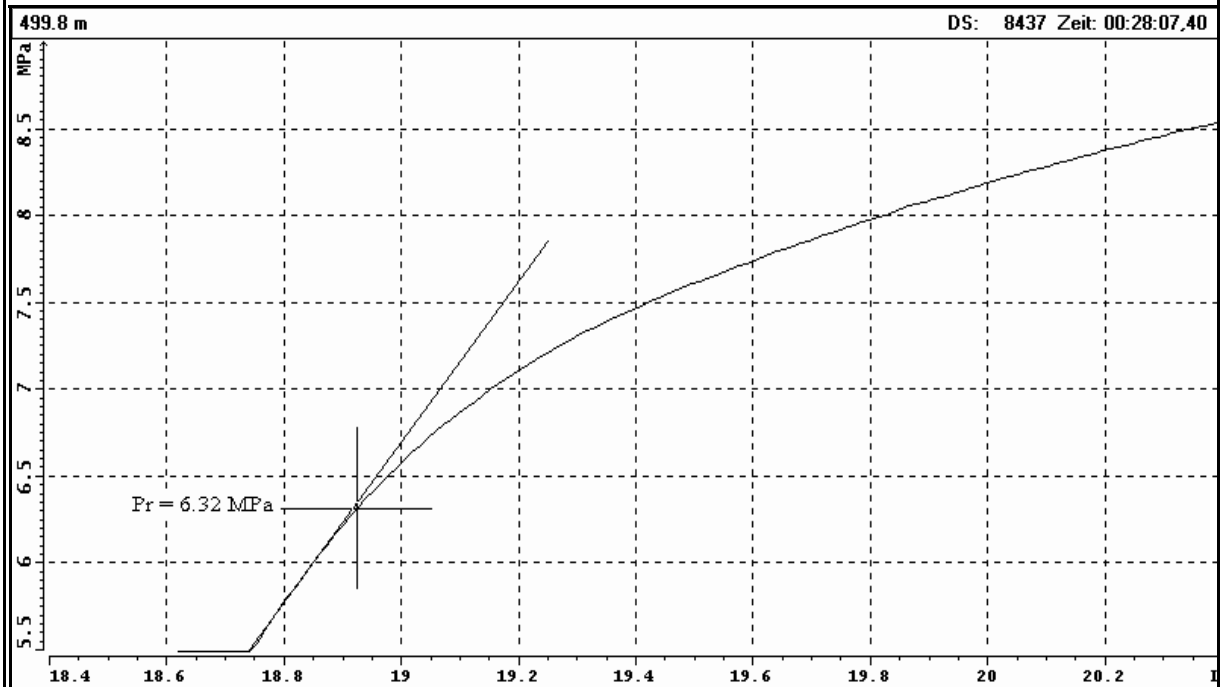




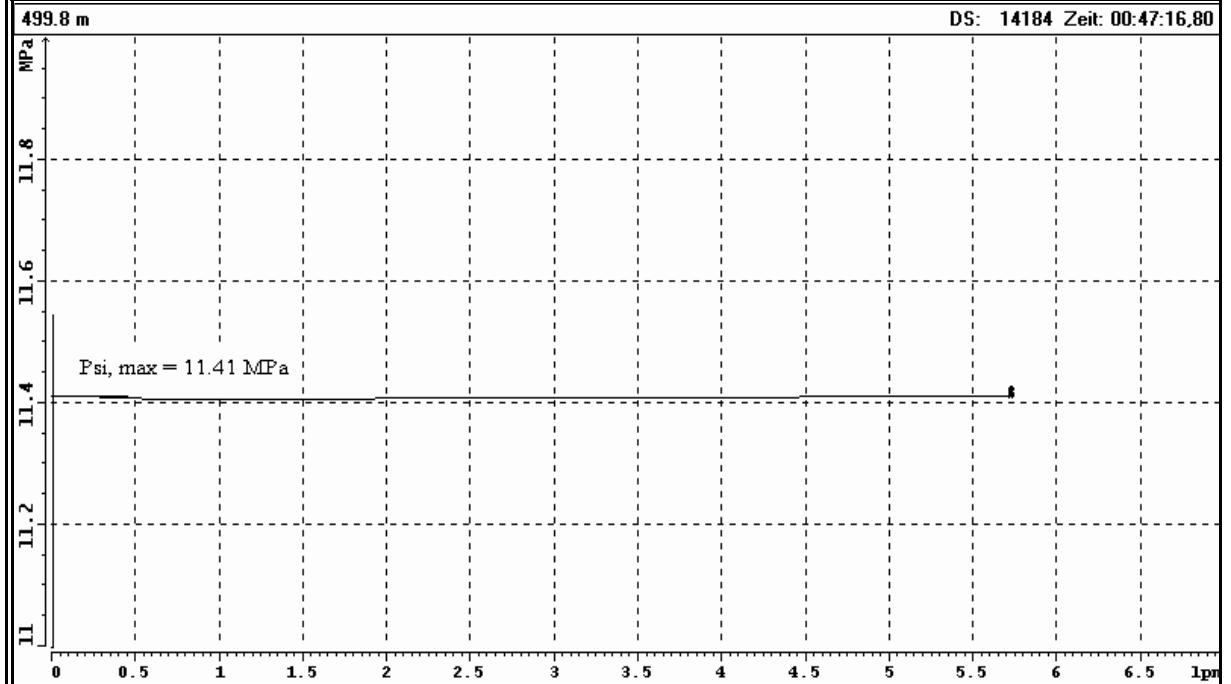
### Test at 499.8 m MD: Estimation of $P_r$ (Frac (1. Refrac) - Cycle)



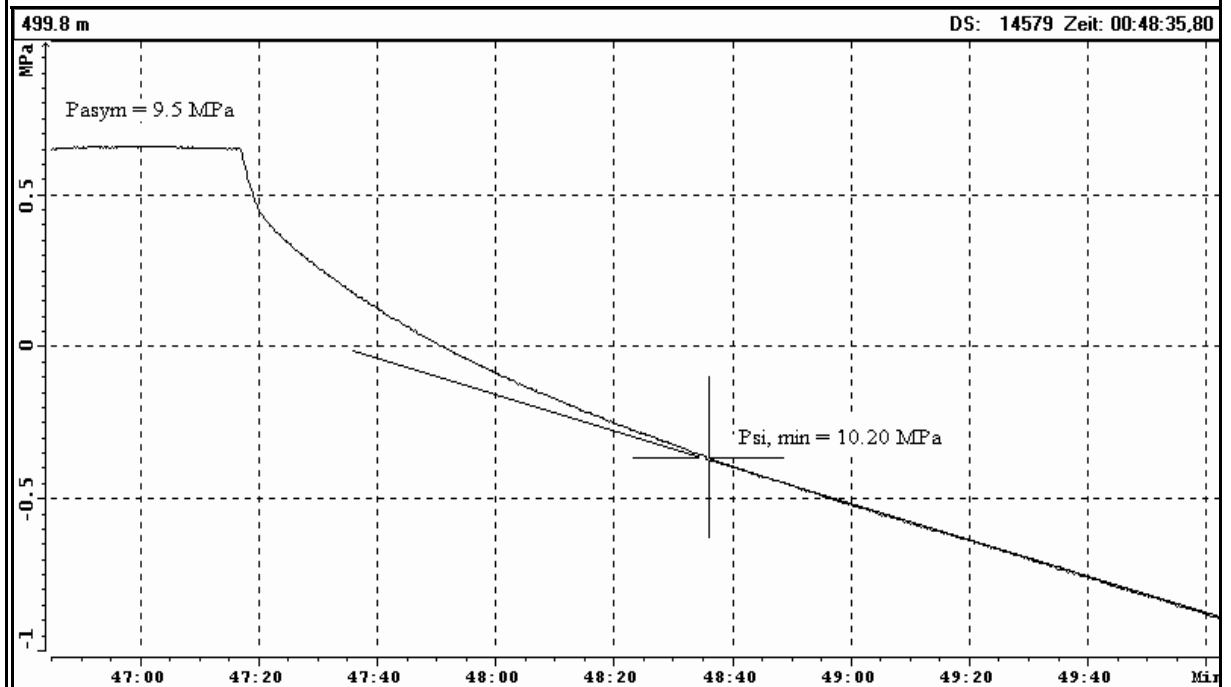
### Test at 499.8 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)



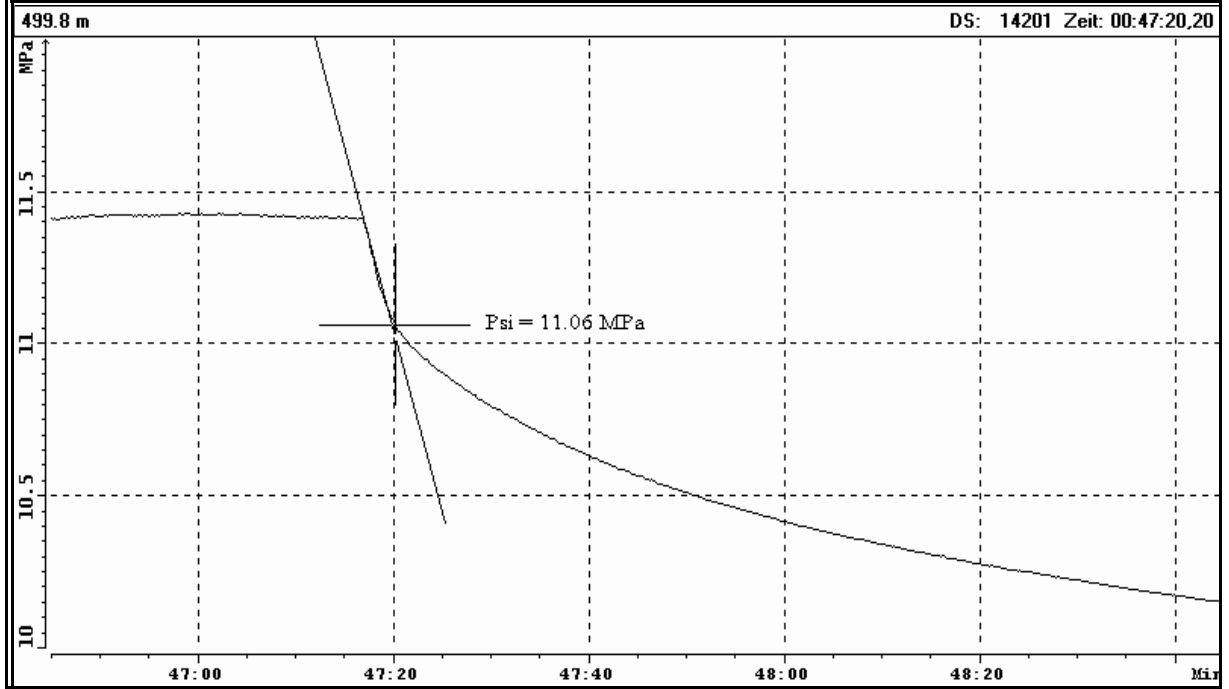
### Test at 499.8 m MD: Estimation of $P_{si, max}$ (3. Refrac - Cycle)



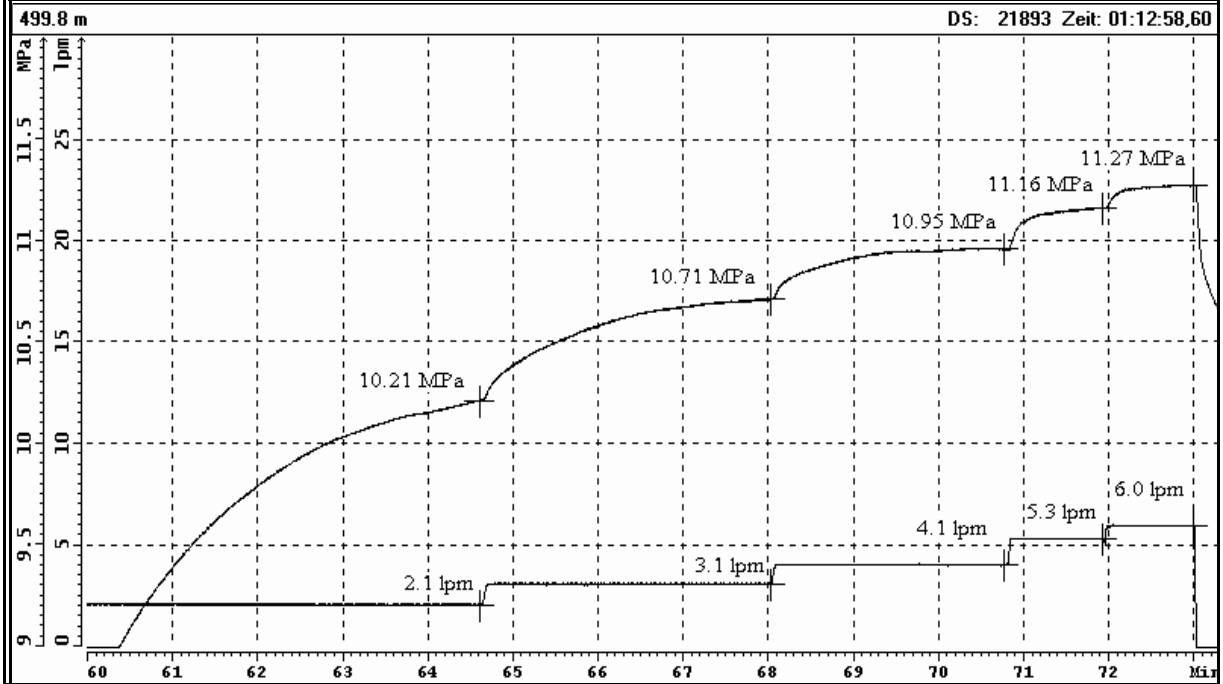
### Test at 499.8 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)



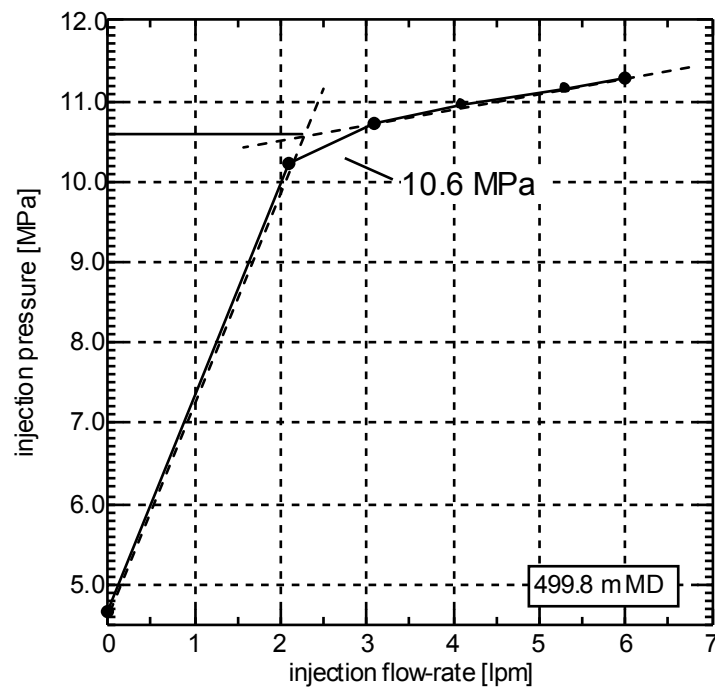
### Test at 499.8 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 499.8 m MD: Analysis of Slow - Pump / Step - Rate - Test



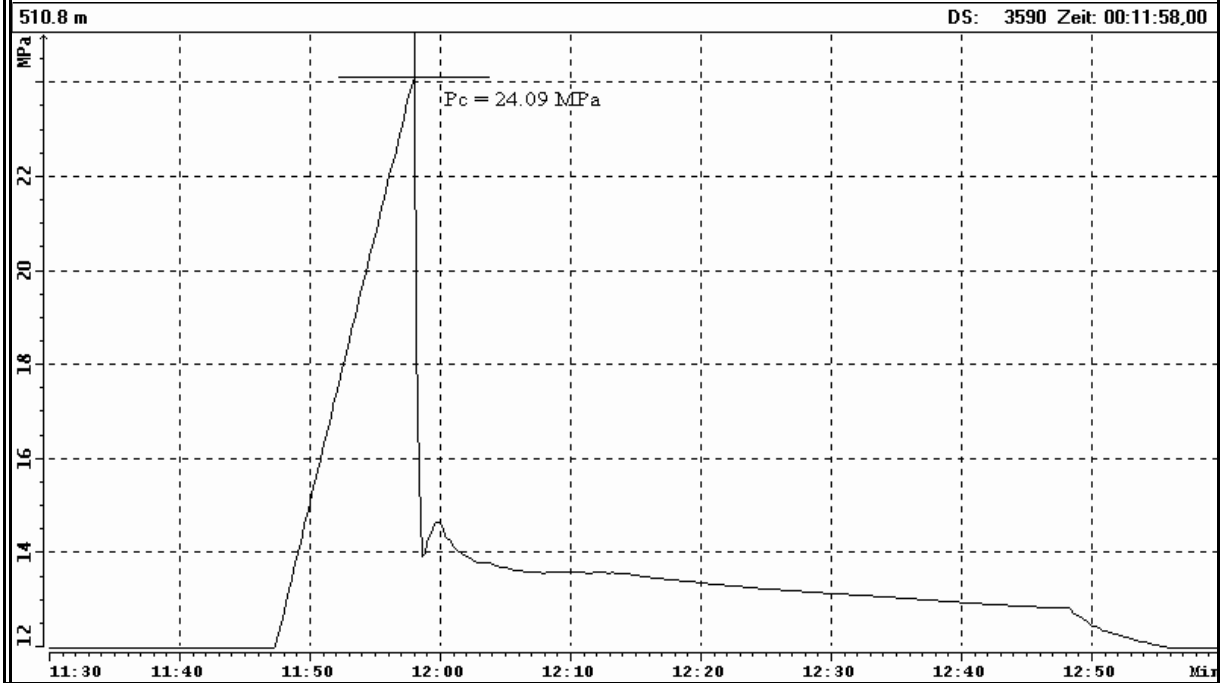
### Test at 499.8 m MD: Examination of $P_{si}$ (Step - Rate - Test)



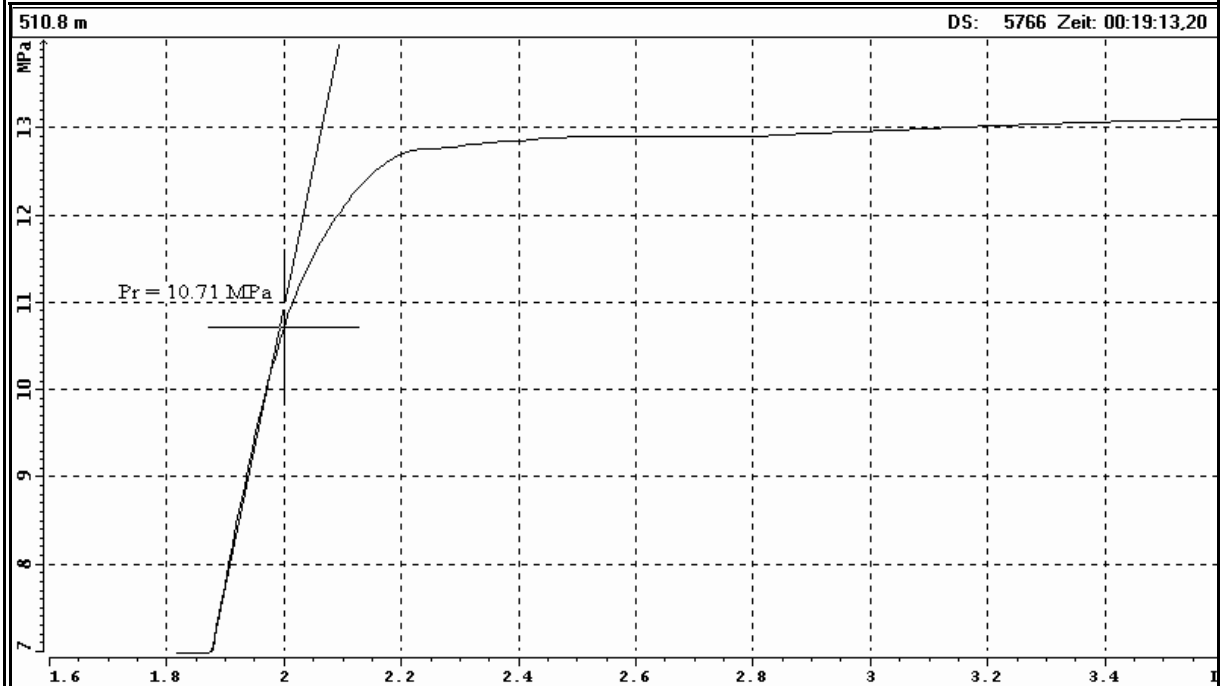




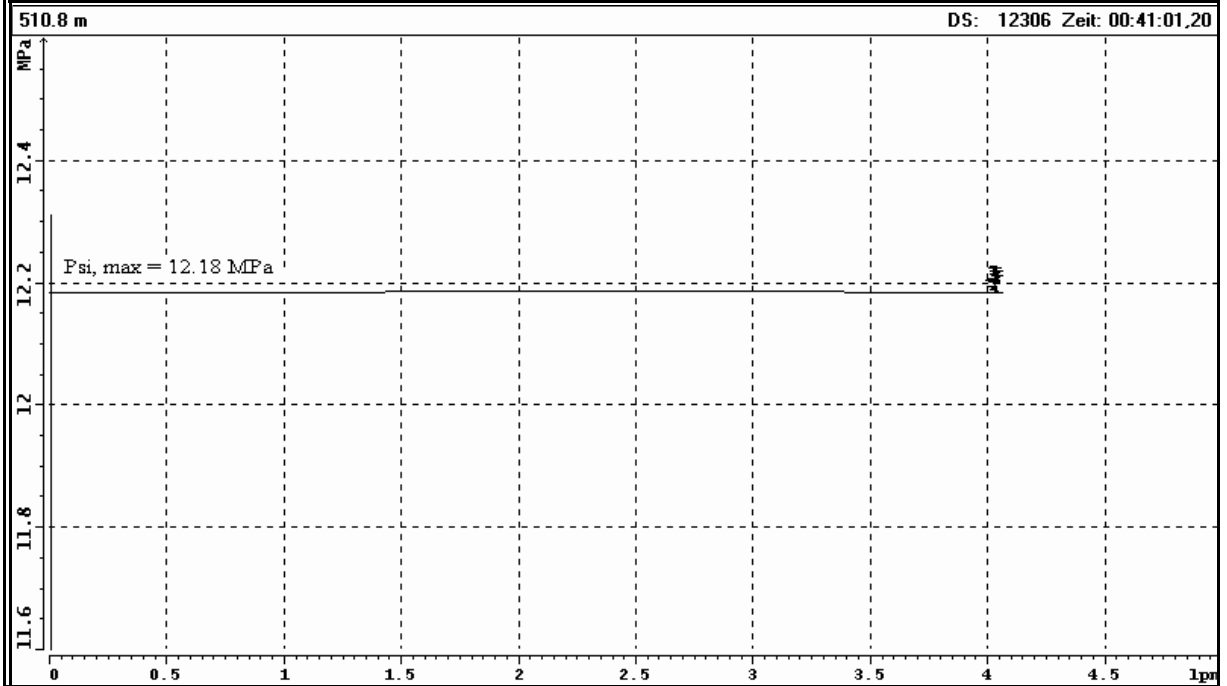
### Test at 510.8 m MD: Estimation of $P_c$ (Frac - Cycle)



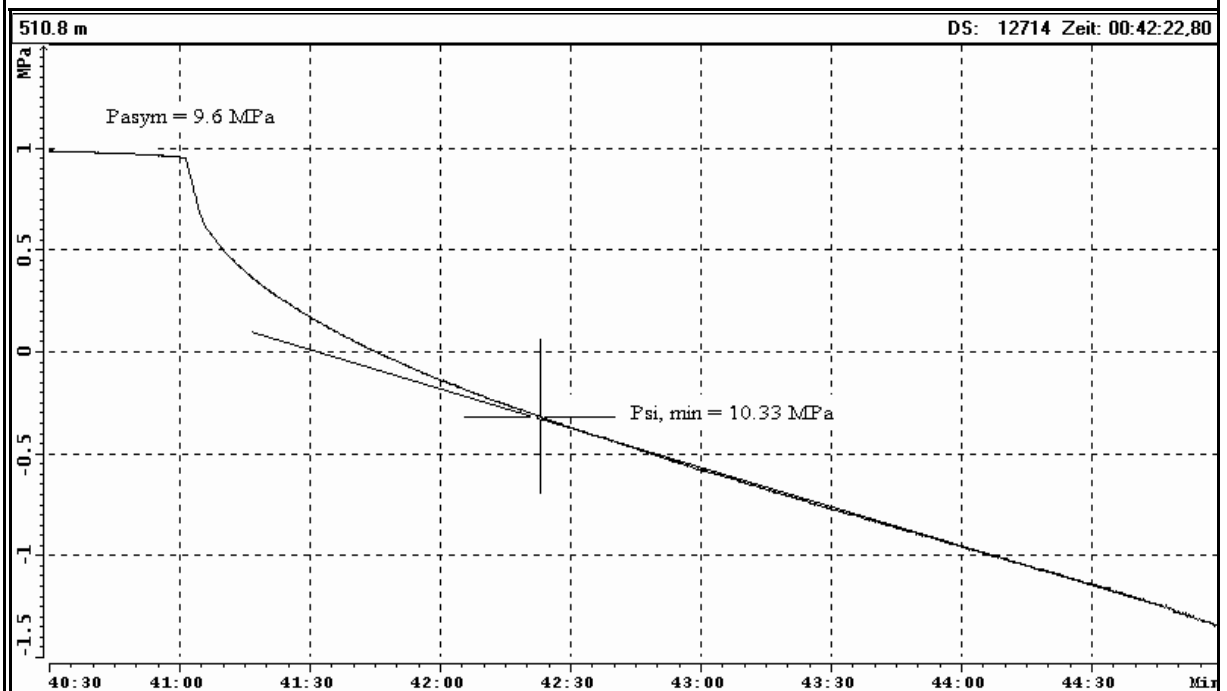
### Test at 510.8 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



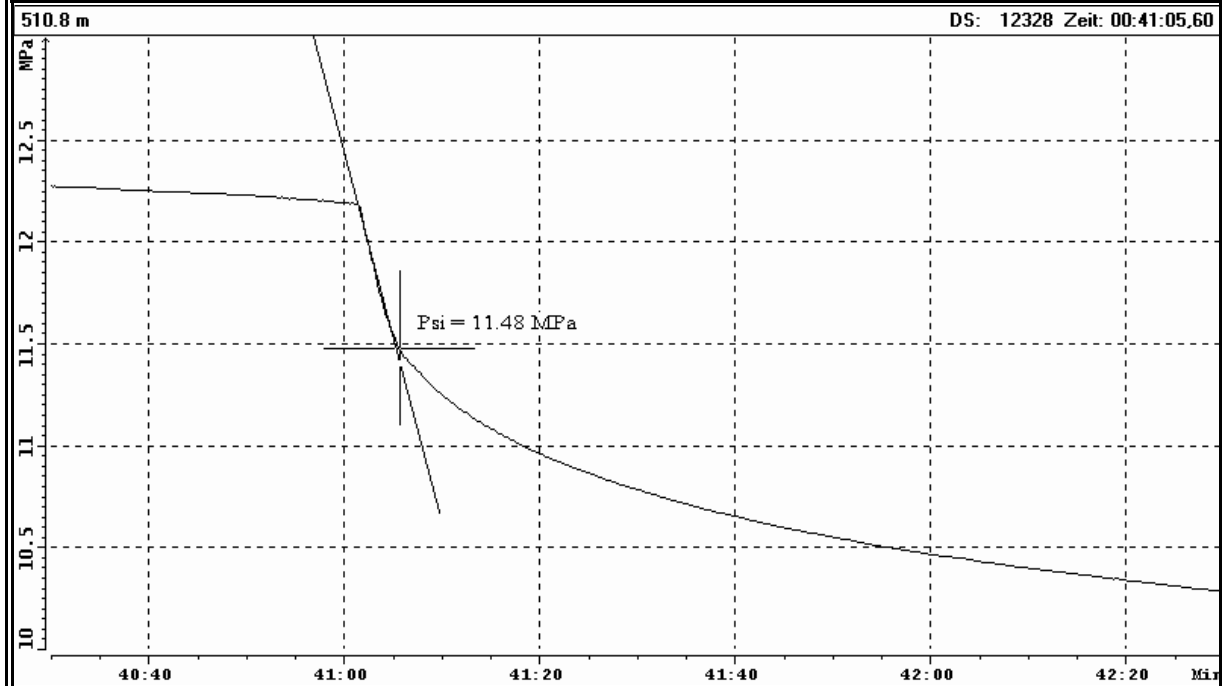
### Test at 510.8 m MD: Estimation of $P_{si, max}$ (3. Refrac - Cycle)



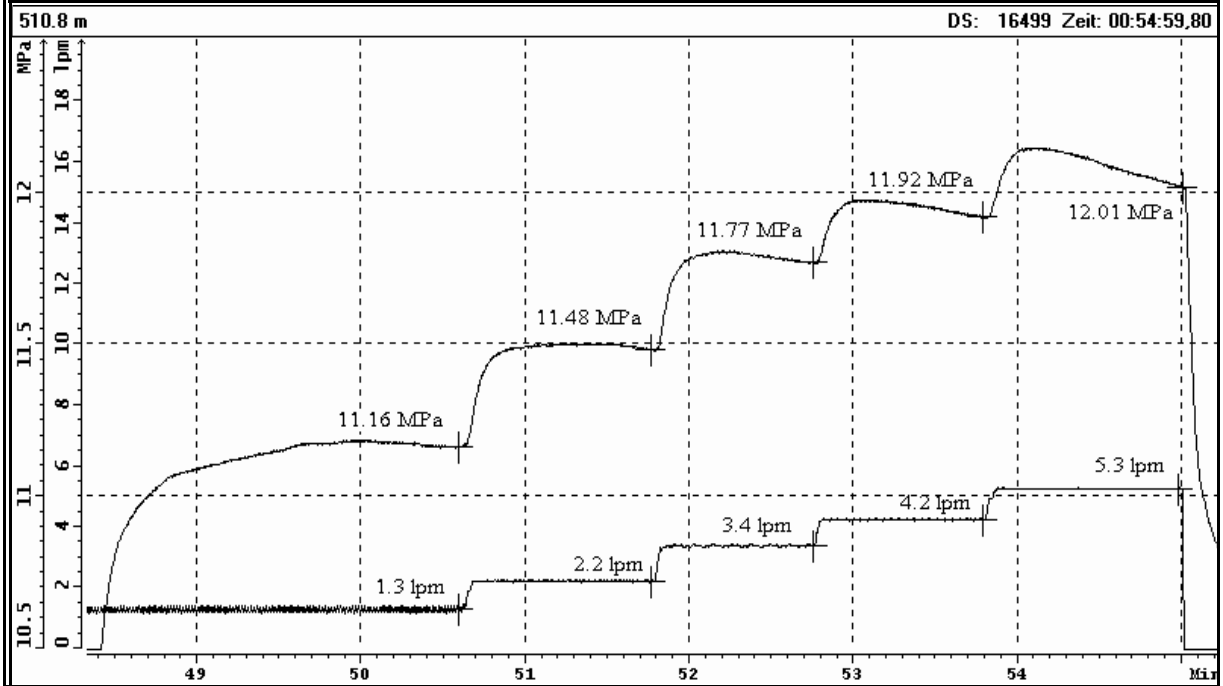
### Test at 510.8 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)



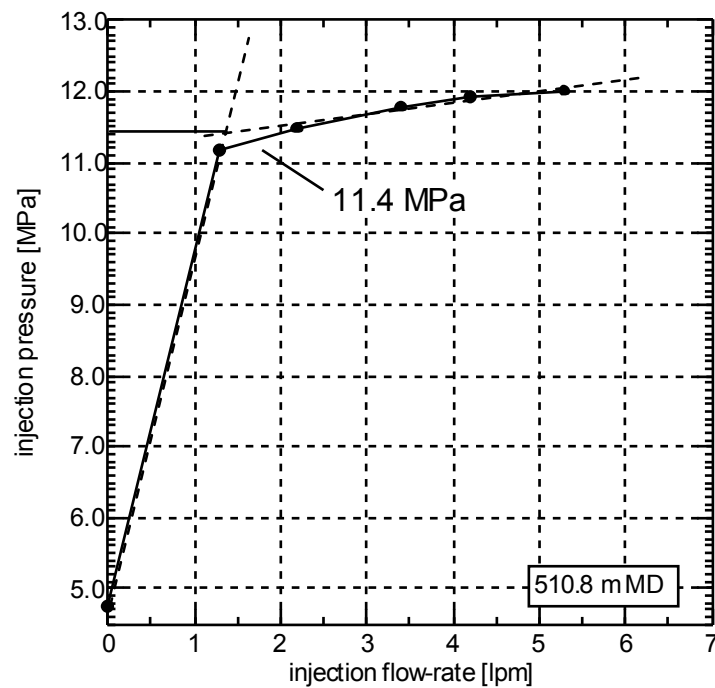
### Test at 510.8 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 510.8 m MD: Analysis of Slow - Pump / Step - Rate - Test

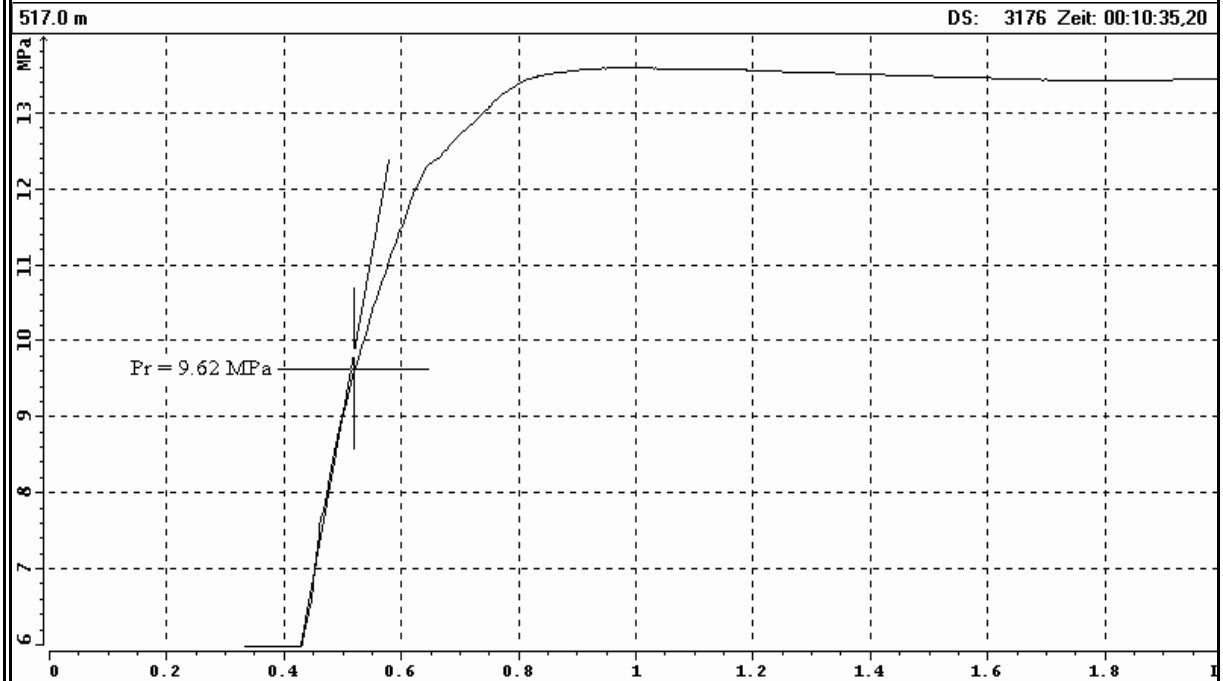


### Test at 510.8 m MD: Examination of $P_{si}$ (Step - Rate - Test)

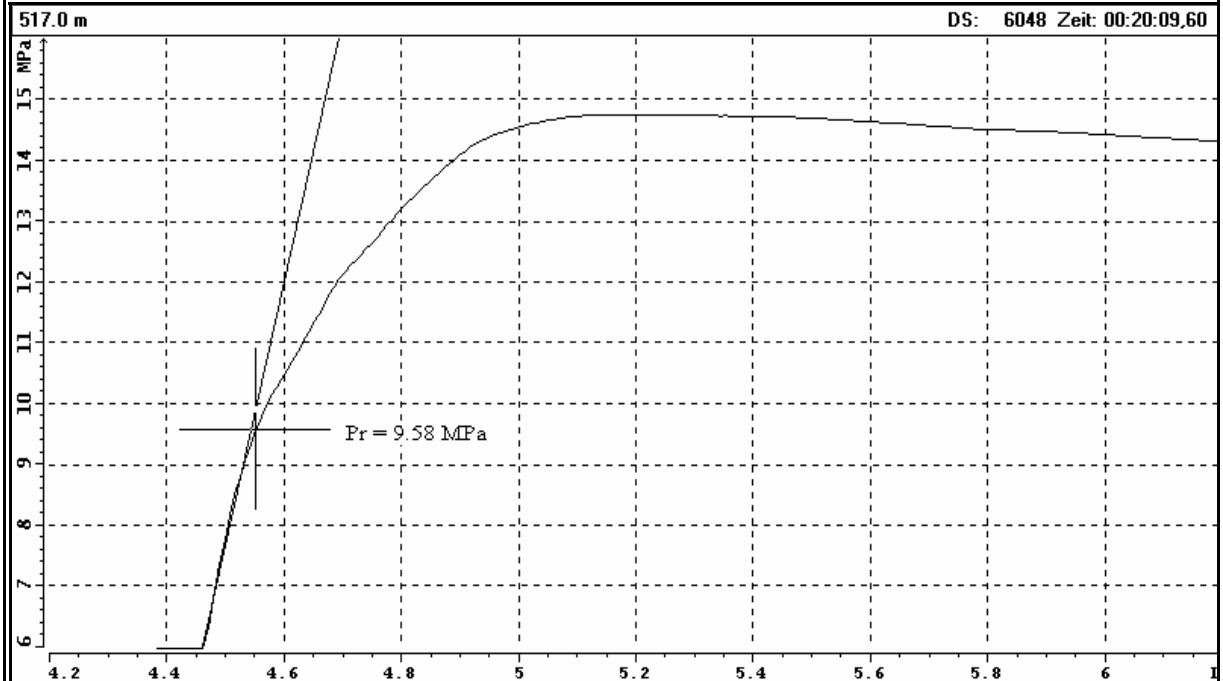




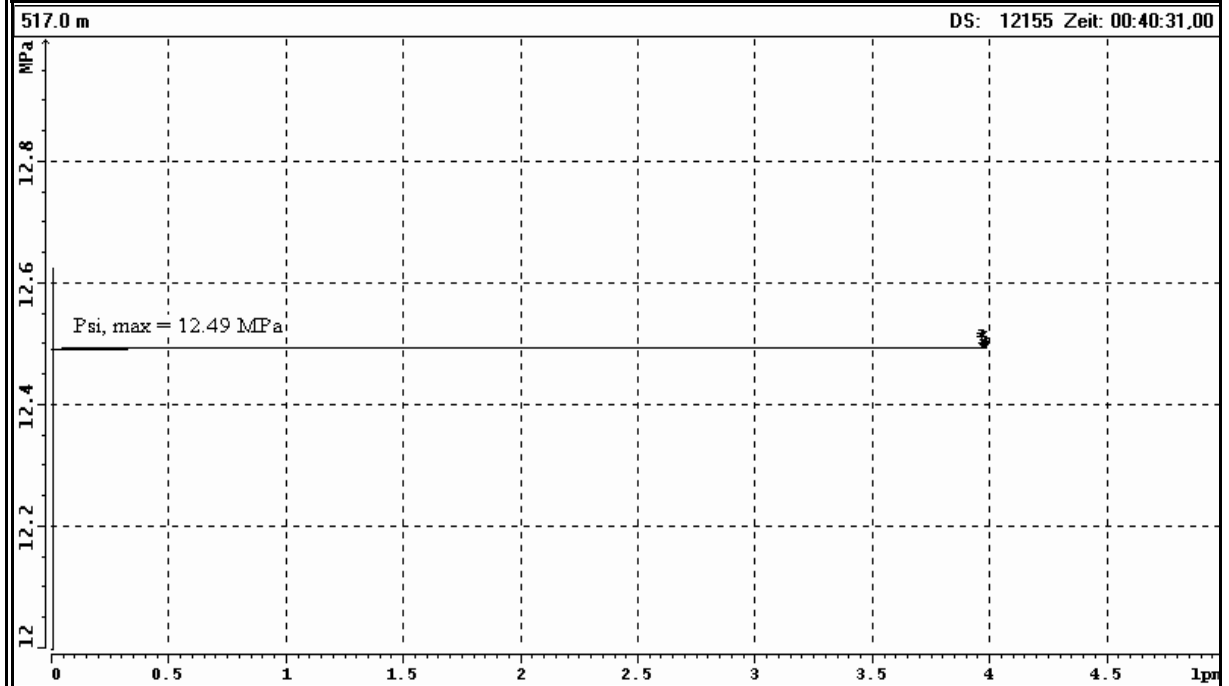
### Test at 517.0 m MD: Estimation of $P_r$ (Frac (1. Refrac) - Cycle)



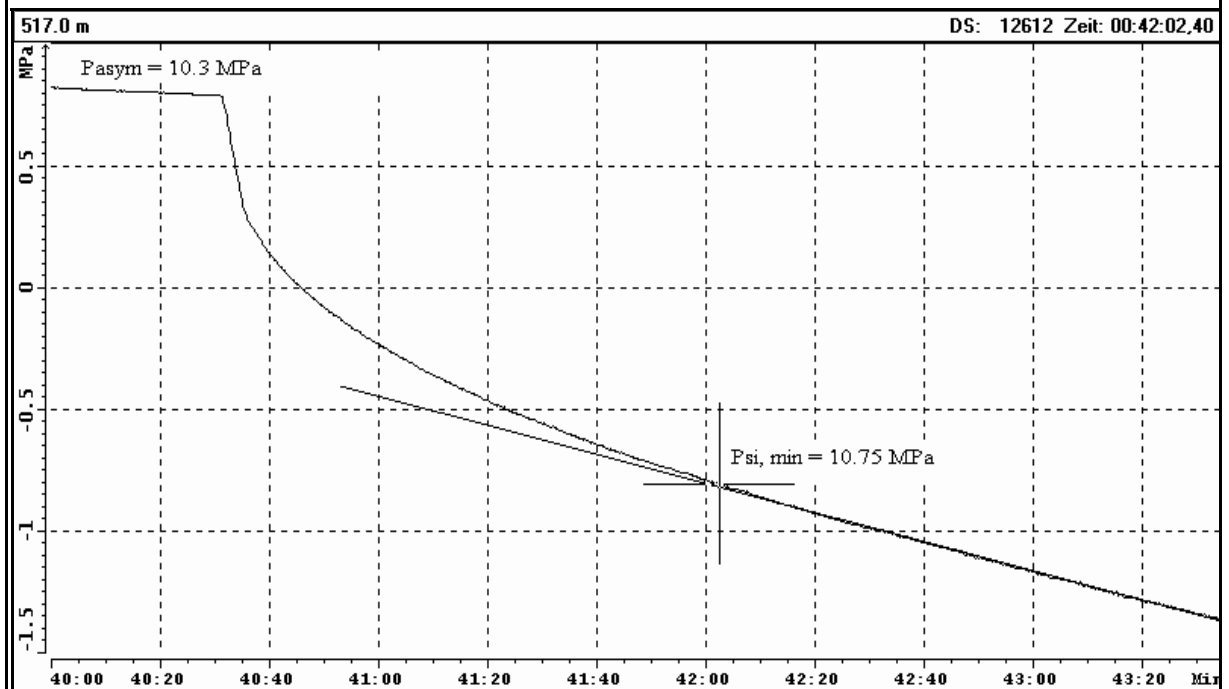
### Test at 517.0 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)



### Test at 517.0 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)

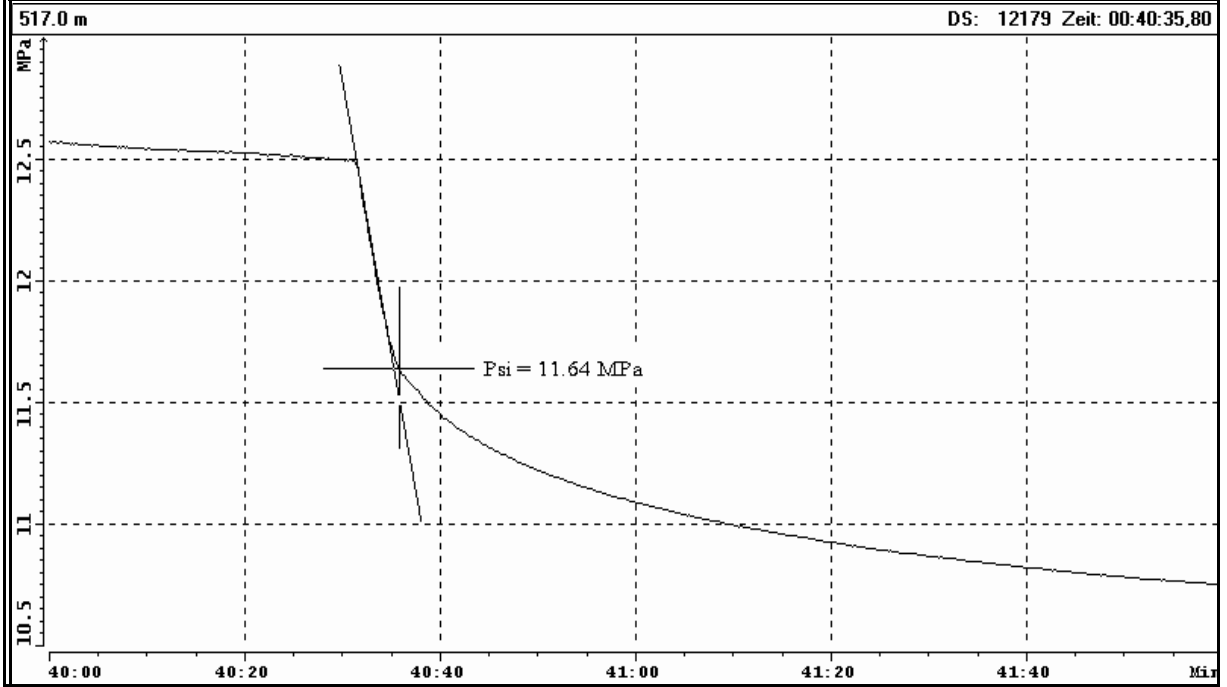


### Test at 517.0 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)

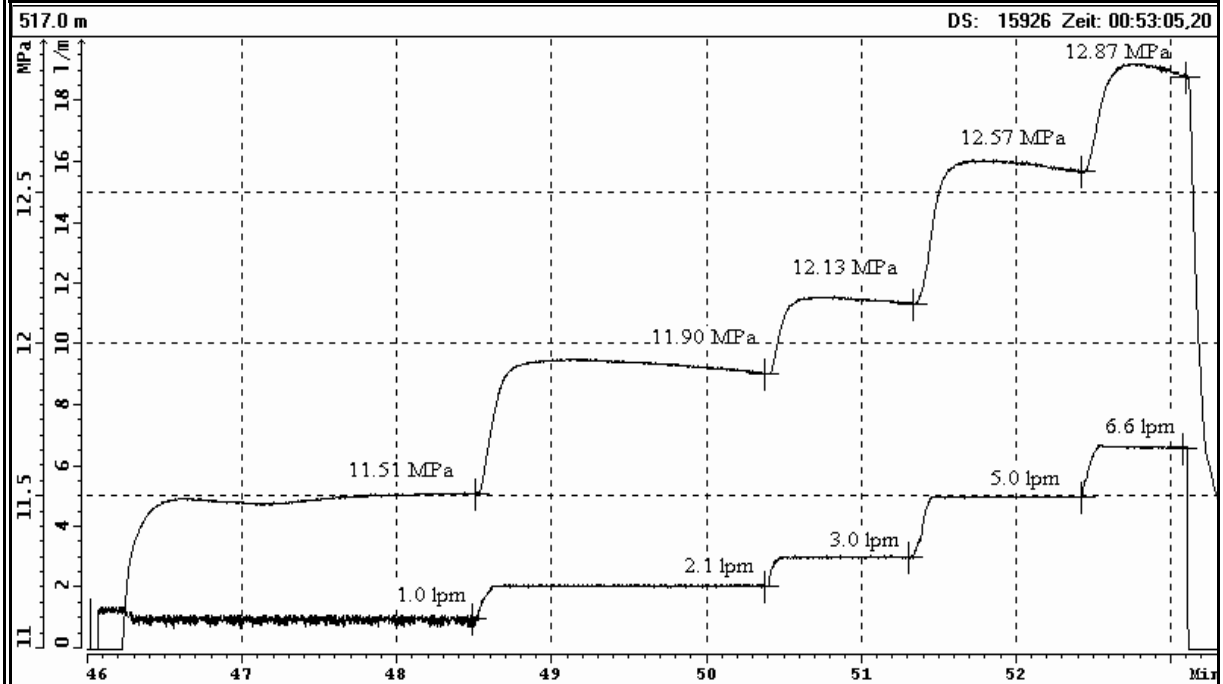




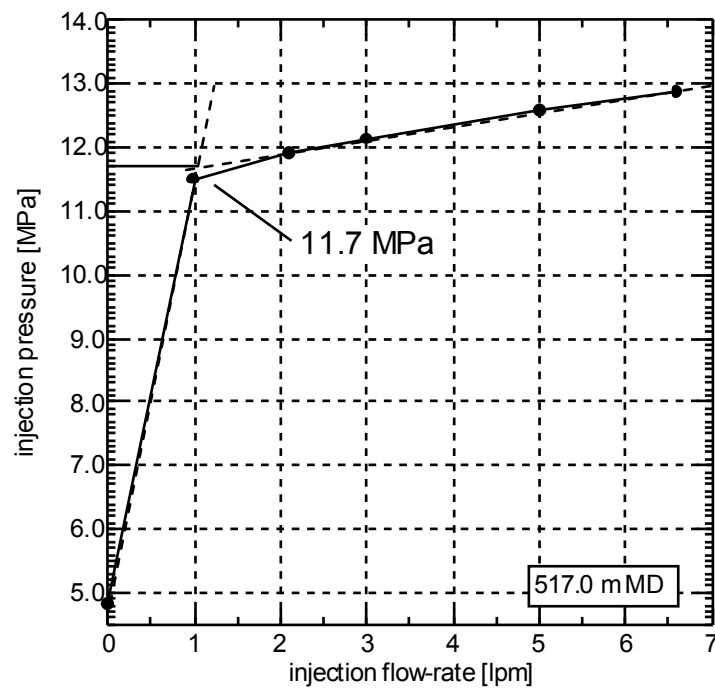
### Test at 517.0 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



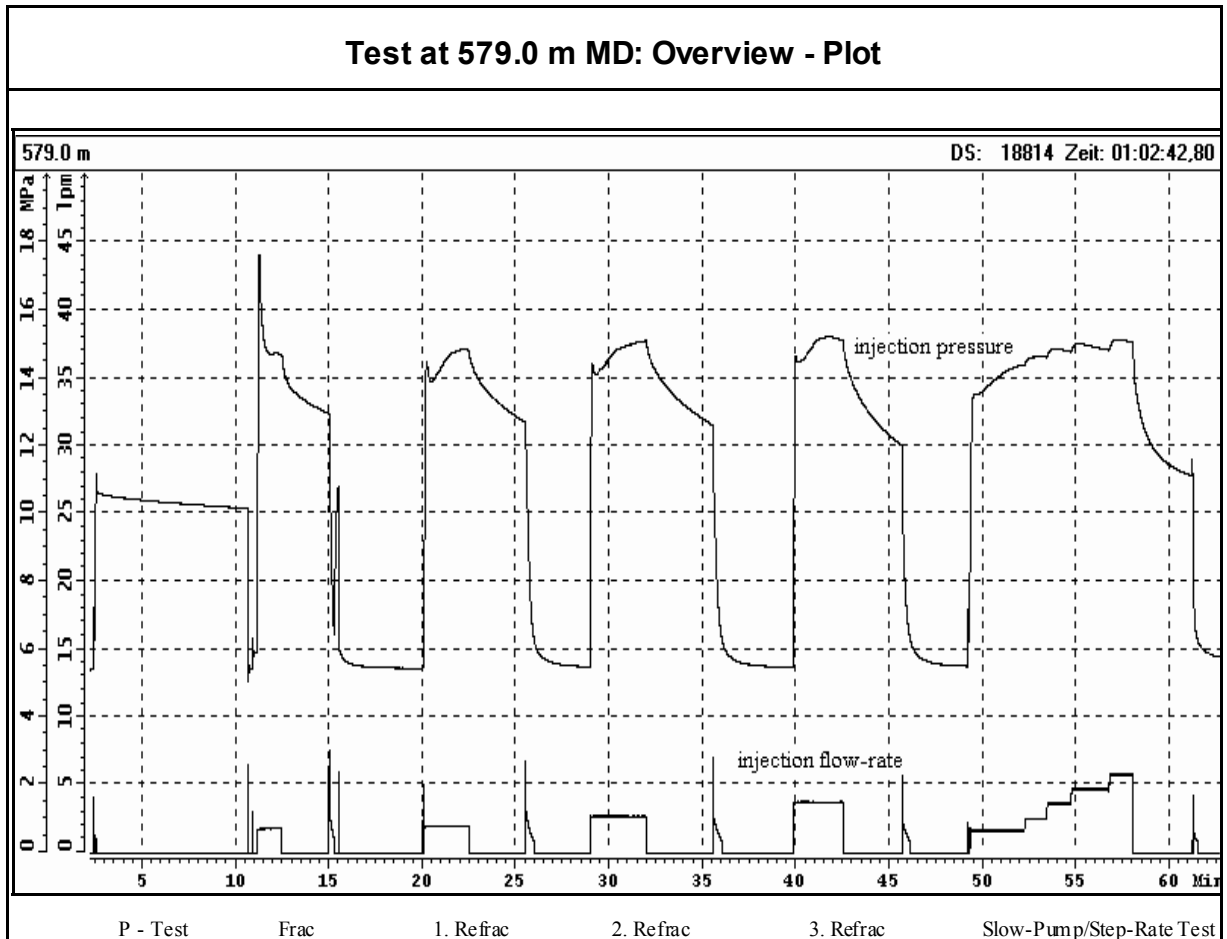
### Test at 517.0 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 517.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST AT 579.0 m MD / 552.2 m TVD

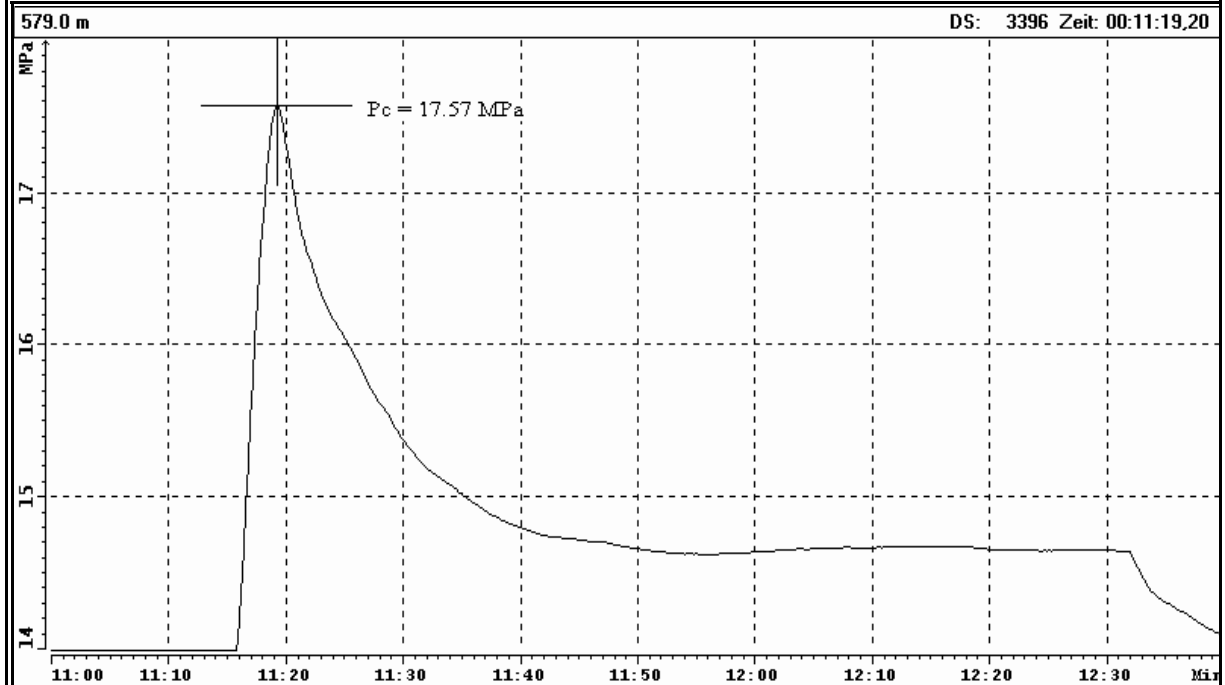


### TEST SUMMARY

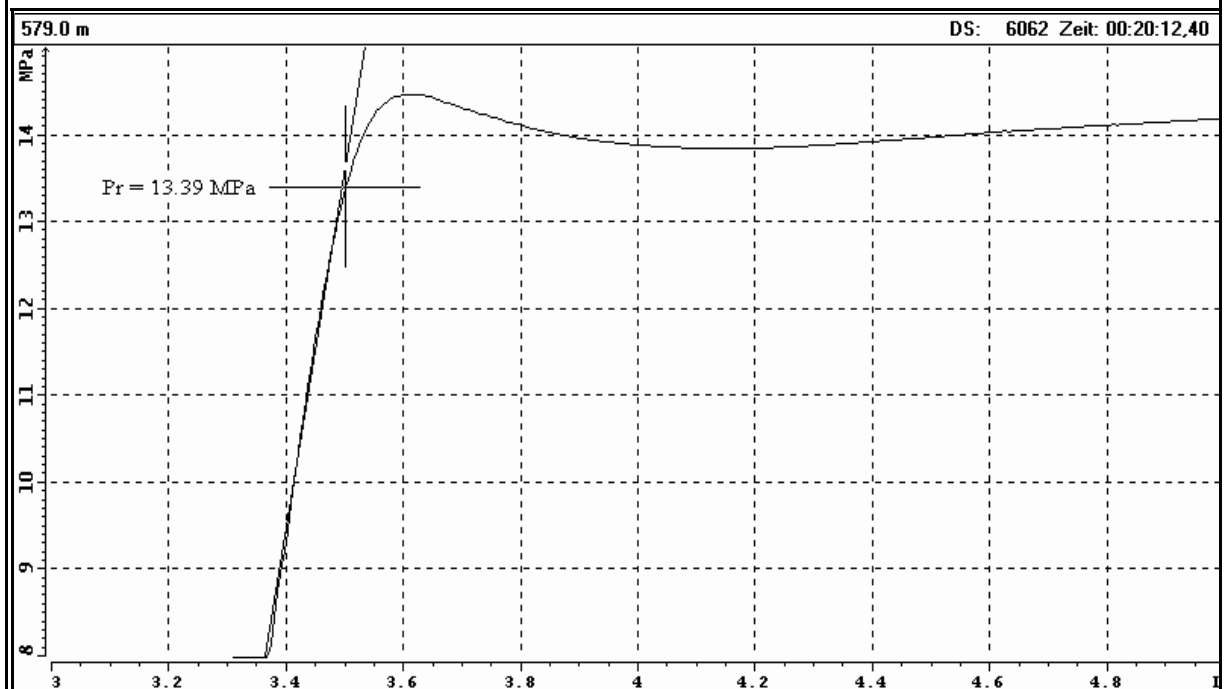
P - Test :	pressure decrease: 0.45 MPa in 7 min. 59 sec.
Frac - Cycle :	injection-rate $Q_i = 1.8$ lpm, injected volume $V_i = 2.4$ l, back-flow volume $V_r = 0.7$ l clear fracture initiation (breakdown event)
1. Refrac - Cycle :	$Q_i = 2.0$ lpm, $V_i = 4.8$ l, $V_r = 0.8$ l
2. Refrac - Cycle :	$Q_i = 2.7$ lpm, $V_i = 8.1$ l, $V_r = 0.9$ l
3. Refrac - Cycle :	$Q_i = 3.7$ lpm, $V_i = 9.9$ l, $V_r = 0.7$ l
Step-Rate Test :	$Q_i = 1.6$ - $5.8$ lpm, $V_i = 29.4$ l, $V_r = 0.4$ l

total injected volume = 54.9 l, recovered volume = 3.5 l (6.4 %)

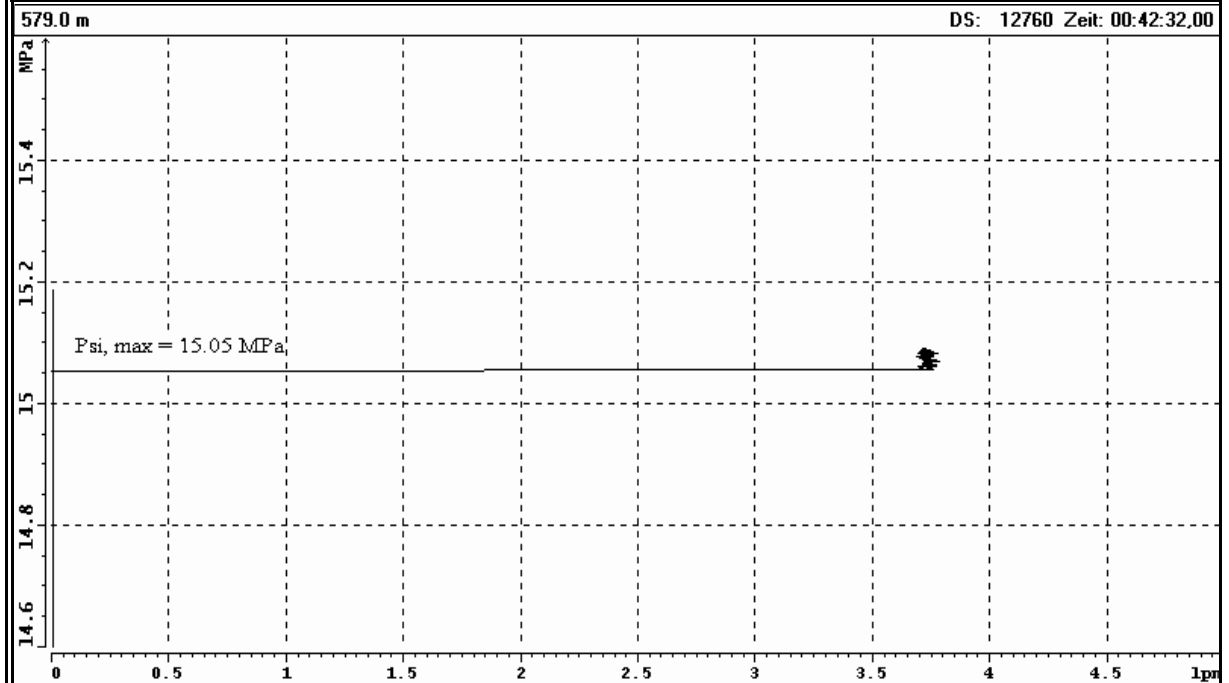
### Test at 579.0 m MD: Estimation of $P_c$ (Frac - Cycle)



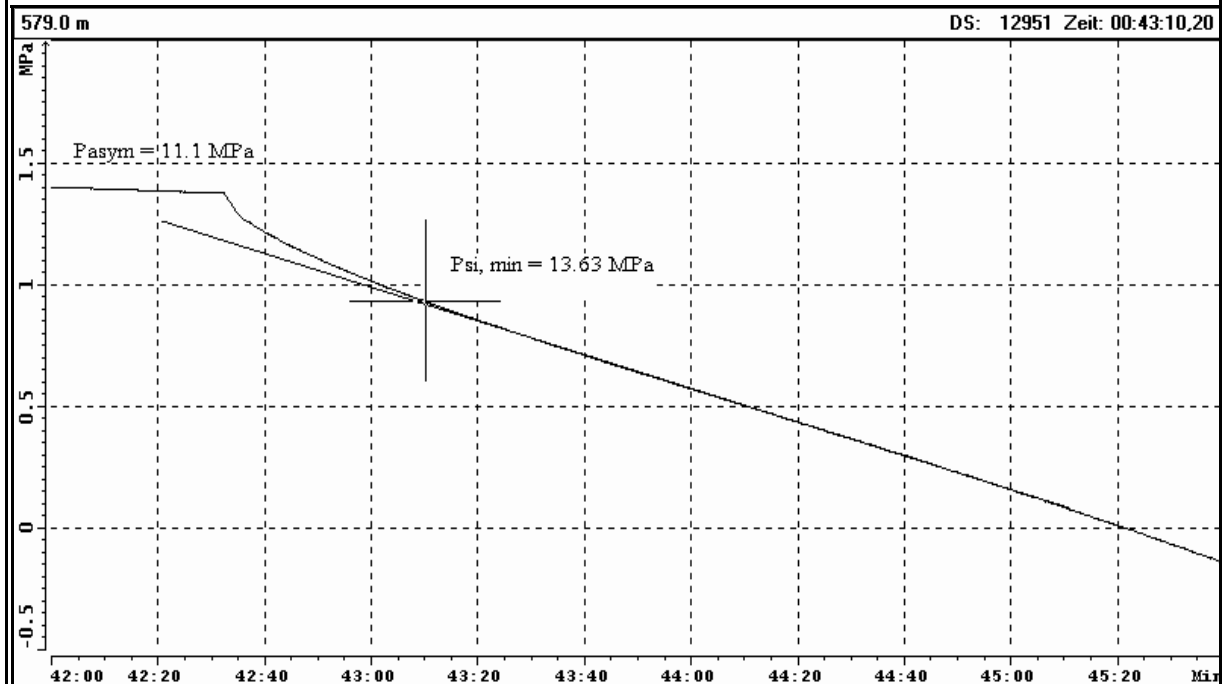
### Test at 579.0 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



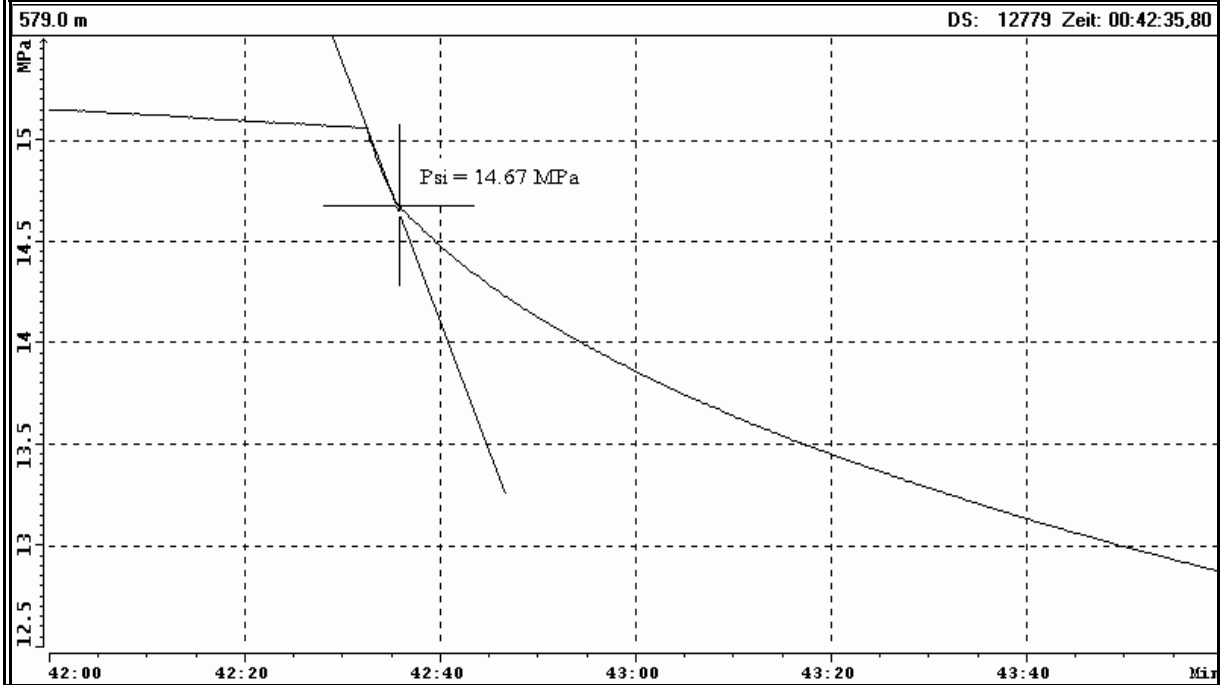
### Test at 579.0 m MD: Estimation of $P_{si, max}$ (3. Refrac - Cycle)



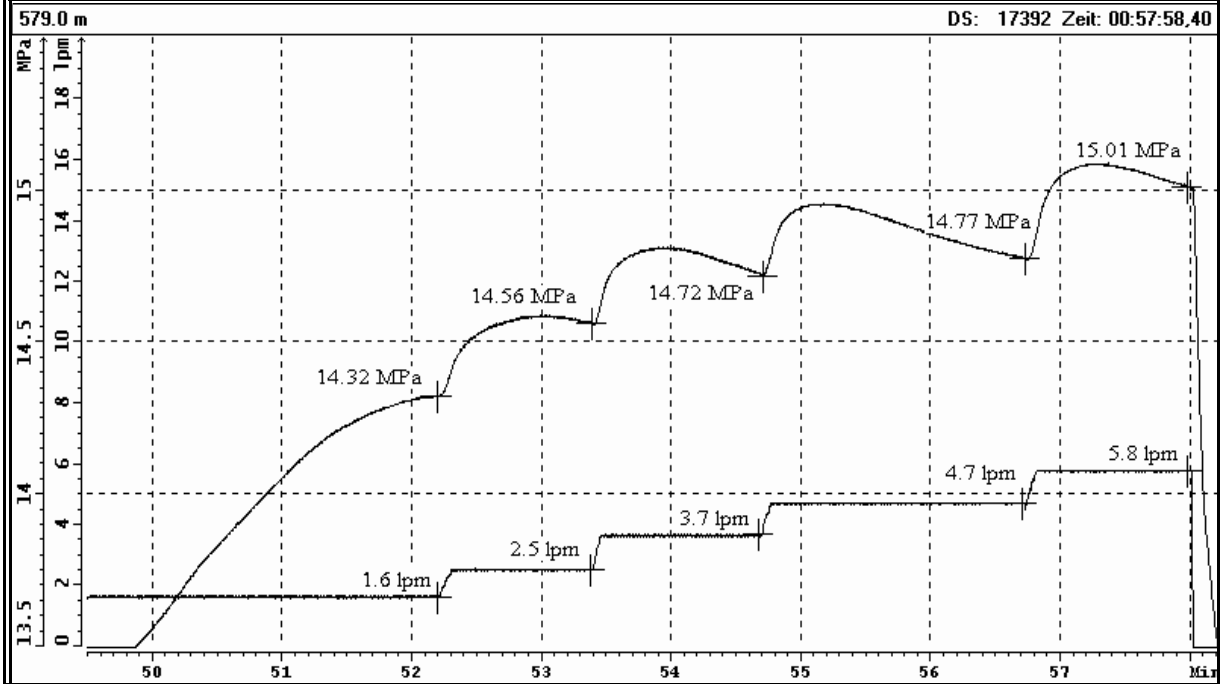
### Test at 579.0 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)



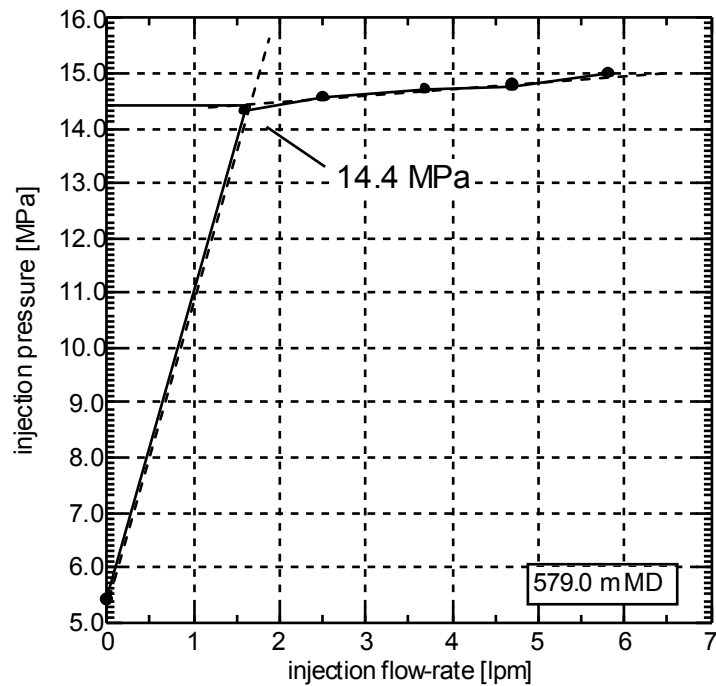
### Test at 579.0 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 579.0 m MD: Analysis of Slow - Pump / Step - Rate - Test



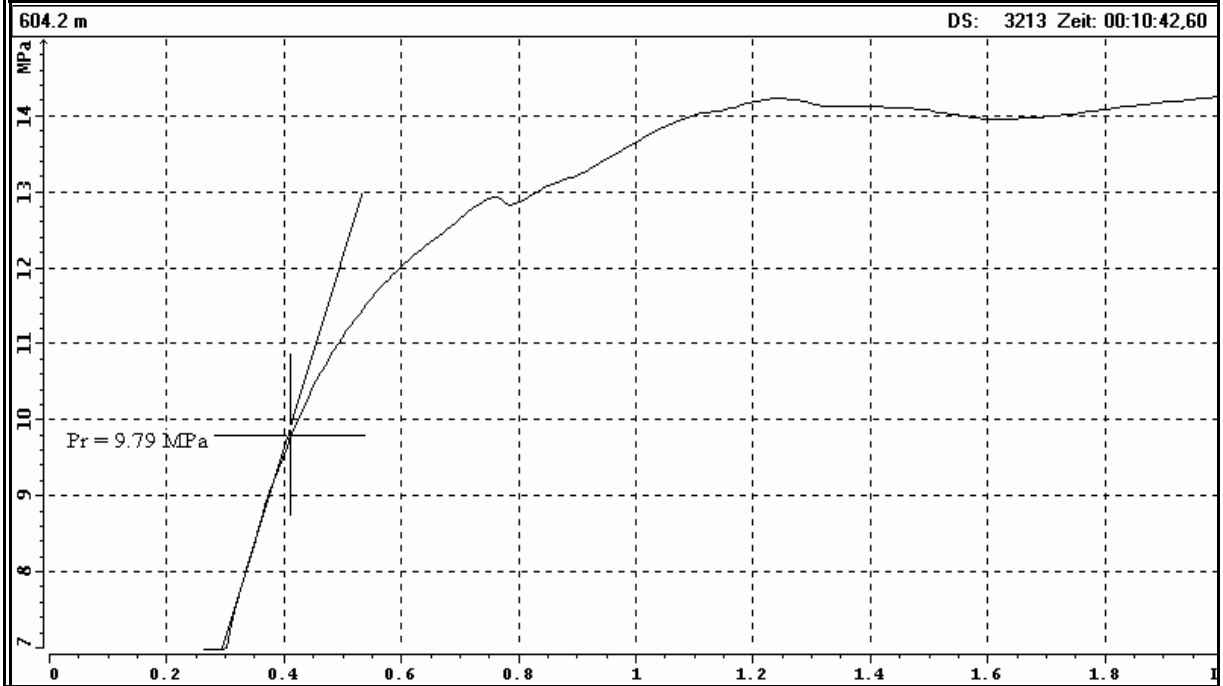
### Test at 579.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)



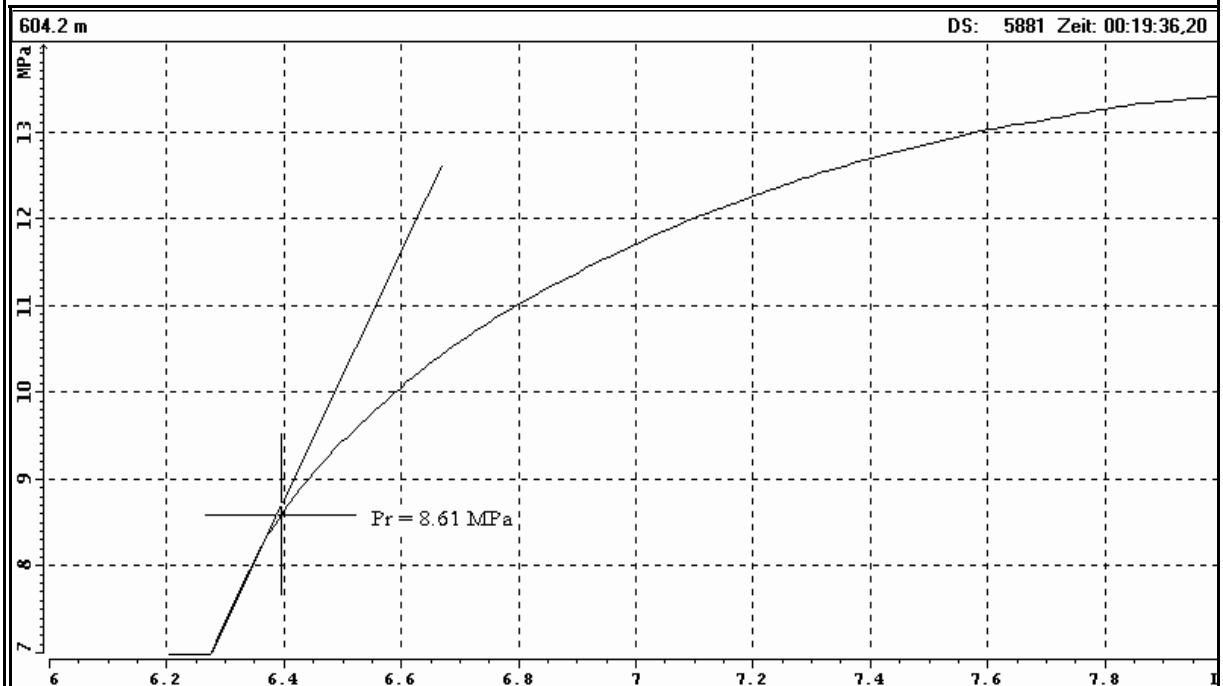




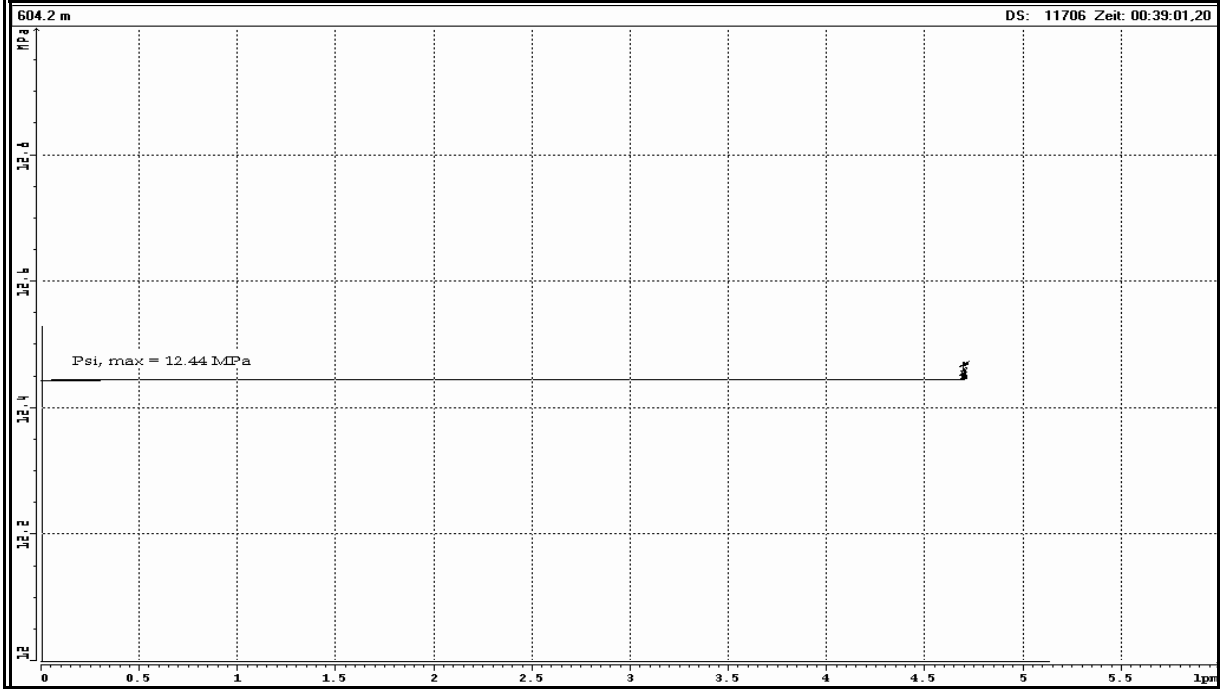
### Test at 604.2 m MD: Estimation of $P_r$ (1. Refrac) - Cycle



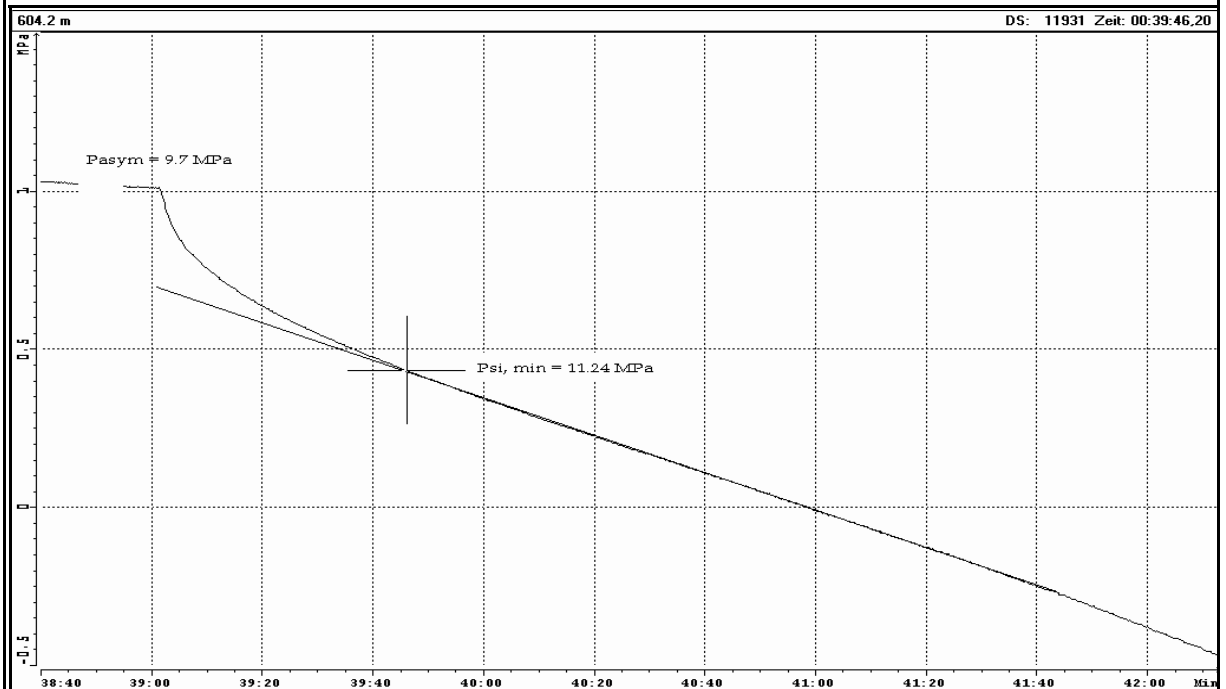
### Test at 604.2 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)



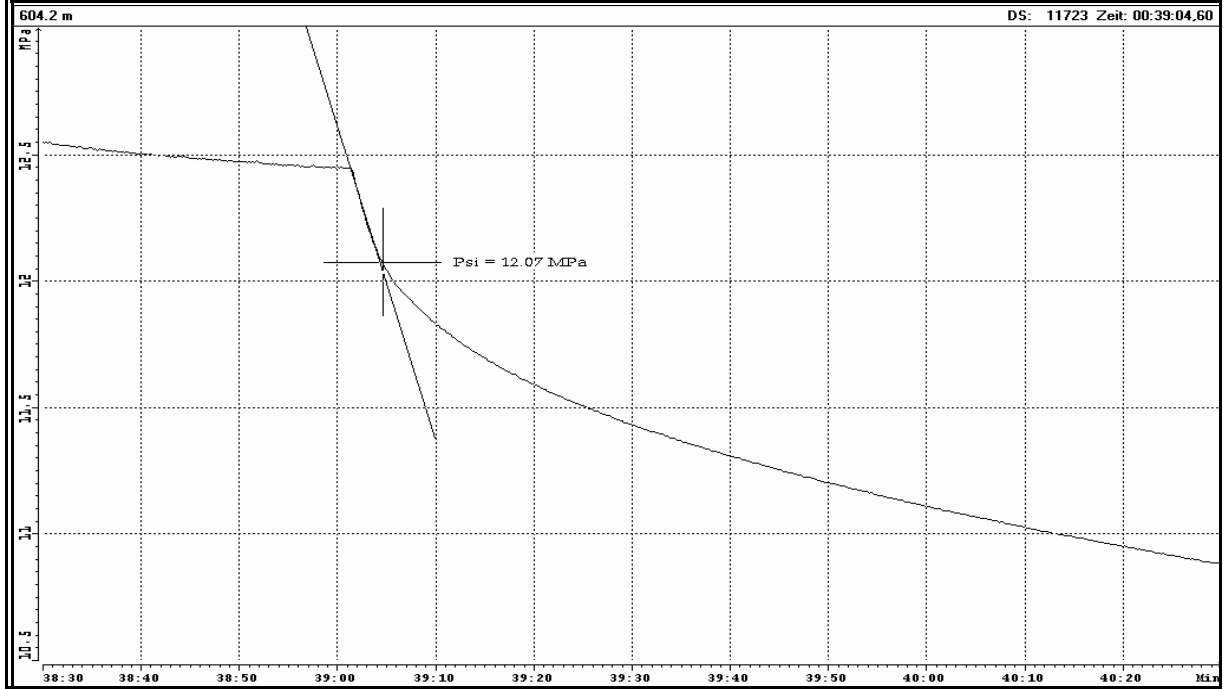
### Test at 604.2 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



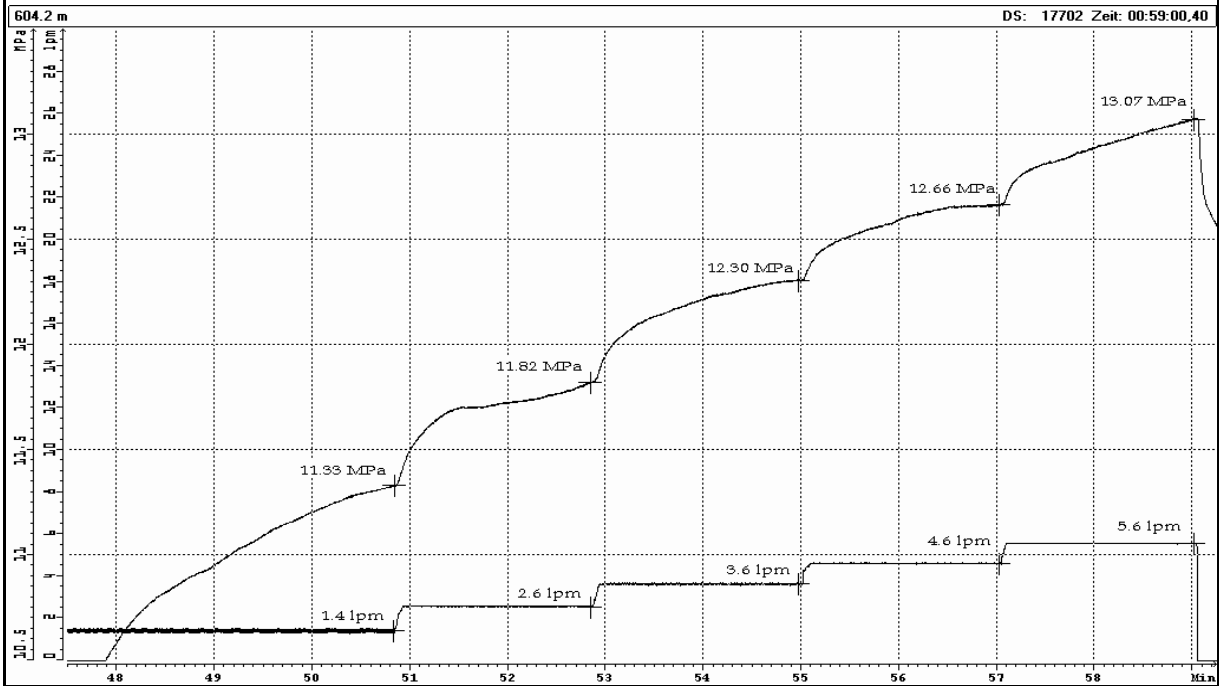
### Test at 604.2 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)



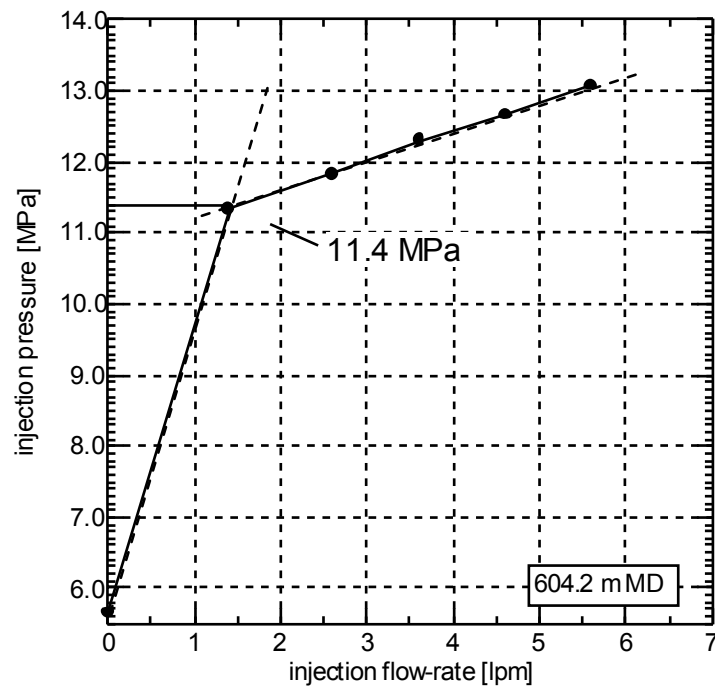
# Test at 604.2 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



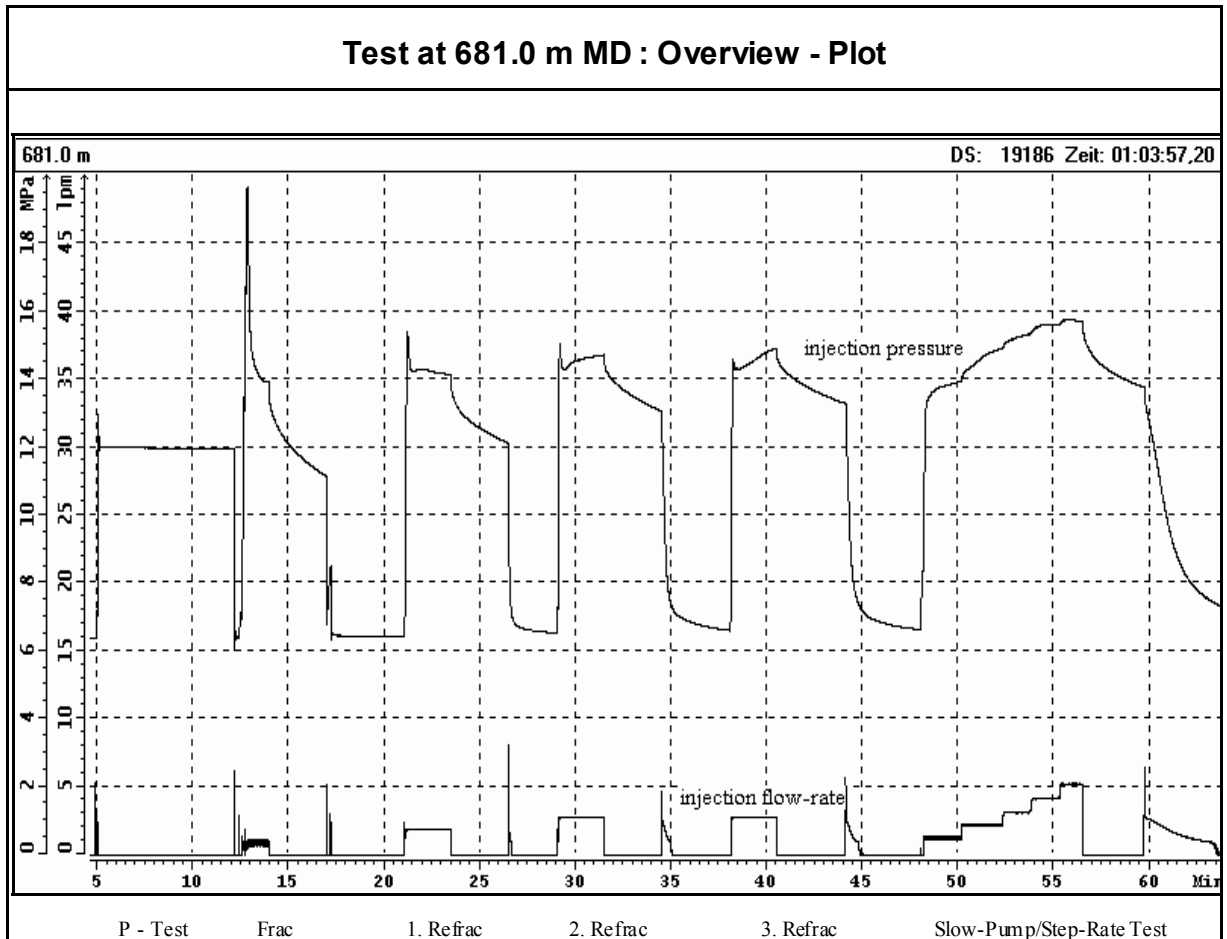
### Test at 604.2 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 604.2 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST AT 681.0 m MD / 652.9 m TVD



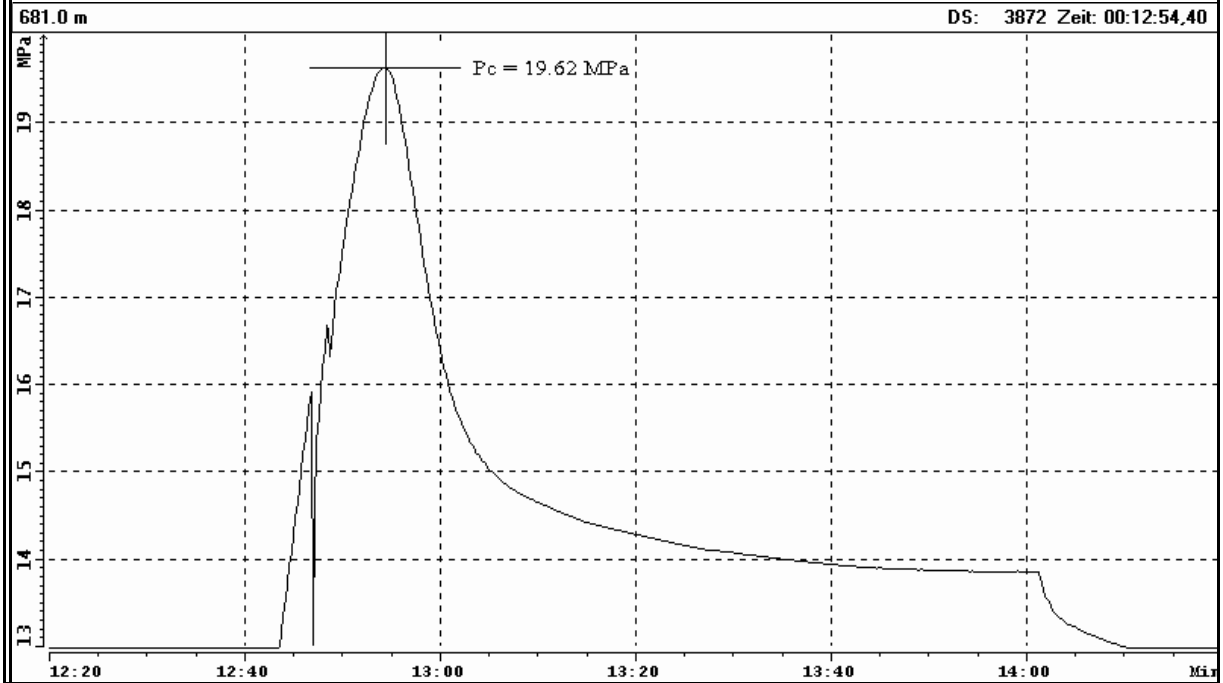
### TEST SUMMARY

P - Test :	pressure decrease: 0.1 MPa in 6 min. 58 sec.
Frac - Cycle :	injection - rate $Q_i = 1.0$ lpm, injected volume $V_i = 1.3$ l, back-flow volume $V_r = 0.2$ l clear fracture initiation (breakdown event)
1. Refrac - Cycle :	$Q_i = 1.9$ lpm, $V_i = 4.6$ l, $V_r = 0.3$ l
2. Refrac - Cycle :	$Q_i = 2.8$ lpm, $V_i = 6.7$ l, $V_r = 0.8$ l
3. Refrac - Cycle :	$Q_i = 2.8$ lpm, $V_i = 6.5$ l, $V_r = 1.1$ l
Step-Rate Test :	$Q_i = 1.3$ - $5.2$ lpm, $V_i = 24.5$ l, $V_r = 6.2$ l

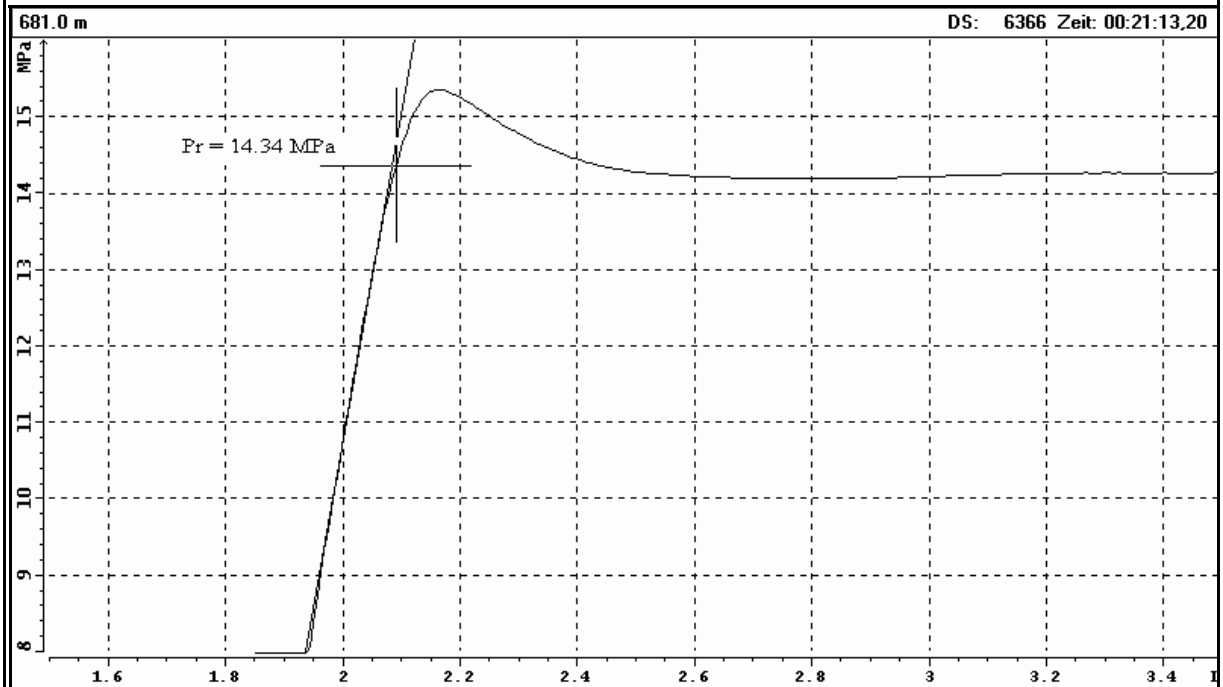
total injected volume = 43.6 l, recovered volume = 8.6 l (19.7 %)

*Remark: During the injection - cycles, an increase of the shut-in pressure was observed. The  $P_{si}$  - value was therefore determined from the 1. refrac - cycle.*

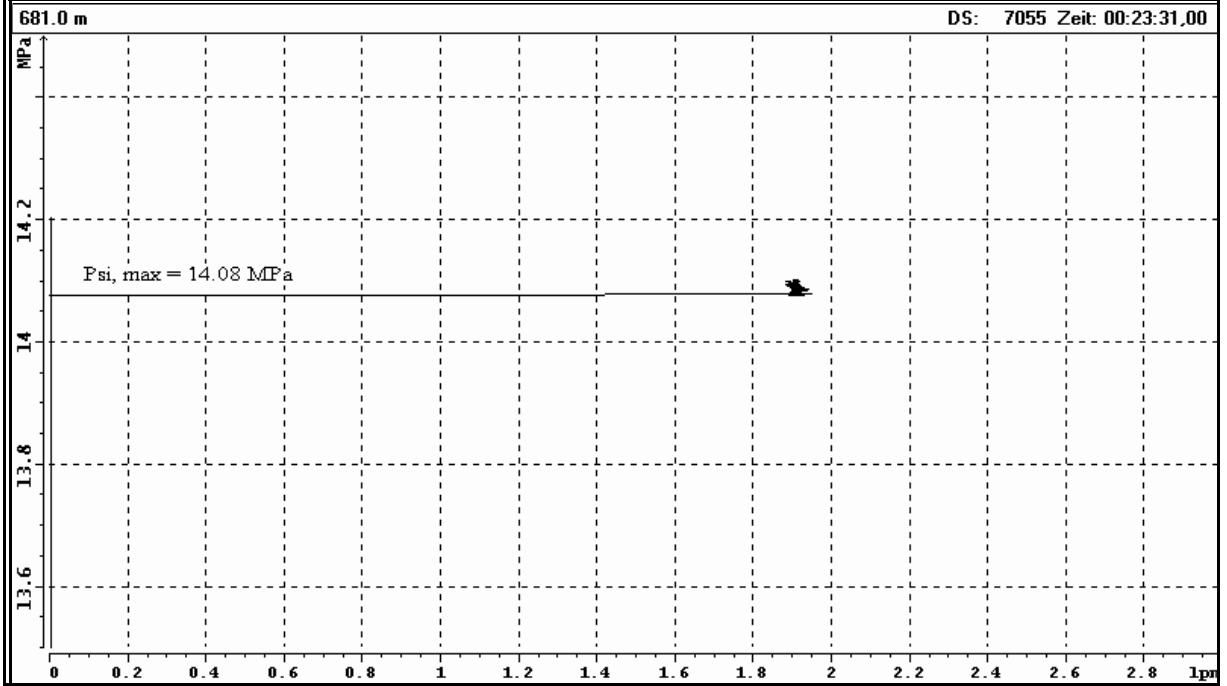
### Test at 681.0 m MD: Estimation of $P_c$ (Frac - Cycle)



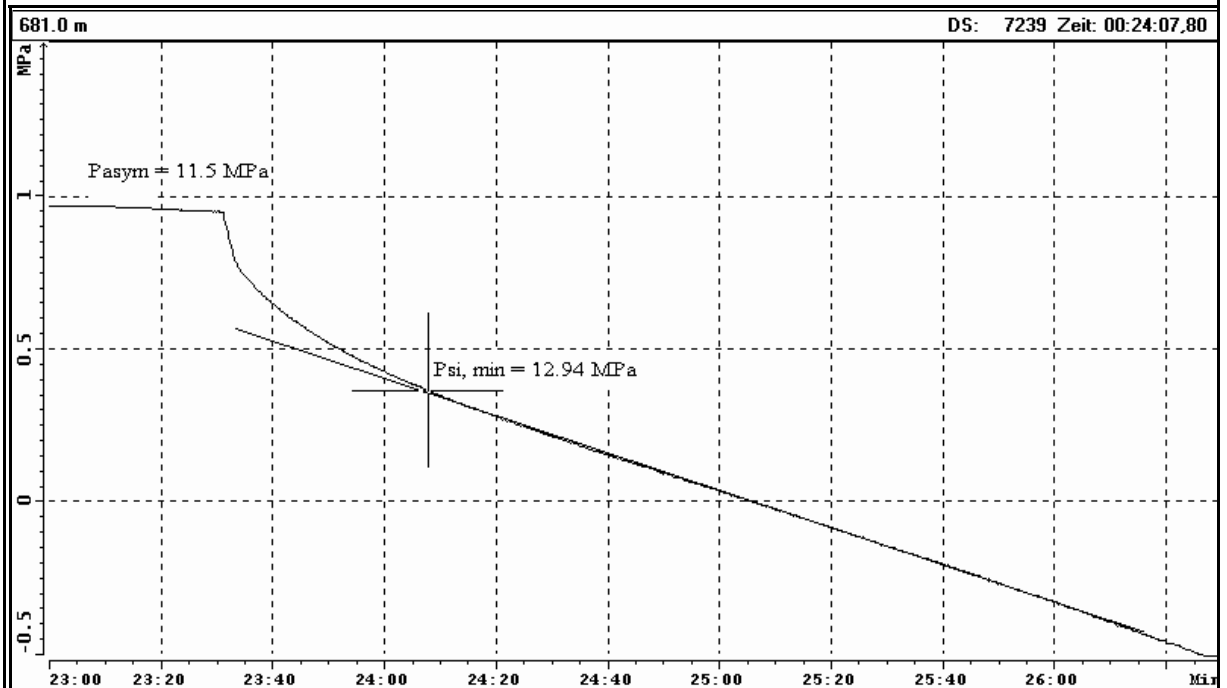
### Test at 681.0 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



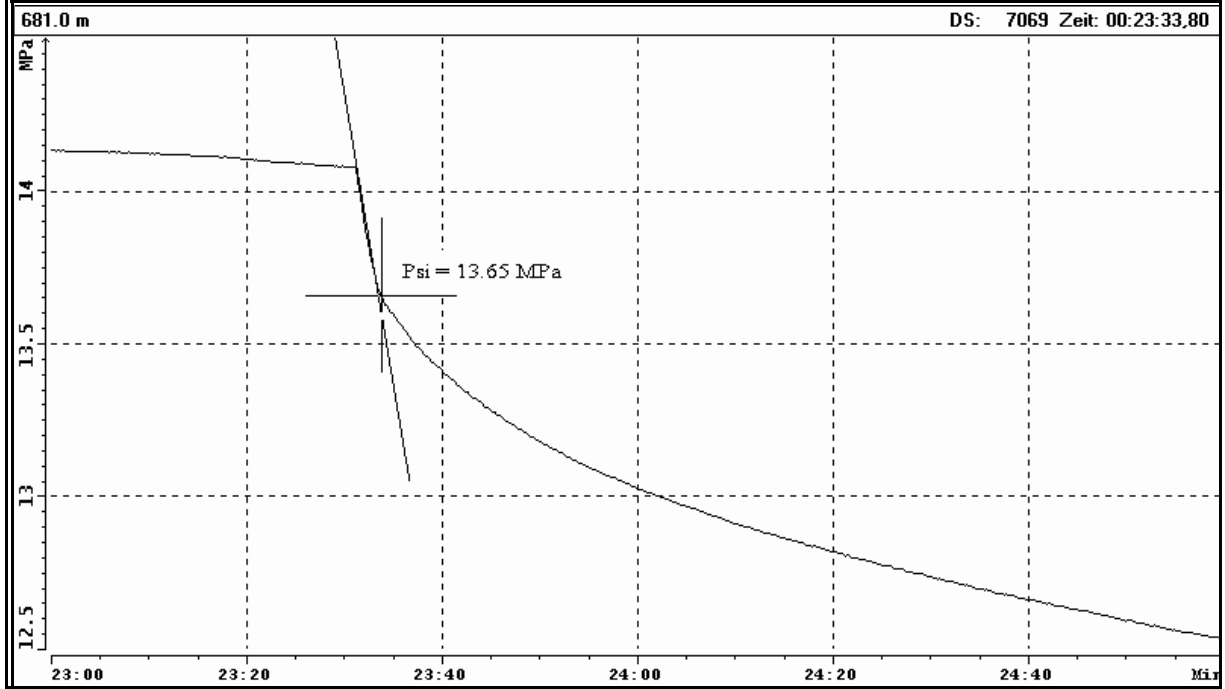
### Test at 681.0 m MD: Estimation of $P_{si, max}$ (1. Refrac - Cycle)



### Test at 681.0 m MD: Estimation of $P_{si, min}$ (1. Refrac - Cycle)

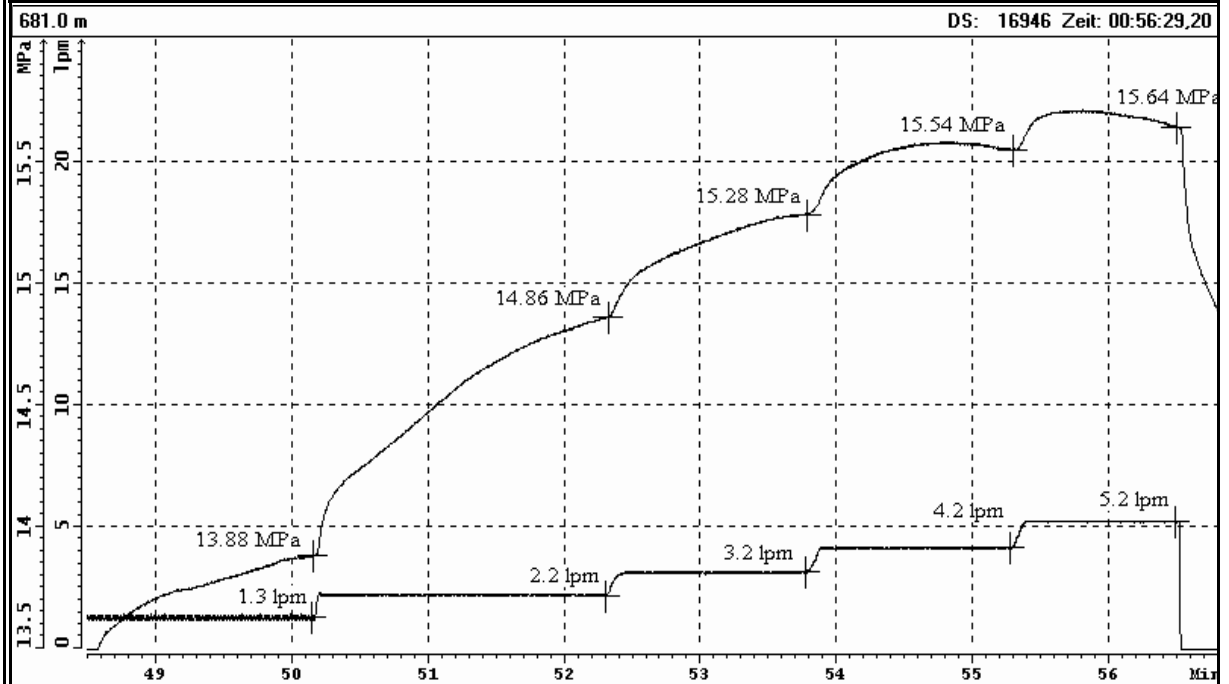


# Test at 681.0 m MD: Estimation of $P_{si}$ (1. Refrac - Cycle)

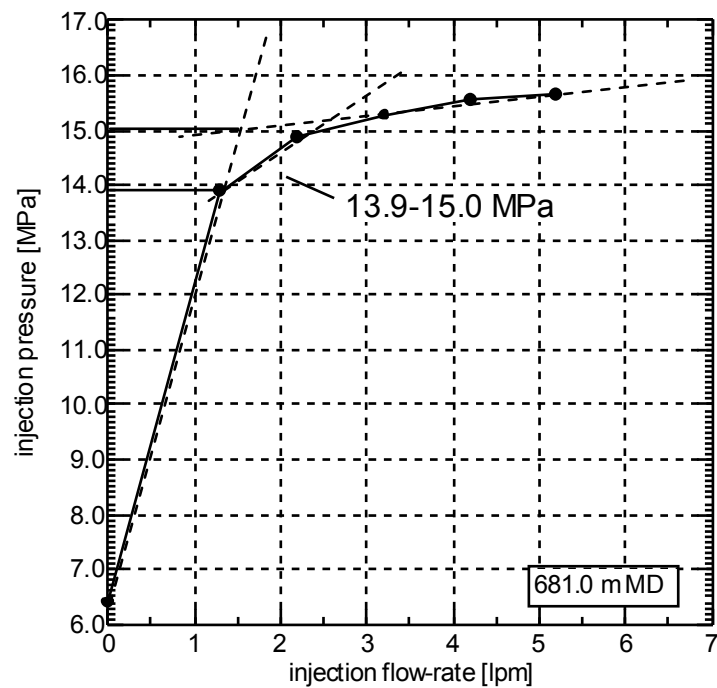




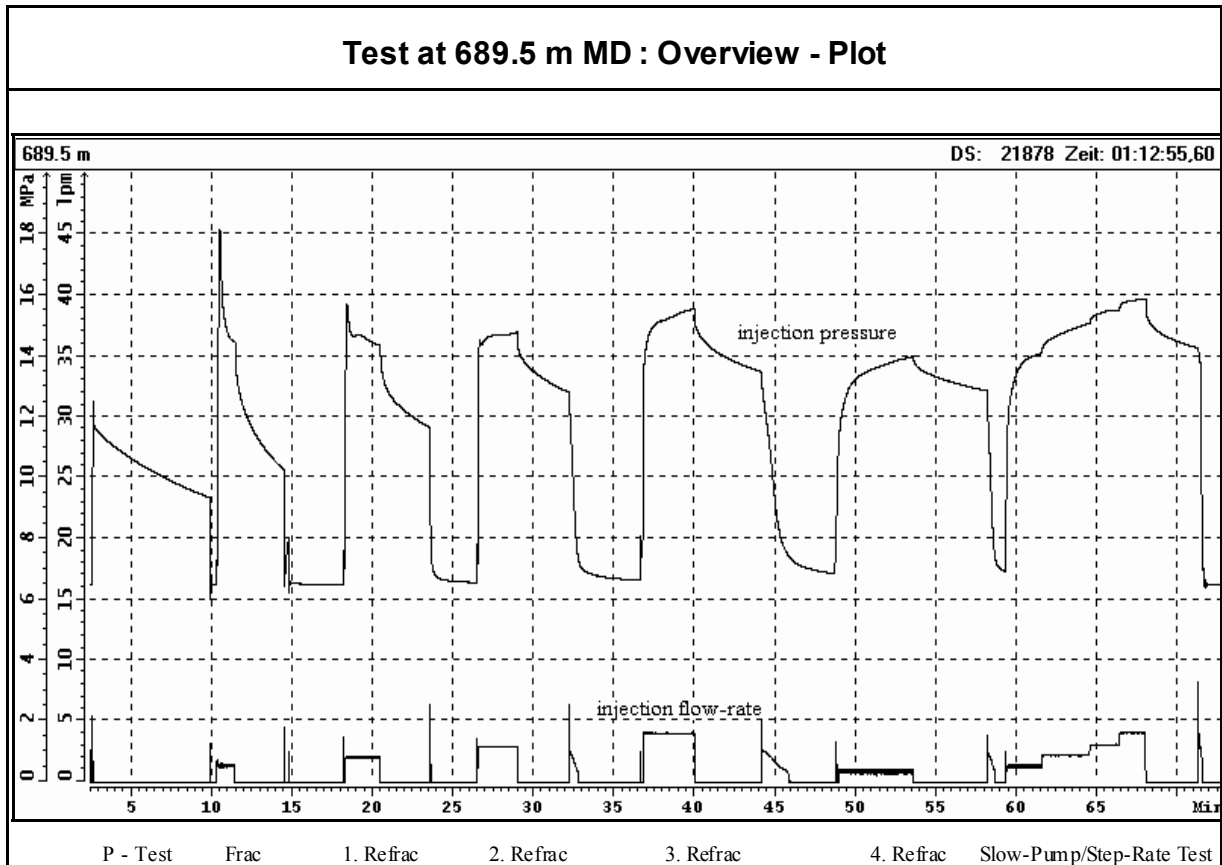
### Test at 681.0 m MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 681.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)



## TEST AT 689.5 m MD / 661.3 m TVD



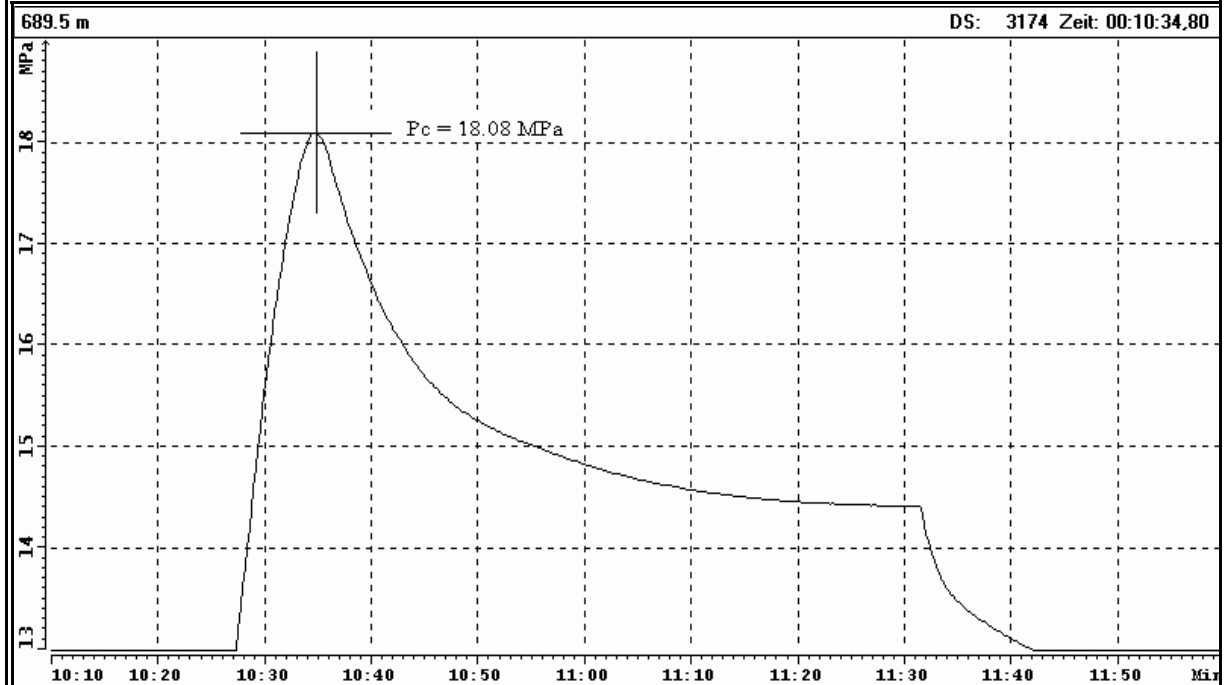
### TEST SUMMARY

P - Test :	pressure decrease: 2.3 MPa in 7 min. 9 sec.
Frac - Cycle :	injection - rate $Q_i = 1.4$ lpm, injected volume $V_i = 1.6$ l, back-flow volume $V_r = 0.1$ l clear fracture initiation (breakdown event)
1. Refrac - Cycle :	$Q_i = 2.1$ lpm, $V_i = 4.6$ l, $V_r = 0.2$ l
2. Refrac - Cycle :	$Q_i = 2.9$ lpm, $V_i = 7.2$ l, $V_r = 1.0$ l
3. Refrac - Cycle :	$Q_i = 4.0$ lpm, $V_i = 12.8$ l, $V_r = 3.0$ l
4. Refrac - Cycle :	$Q_i = 1.0$ lpm, $V_i = 4.5$ l, $V_r = 1.0$ l
Step-Rate Test :	$Q_i = 1.3$ - $4.1$ lpm, $V_i = 21.8$ l, $V_r = 0.7$ l

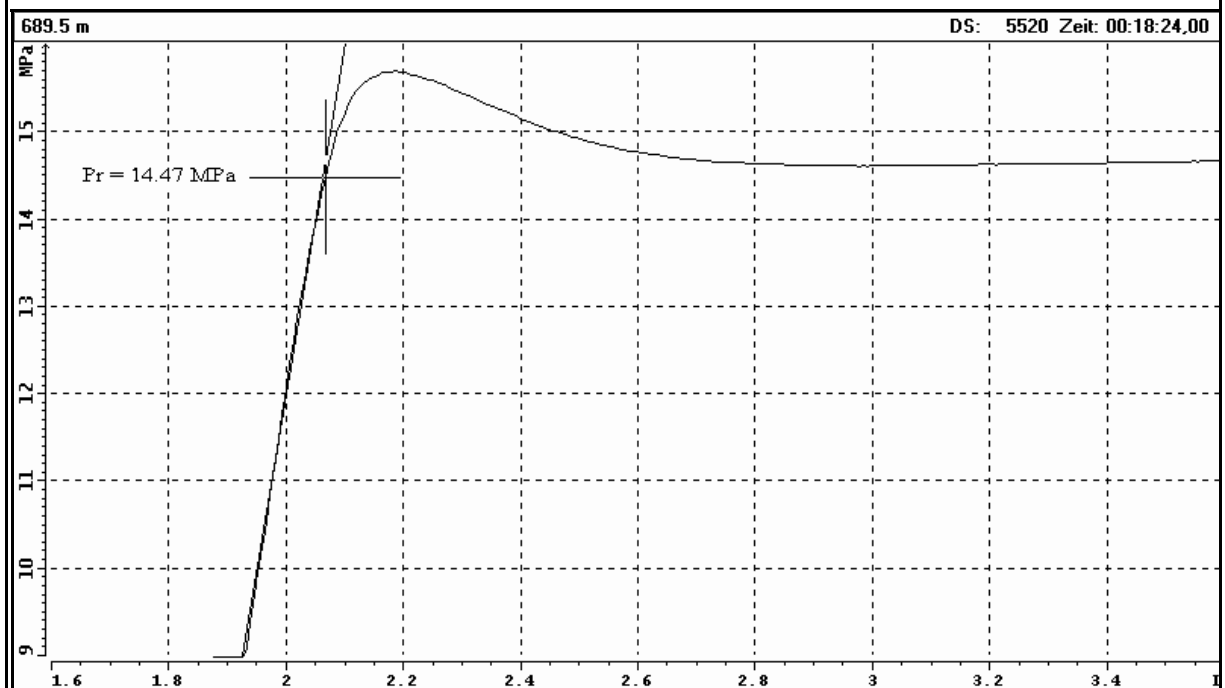
total injected volume = 52.5 l, recovered volume = 6.0 l (11.4 %)

*Remark: During the injection - cycles, an increase of the shut-in pressure was observed. The  $P_{si}$  - value was therefore determined from the 1. refrac - cycle.*

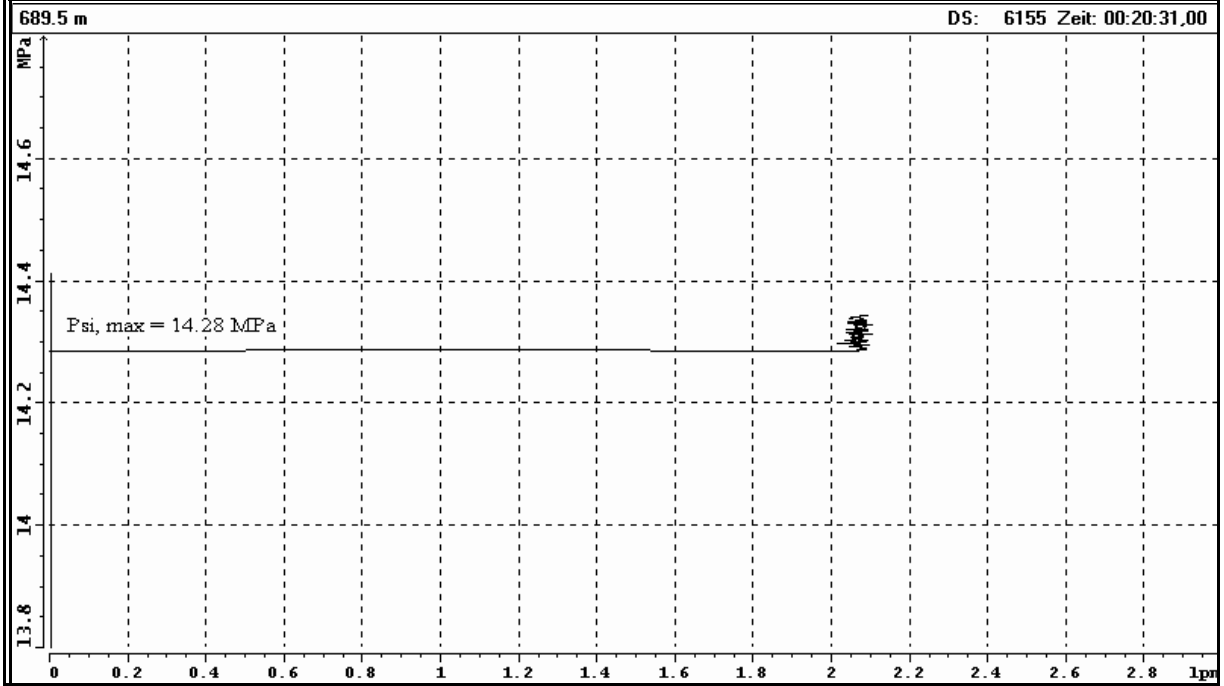
### Test at 689.5 m MD: Estimation of $P_c$ (Frac - Cycle)



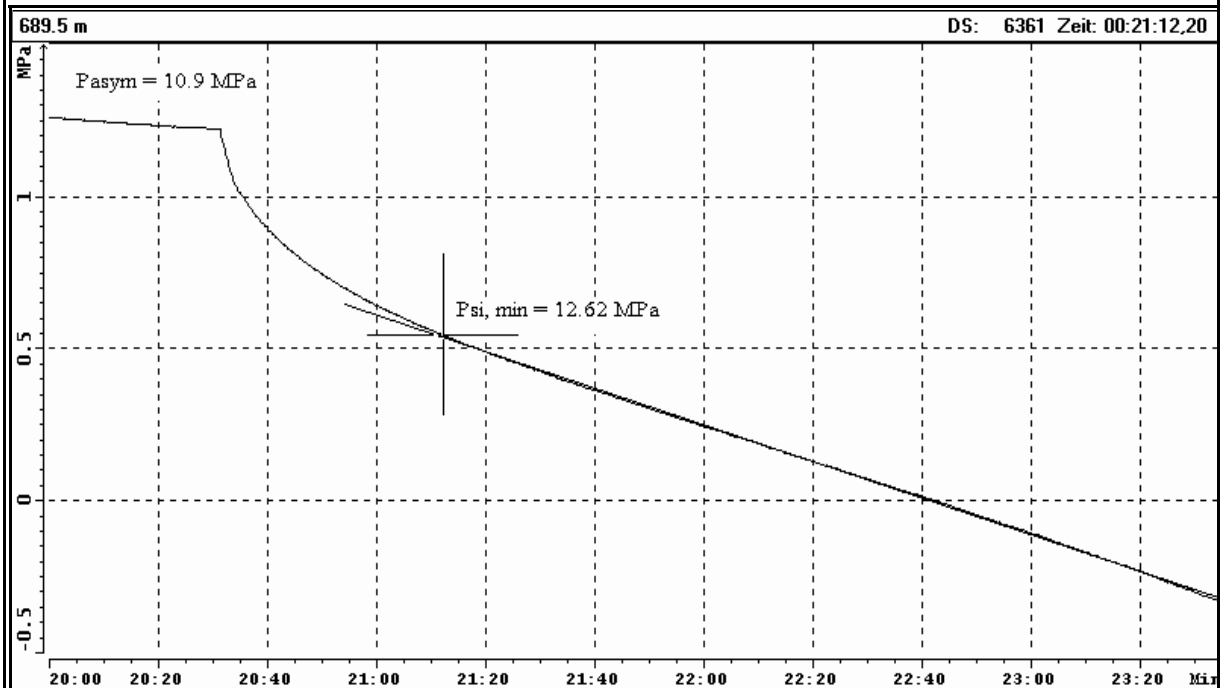
### Test at 689.5 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



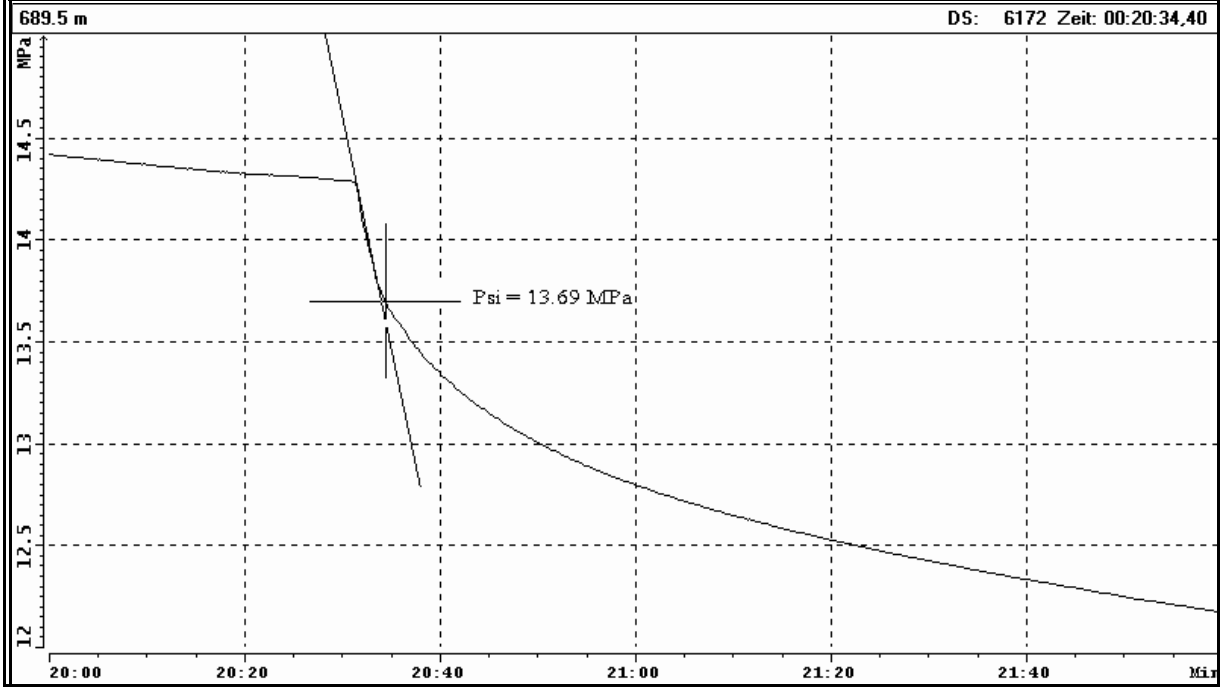
### Test at 689.5 m MD: Estimation of $P_{si, max}$ (1. Refrac - Cycle)



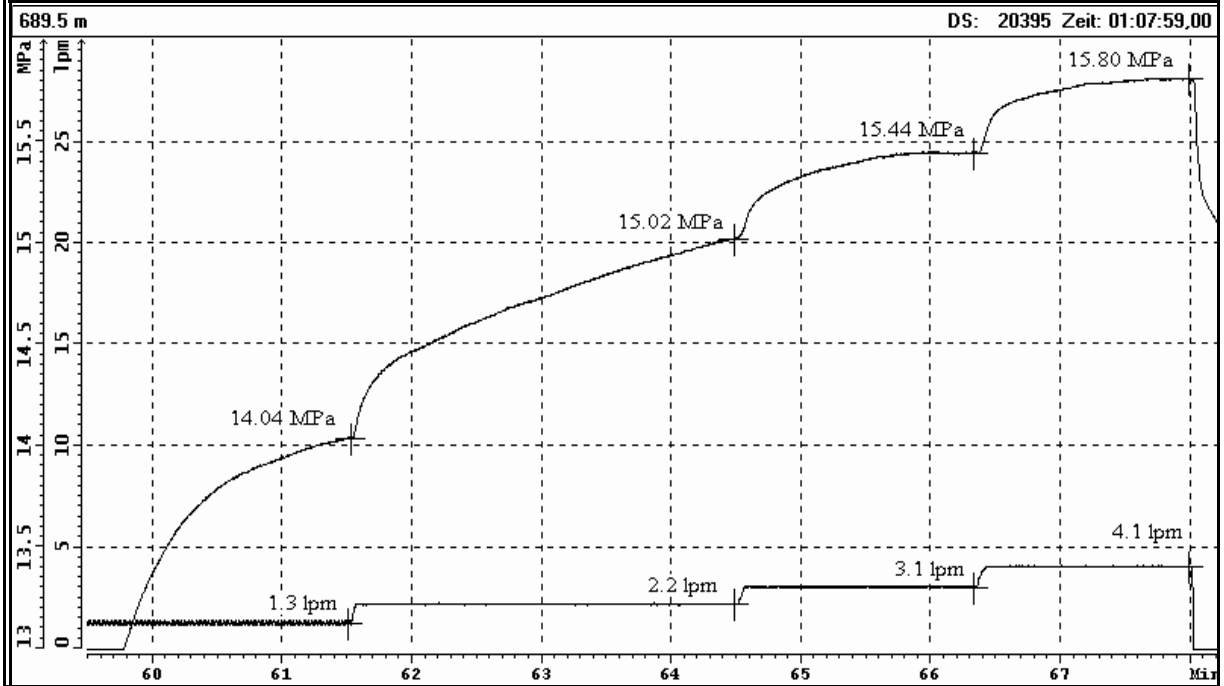
### Test at 689.5 m MD: Estimation of $P_{si, min}$ (1. Refrac - Cycle)



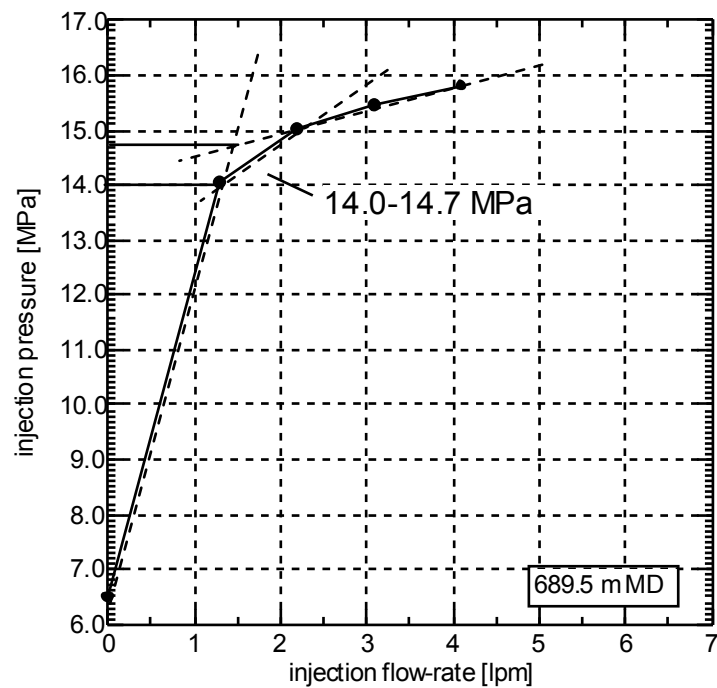
# Test at 689.5 m MD: Estimation of $P_{si}$ (1. Refrac - Cycle)



### Test at 689.5 m MD: Analysis of Slow - Pump / Step - Rate - Test

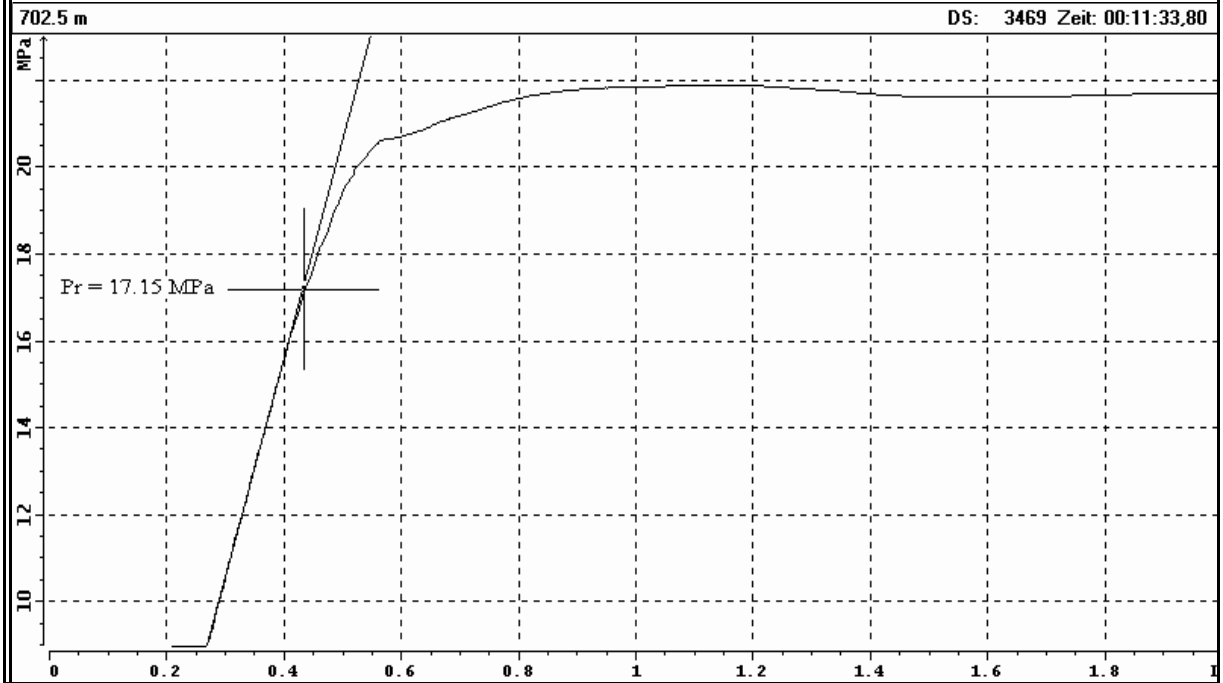


### Test at 689.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)

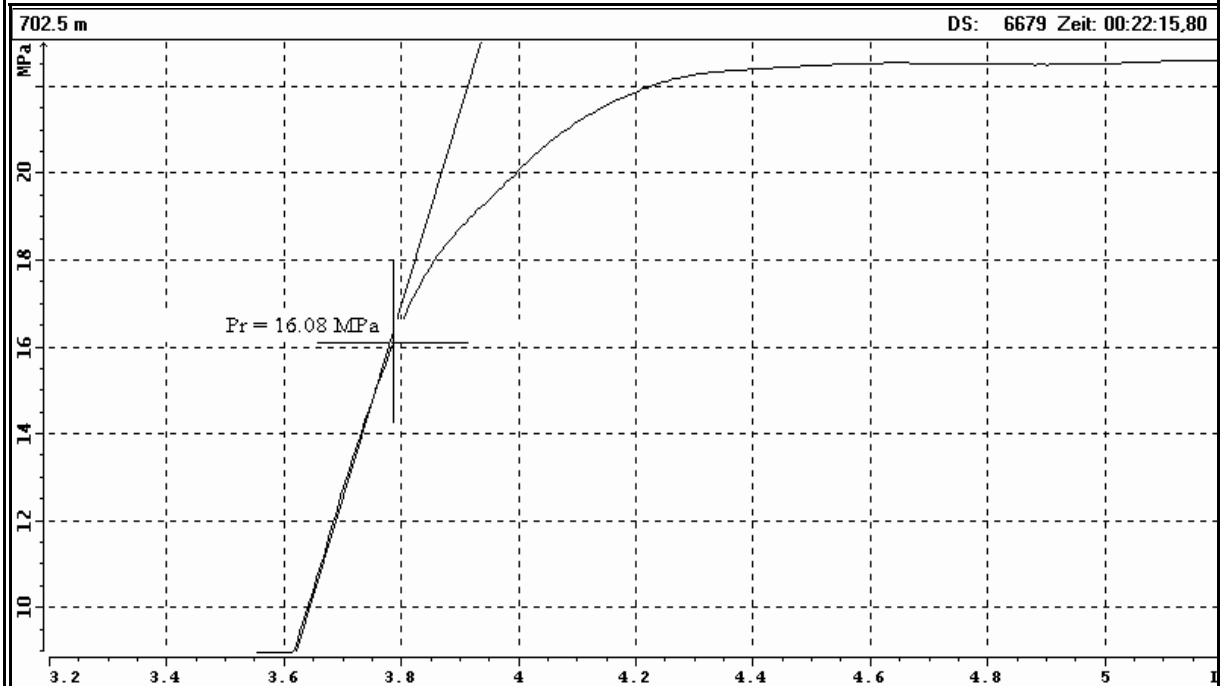




### Test at 702.5 m MD: Estimation of $P_r$ (1. Refrac) - Cycle

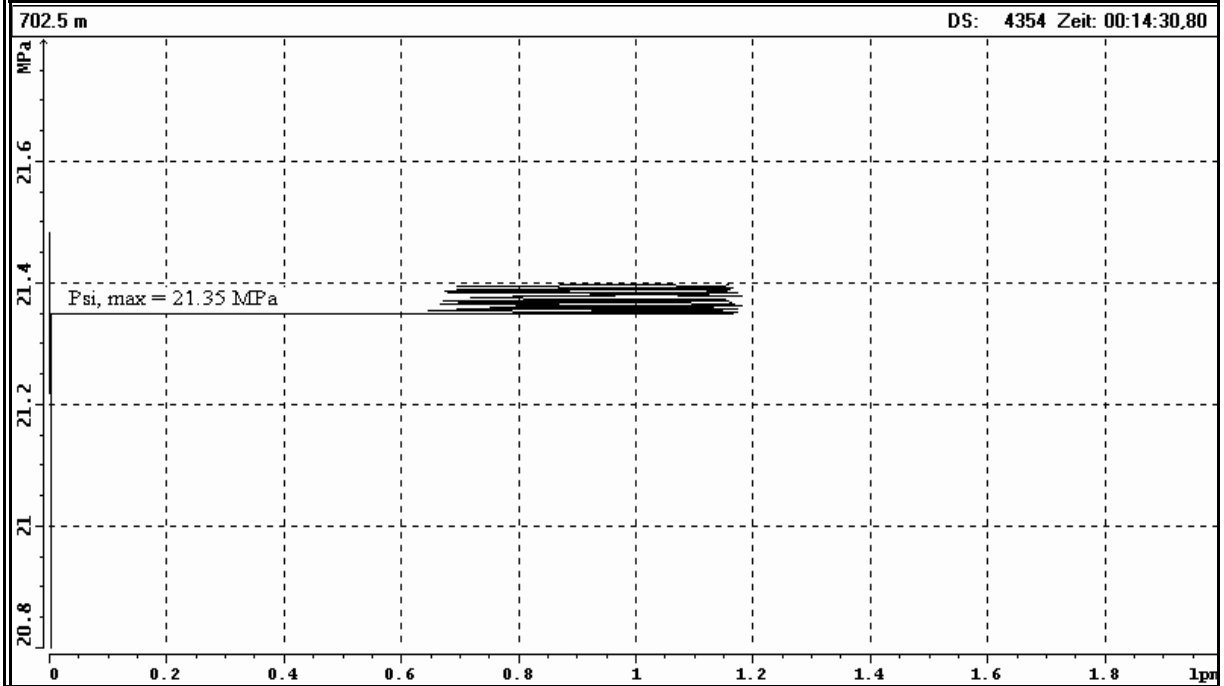


### Test at 702.5 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)

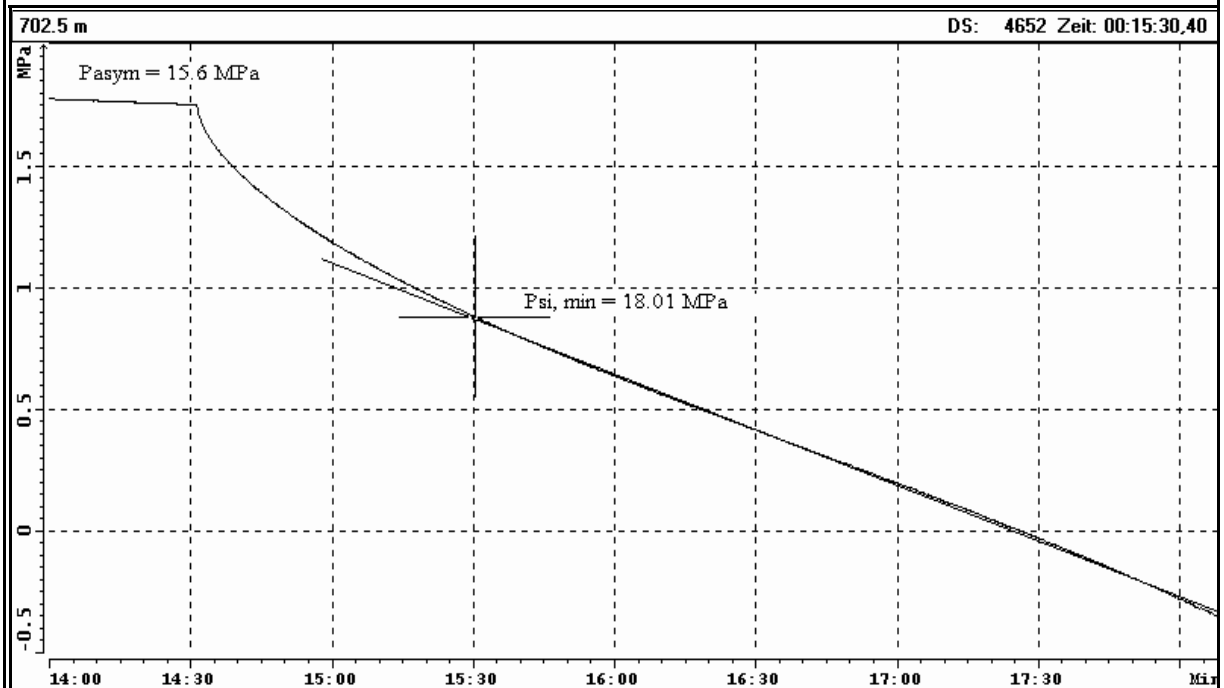




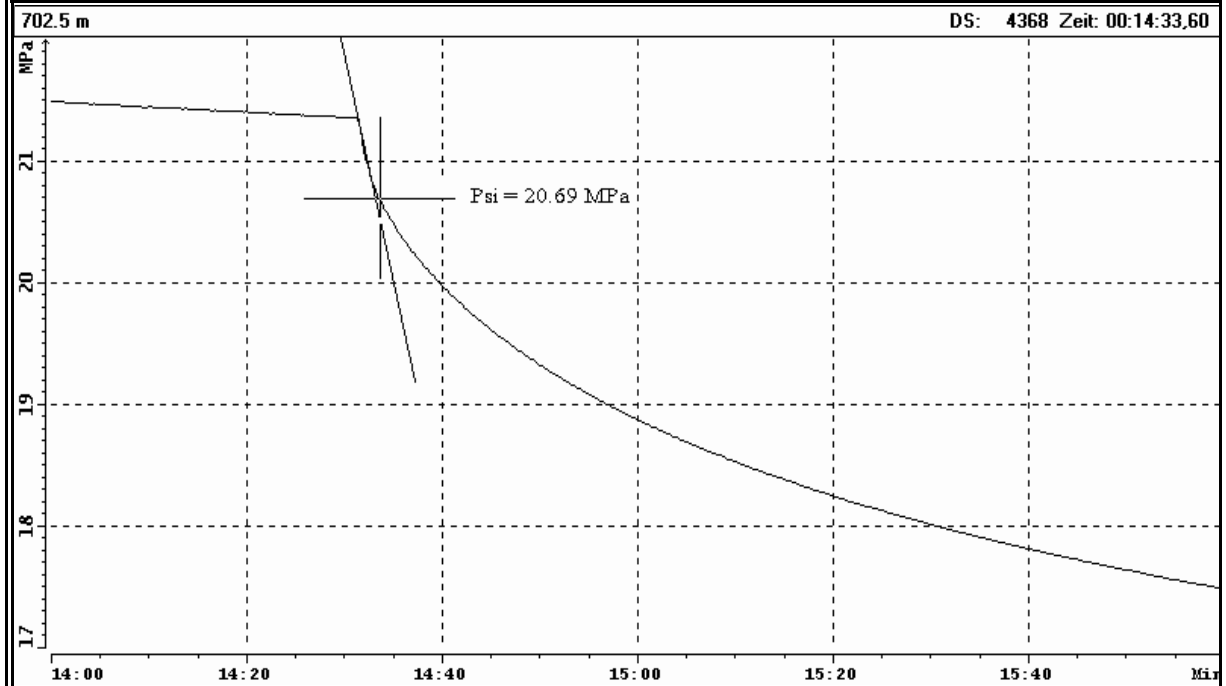
### Test at 702.5 m MD: Estimation of $P_{si, max}$ (1. Refrac - Cycle)



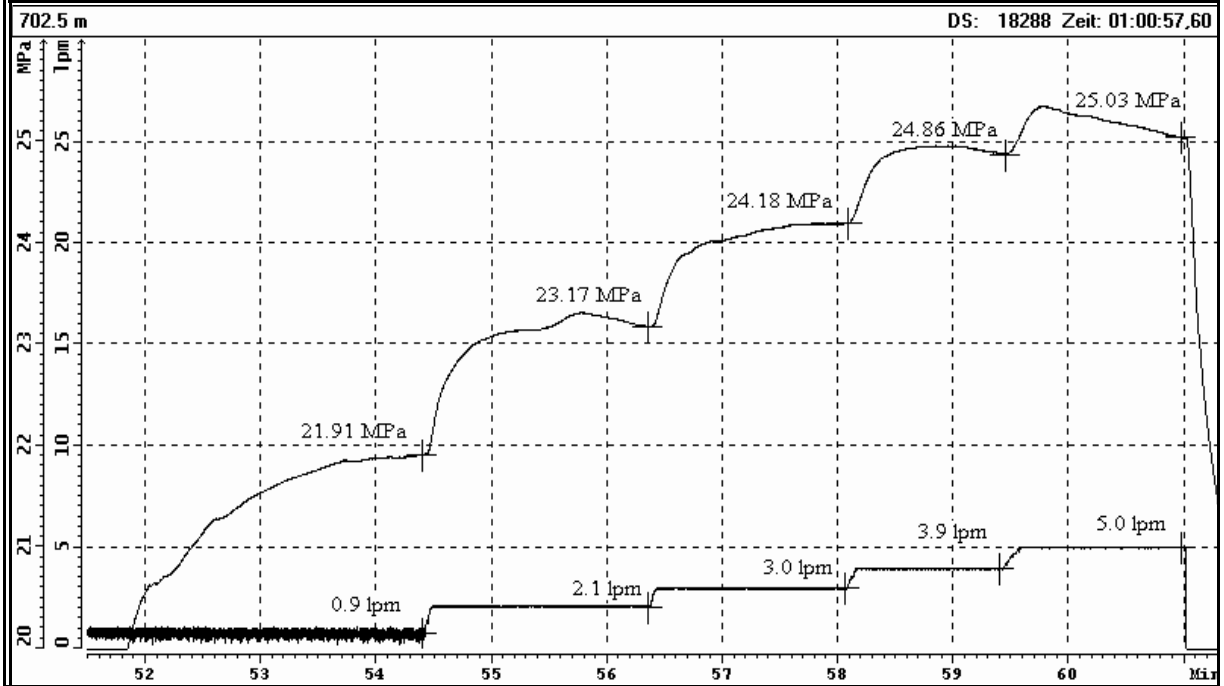
### Test at 702.5 m MD: Estimation of $P_{si, min}$ (1. Refrac - Cycle)



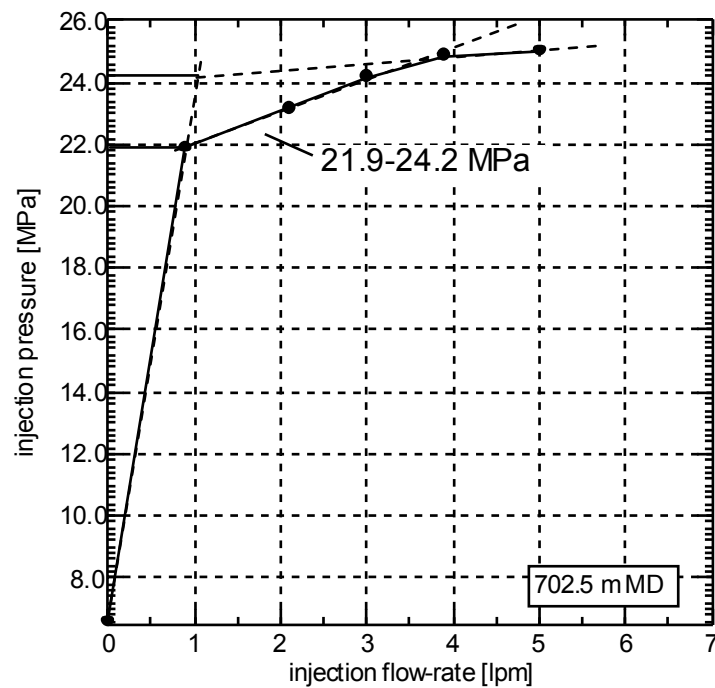
### Test at 702.5 m MD: Estimation of $P_{si}$ (1. Refrac - Cycle)



### Test at 702.5 m MD: Analysis of Slow - Pump / Step - Rate - Test

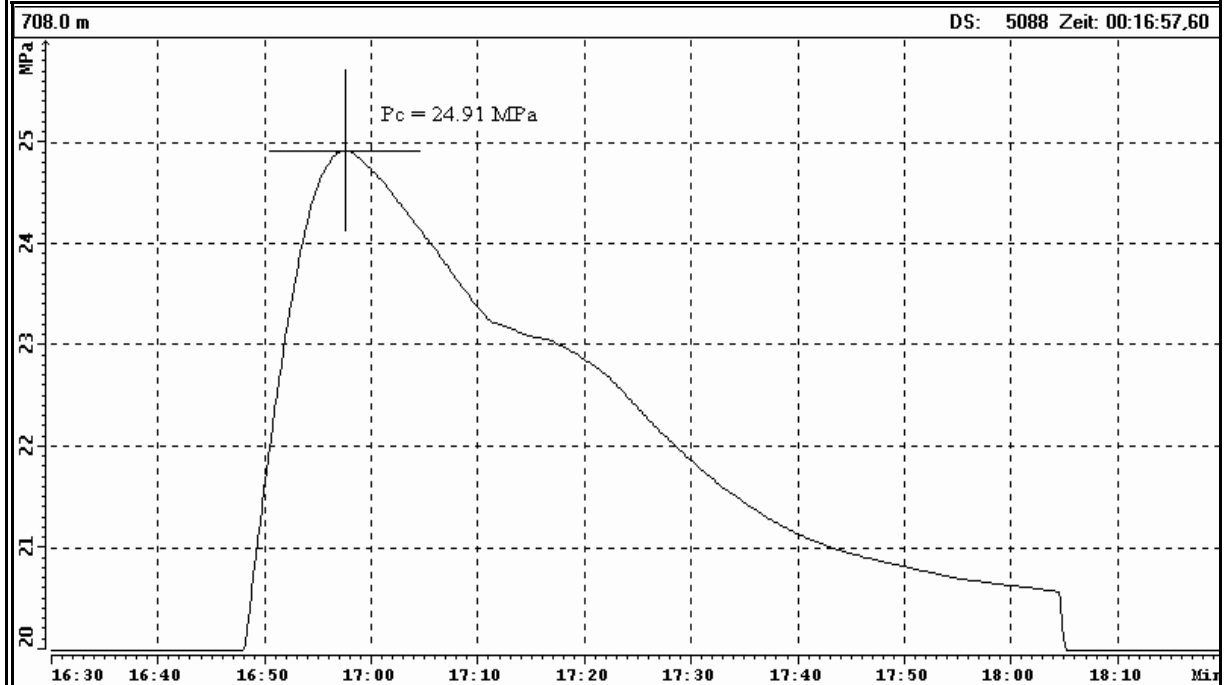


### Test at 702.5 m MD: Examination of $P_{si}$ (Step - Rate - Test)

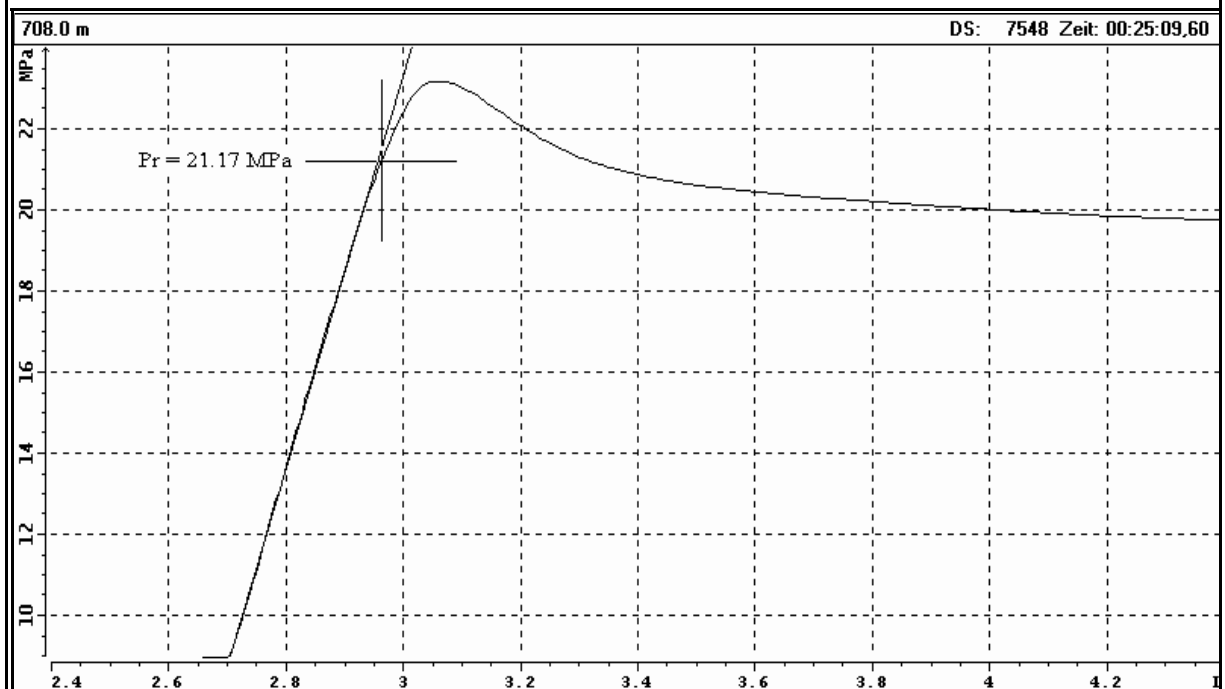




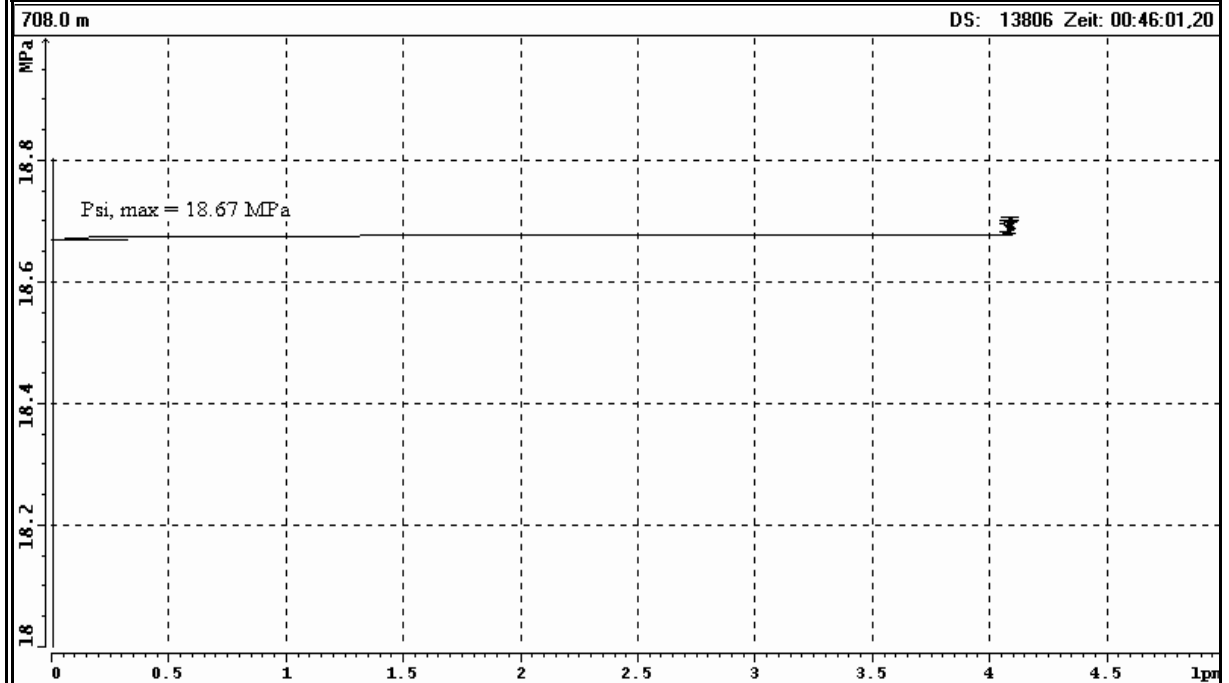
### Test at 708.0 m MD: Estimation of $P_c$ (Frac - Cycle)



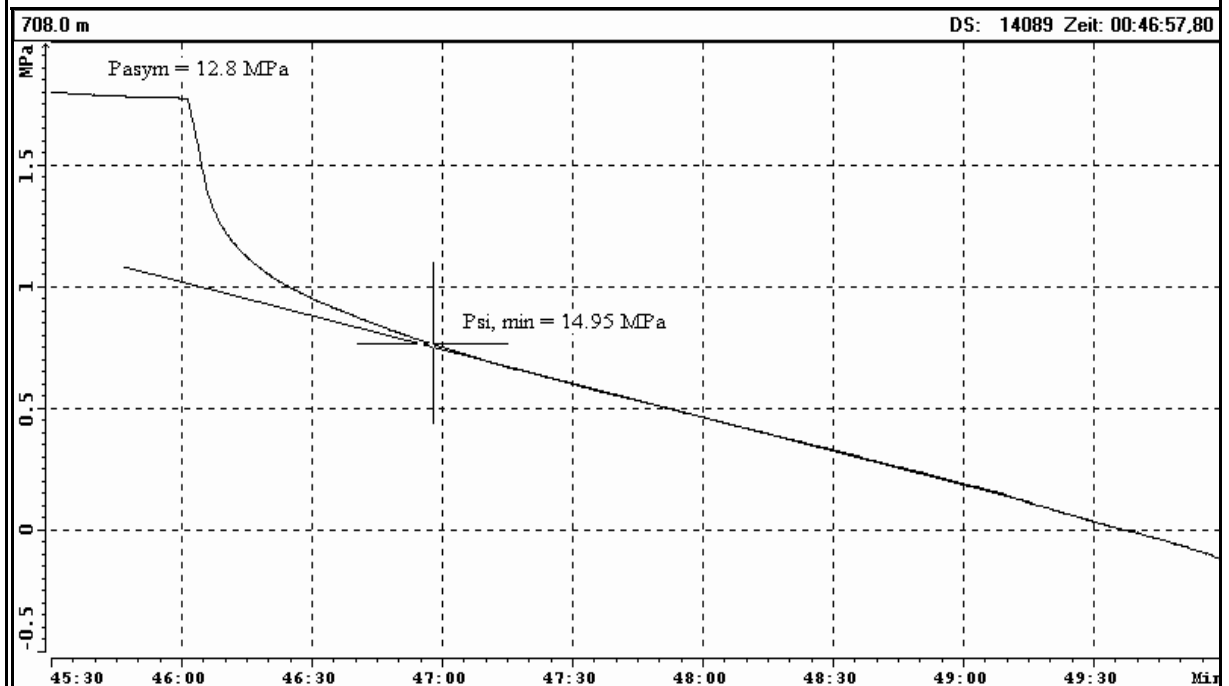
### Test at 708.0 m MD: Estimation of $P_r$ (1. Refrac - Cycle)



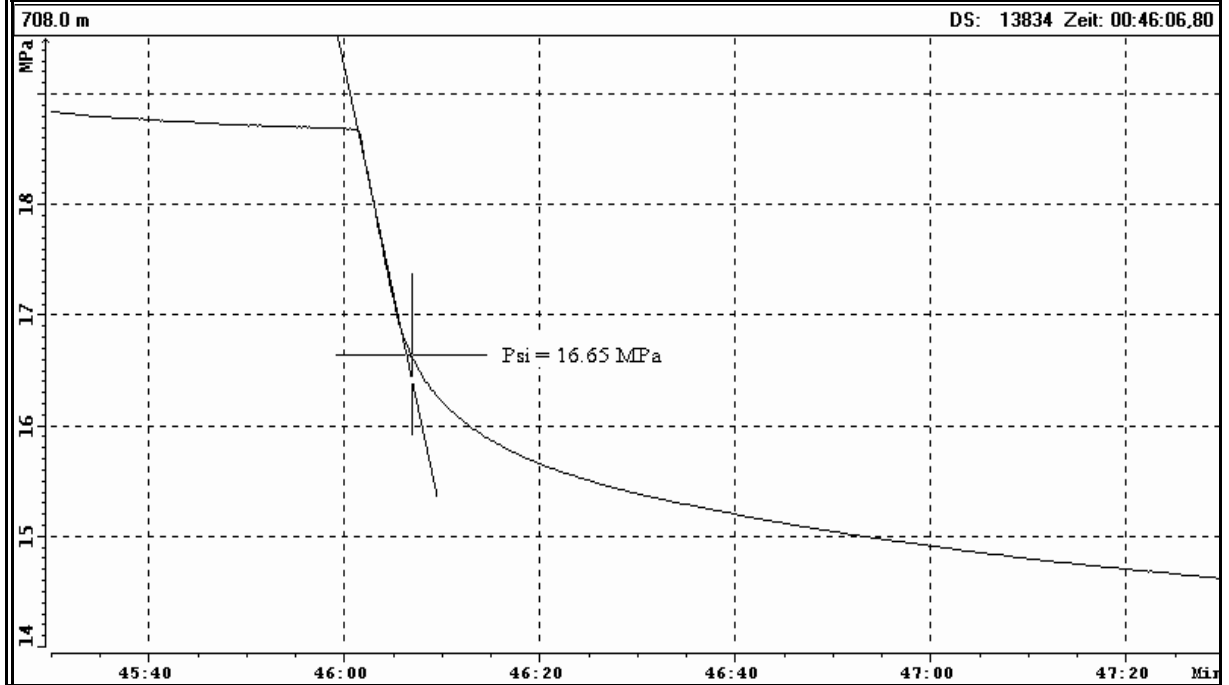
### Test at 708.0 m MD: Estimation of $P_{si, max}$ (3. Refrac - Cycle)



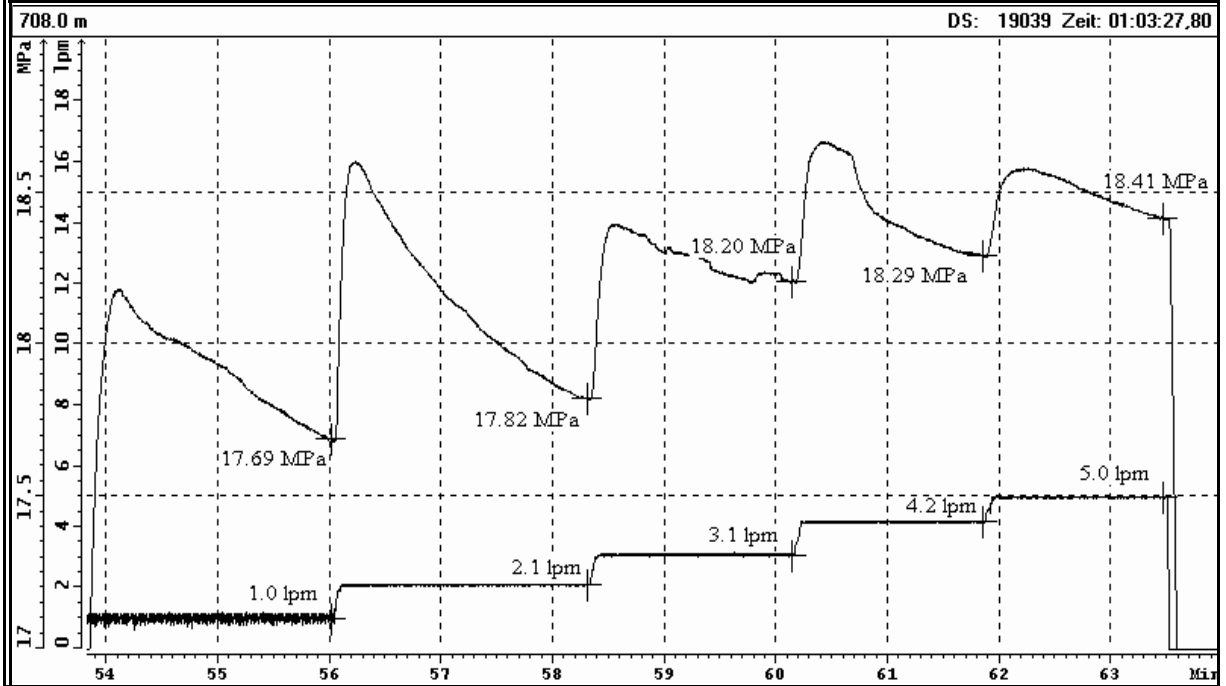
### Test at 708.0 m MD: Estimation of $P_{si, min}$ (3. Refrac - Cycle)



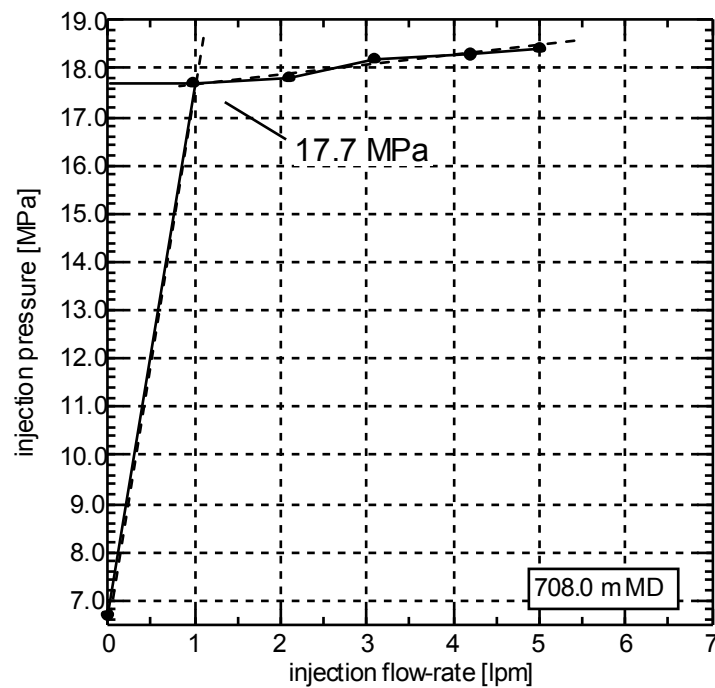
### Test at 708.0 m MD: Estimation of $P_{si}$ (3. Refrac - Cycle)



### Test at 708.0 m MD: Analysis of Slow - Pump / Step - Rate - Test

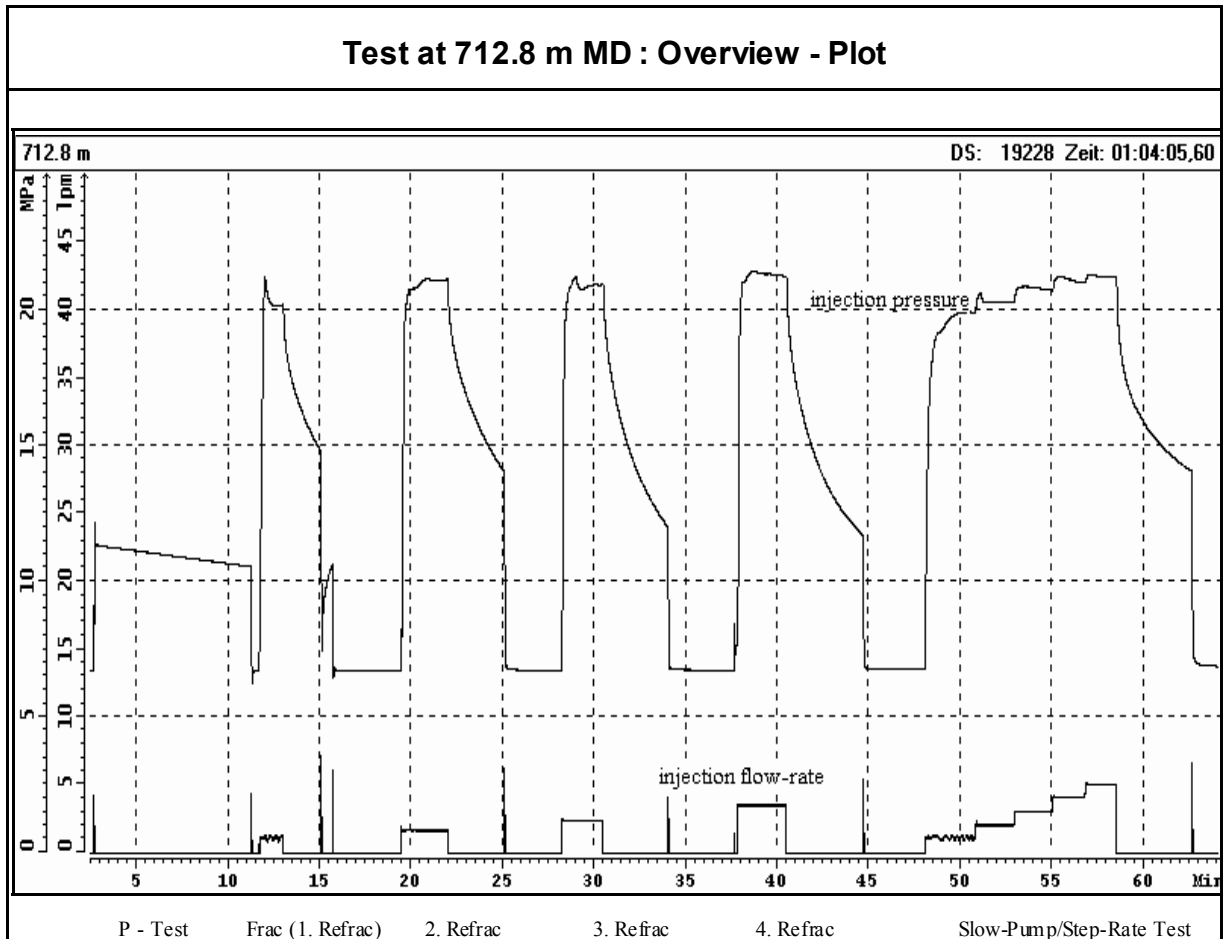


### Test at 708.0 m MD: Examination of $P_{si}$ (Step - Rate - Test)





## TEST AT 712.8 m MD / 684.4 m TVD

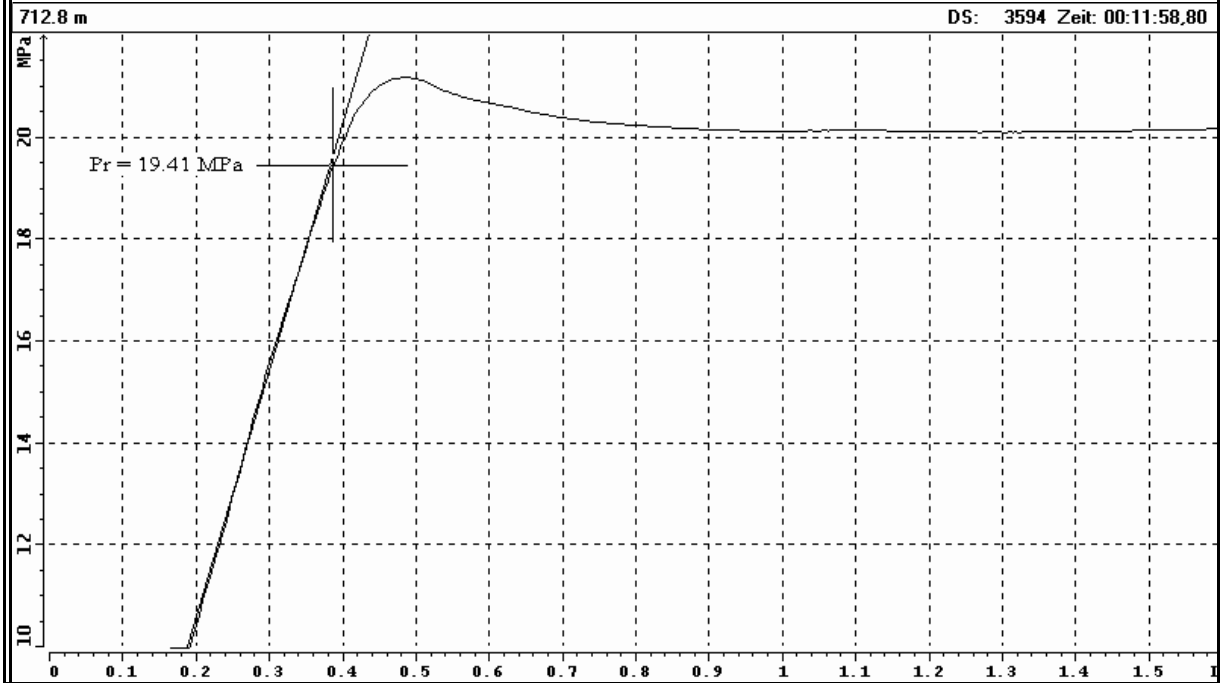


### TEST SUMMARY

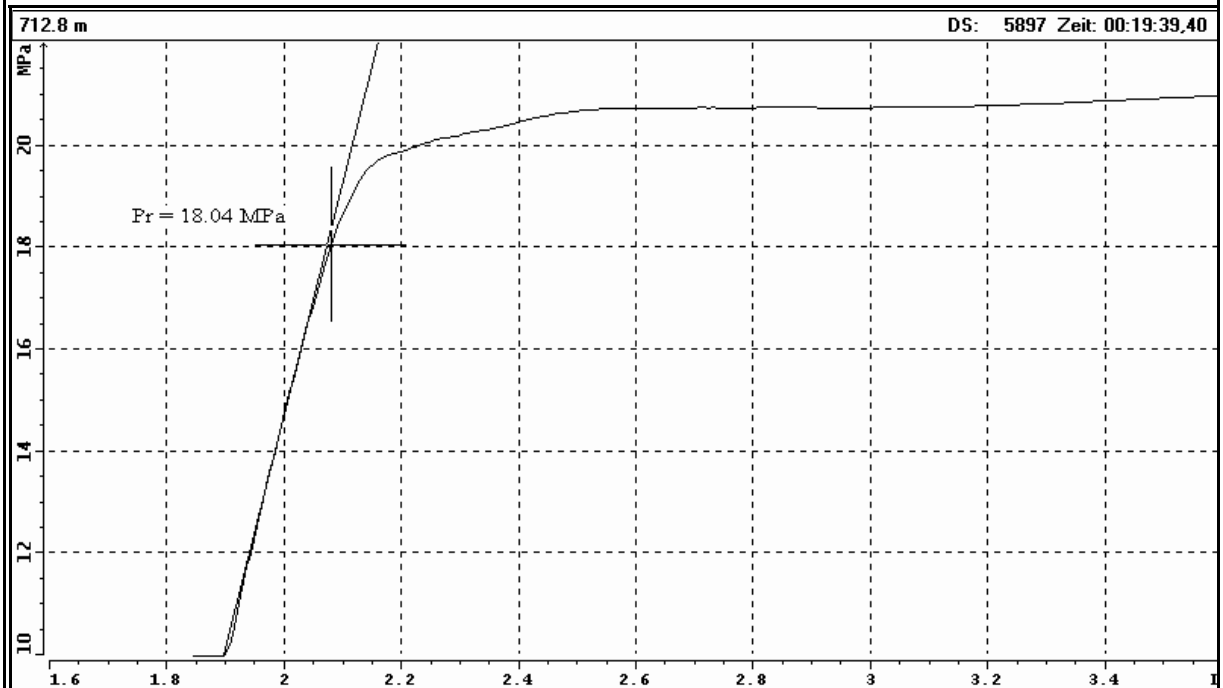
- P - Test :                      pressure decrease: 0.8 MPa in 8 min. 24 sec.
- Frac (1. Refrac) - Cycle :    injection-rate  $Q_i = 1.0$  lpm, injected volume  $V_i = 1.5$  l, back-flow volume  $V_r = 0.2$  l  
    stimulation of an existing fracture (no breakdown event)
2. Refrac - Cycle :             $Q_i = 1.7$  lpm,  $V_i = 4.2$  l,  $V_r = 0.2$  l
3. Refrac - Cycle :             $Q_i = 2.4$  lpm,  $V_i = 5.4$  l,  $V_r = 0.1$  l
4. Refrac - Cycle :             $Q_i = 3.5$  lpm,  $V_i = 9.2$  l,  $V_r = 0.1$  l
- Step-Rate Test :               $Q_i = 1.1$ - $5.0$  lpm,  $V_i = 29.9$  l,  $V_r = 0.2$  l

total injected volume = 50.2 l, recovered volume = 0.8 l (1.6 %)

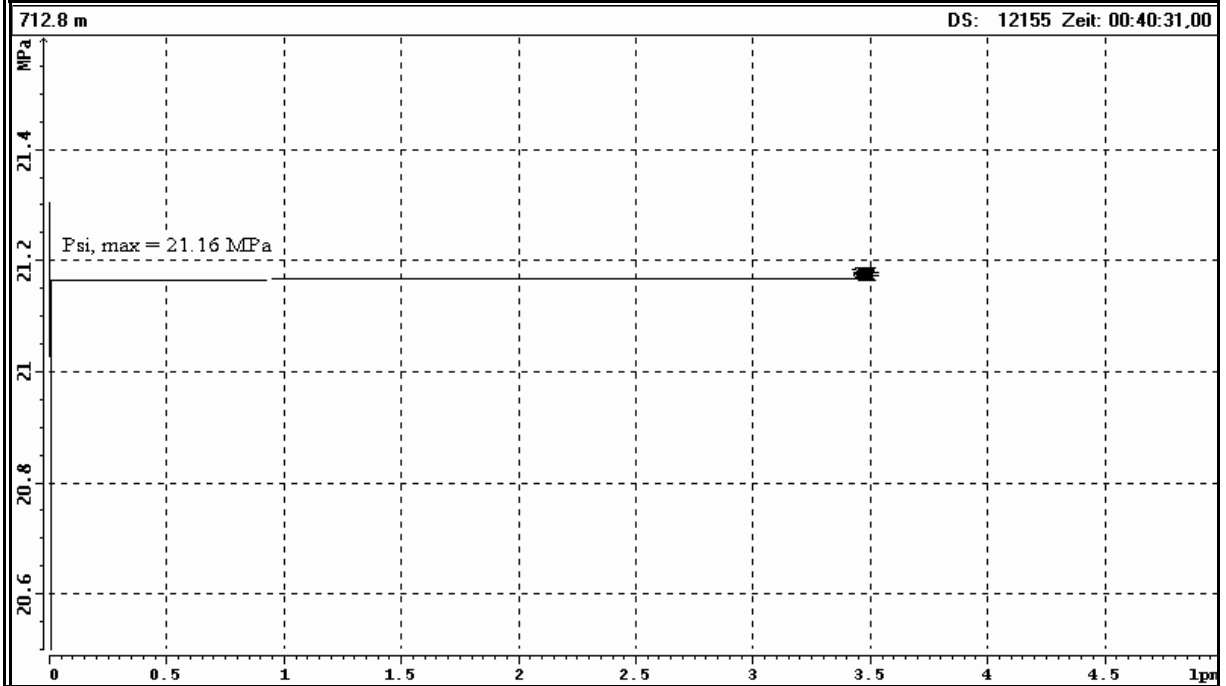
### Test at 712.8 m MD: Estimation of $P_r$ (1. Refrac) - Cycle



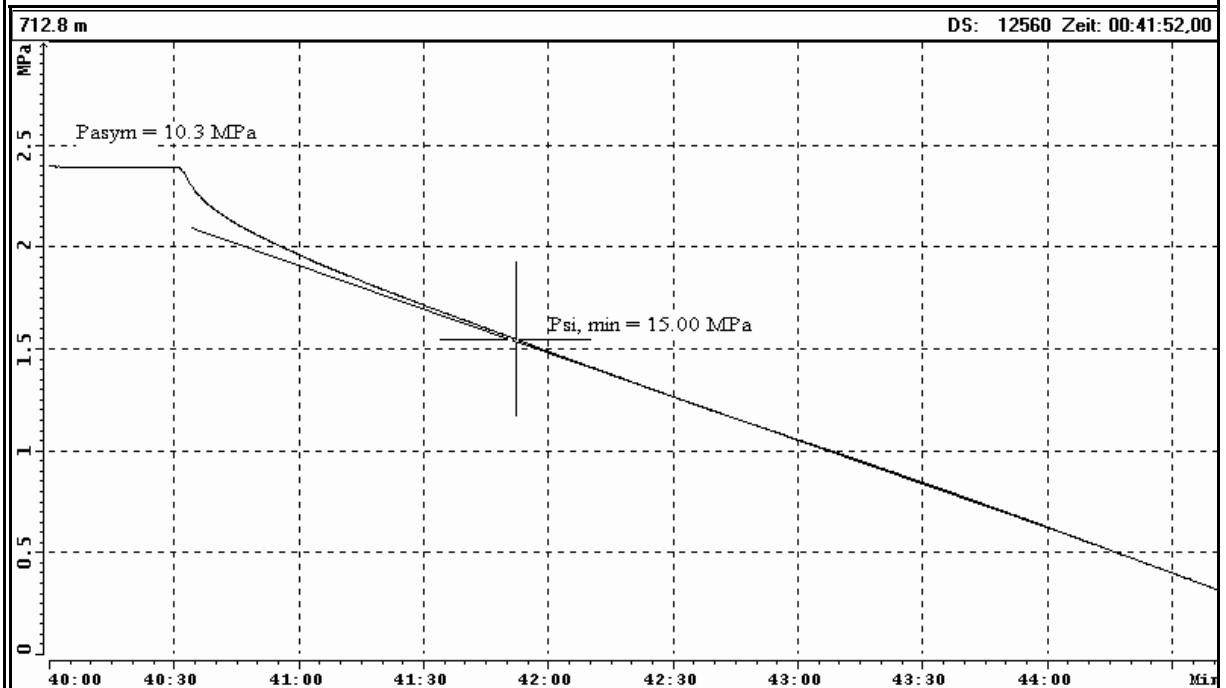
### Test at 712.8 m MD: Estimation of $P_r$ (2. Refrac - Cycle, for comparison)



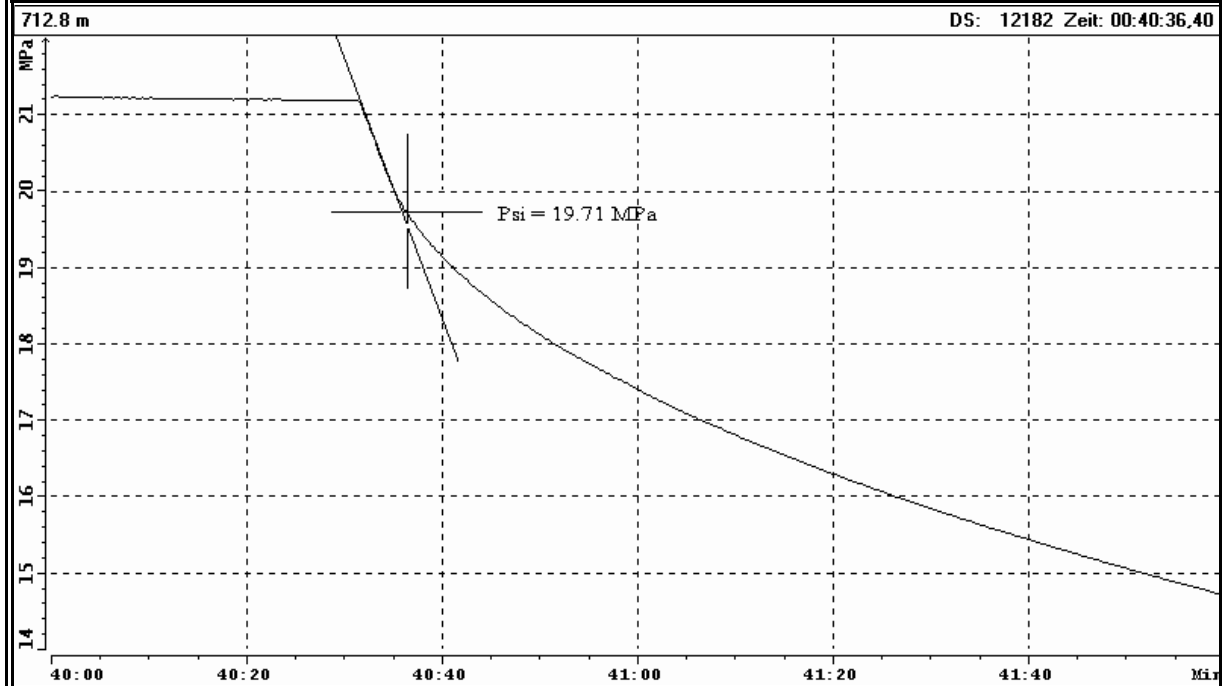
### Test at 712.8 m MD: Estimation of $P_{si, max}$ (4. Refrac - Cycle)



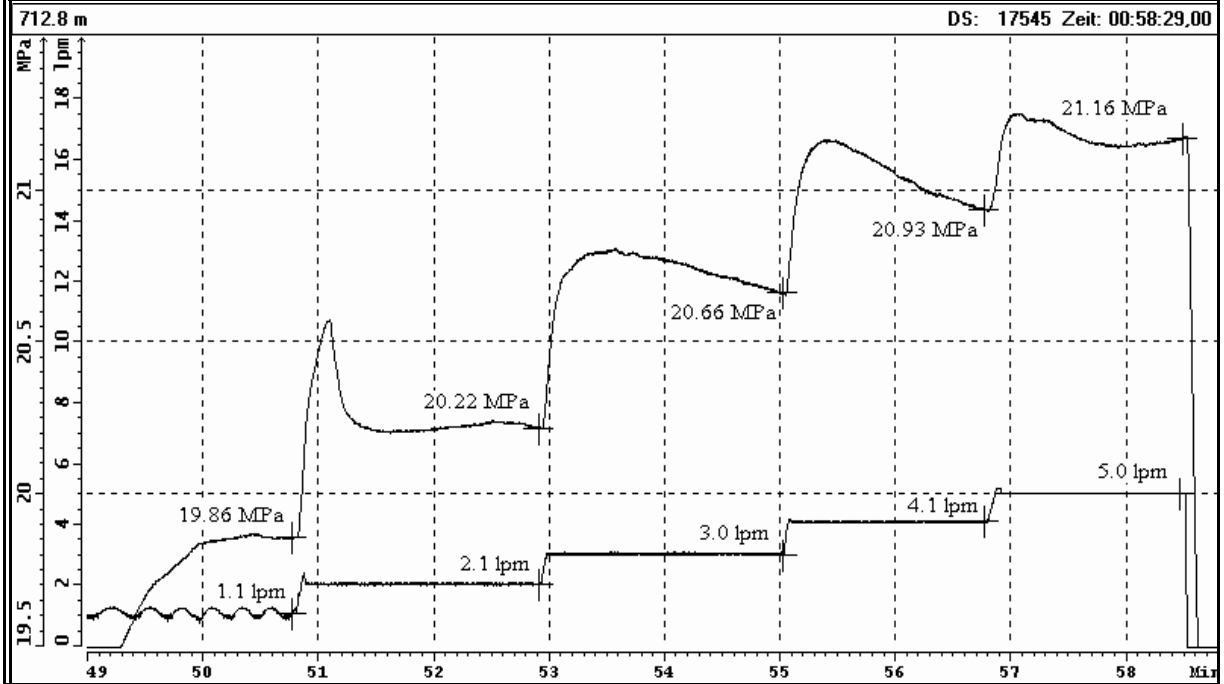
### Test at 712.8 m MD: Estimation of $P_{si, min}$ (4. Refrac - Cycle)



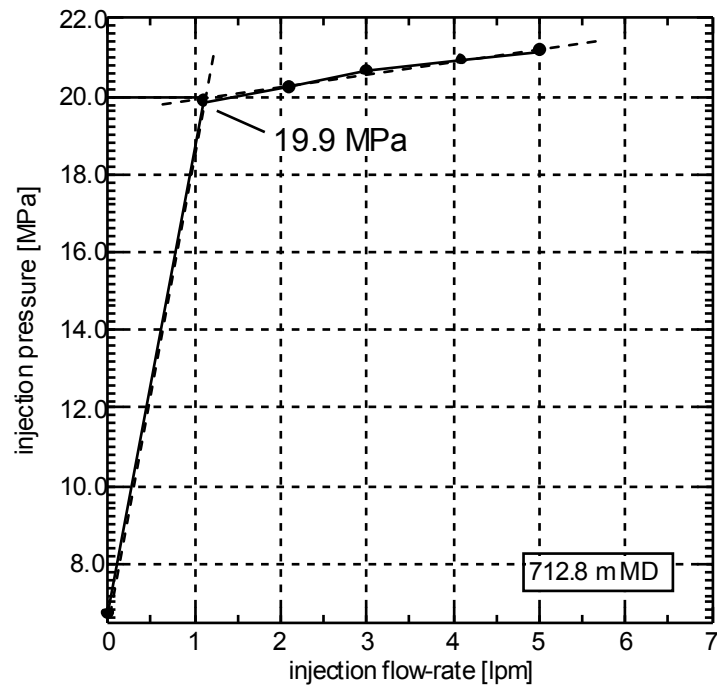
### Test at 712.8 m MD: Estimation of $P_{si}$ (4. Refrac - Cycle)



### Test at 712.8 MD: Analysis of Slow - Pump / Step - Rate - Test



### Test at 712.8 MD: Examination of $P_{si}$ (Step - Rate - Test)



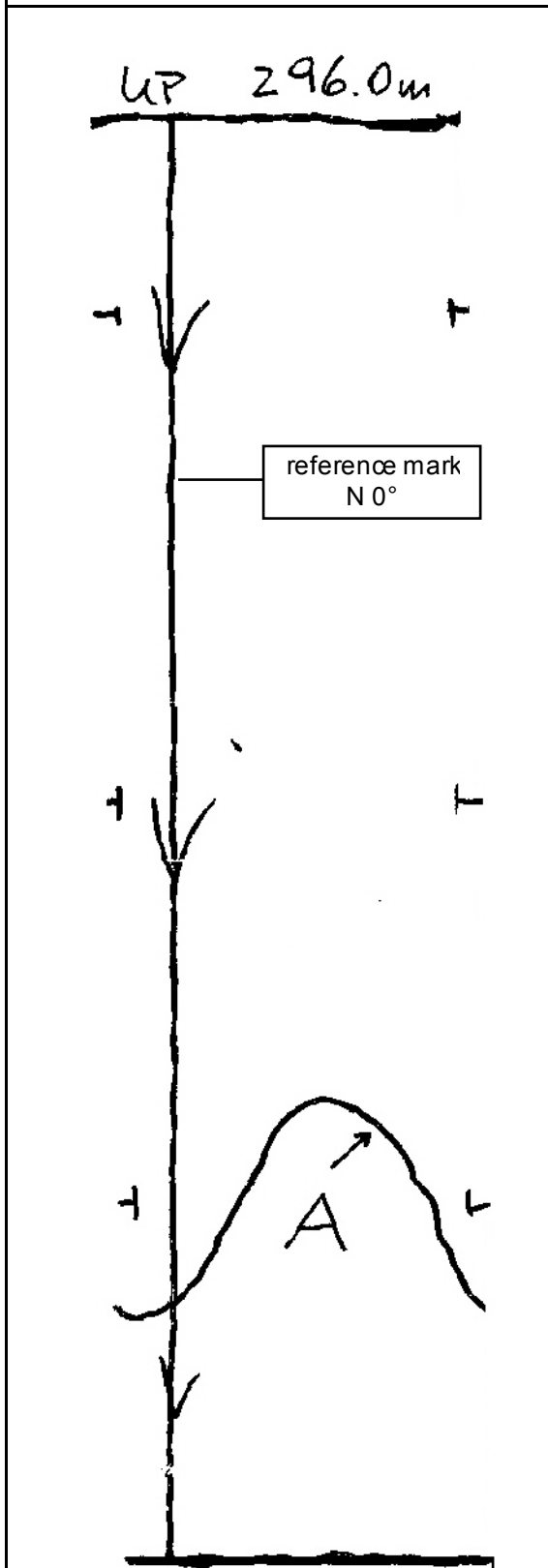
## **APPENDIX B**

### **Fracture traces on the impression packer sleeve**



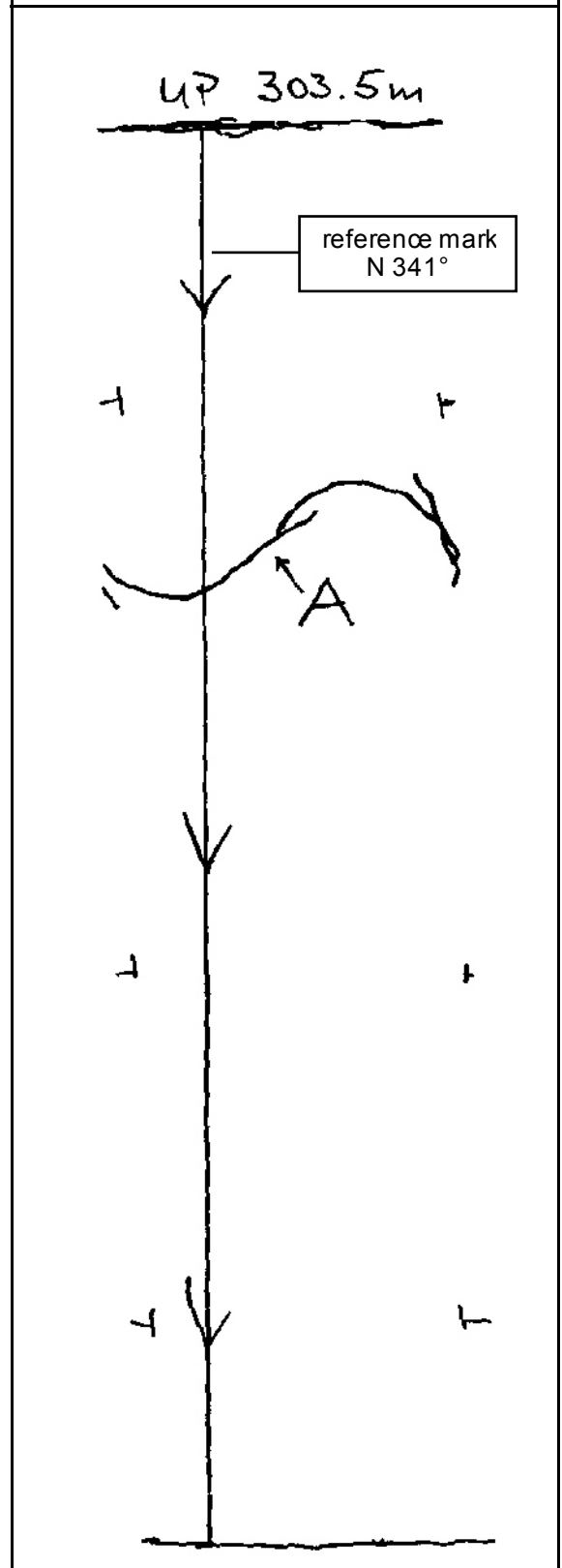
**Test at 296.0 m MD / 280.9 m TVD**

borehole dip / direction: 20° / N 206°



**Test at 303.5 m MD / 288.0 m TVD**

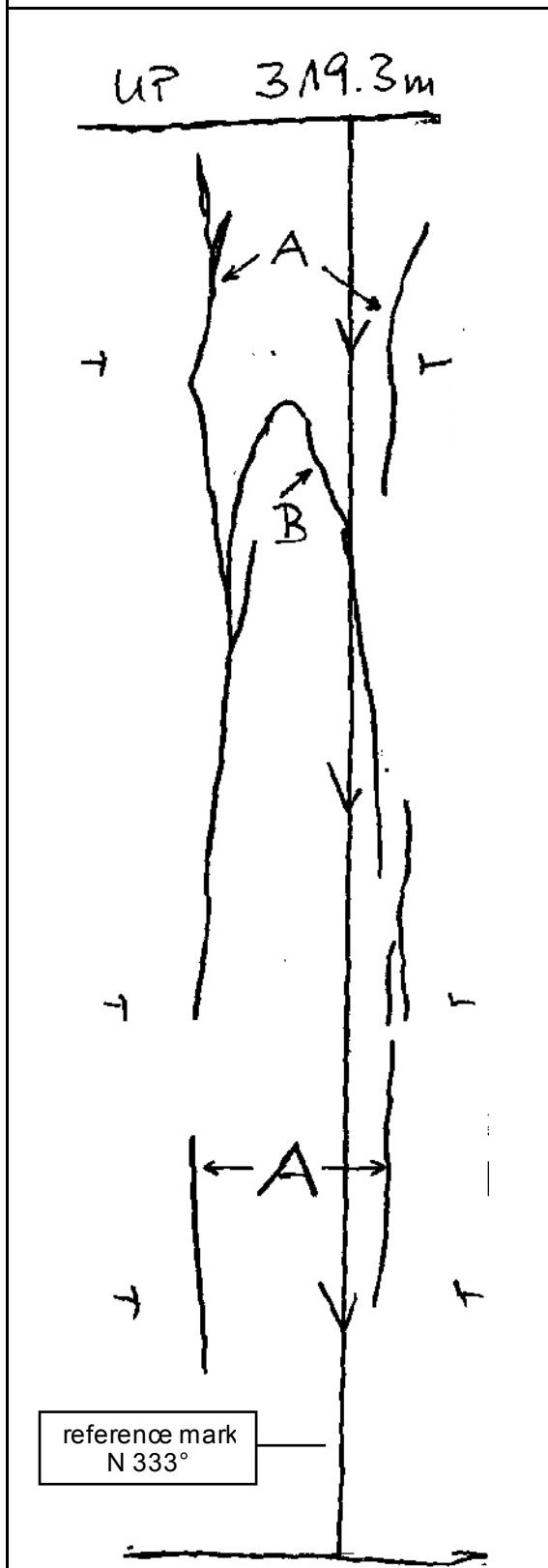
borehole dip / direction: 21° / N 206°





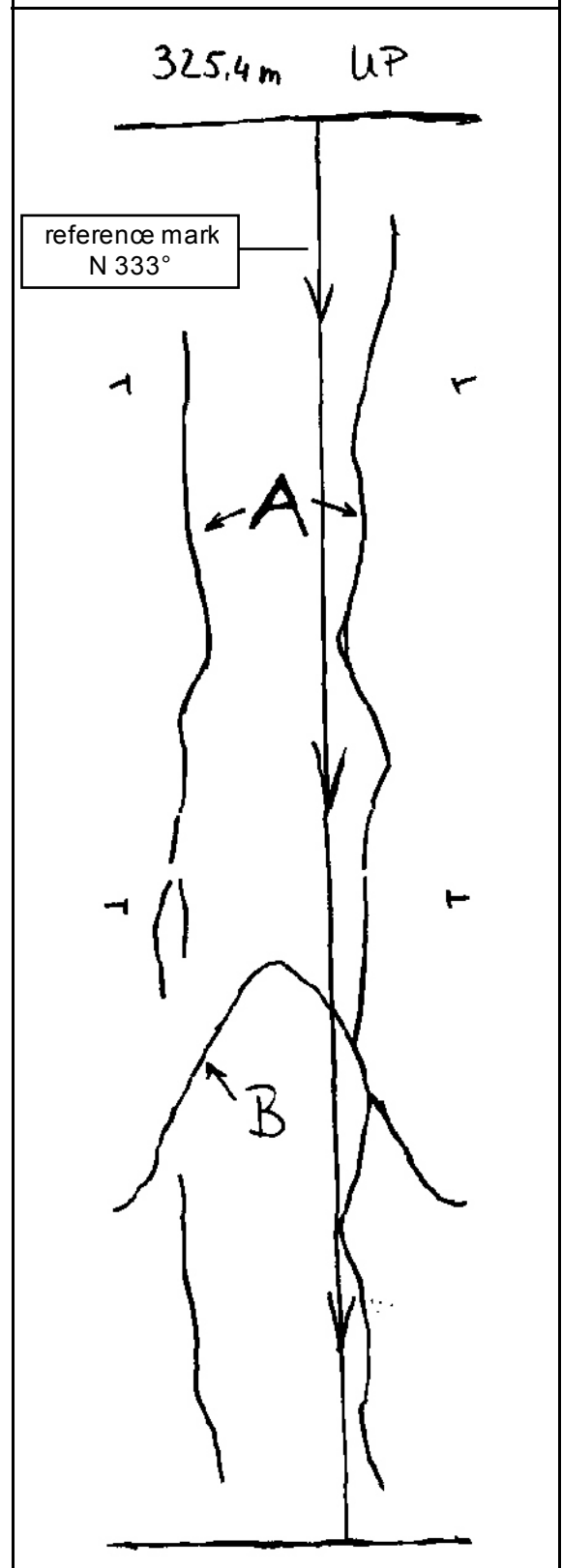
Test at 319.3 m MD / 302.8 m TVD

borehole dip / direction: 21° / N 206°



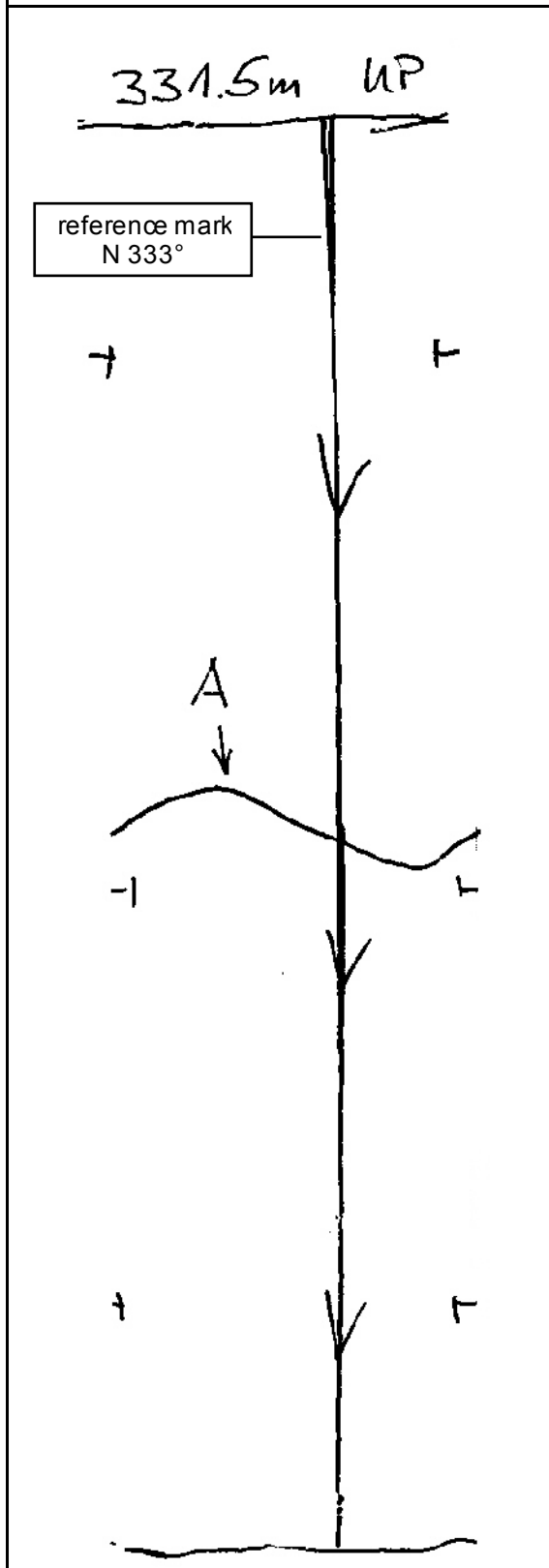
Test at 325.4 m MD / 308.5 m TVD

borehole dip / direction: 21° / N 206°



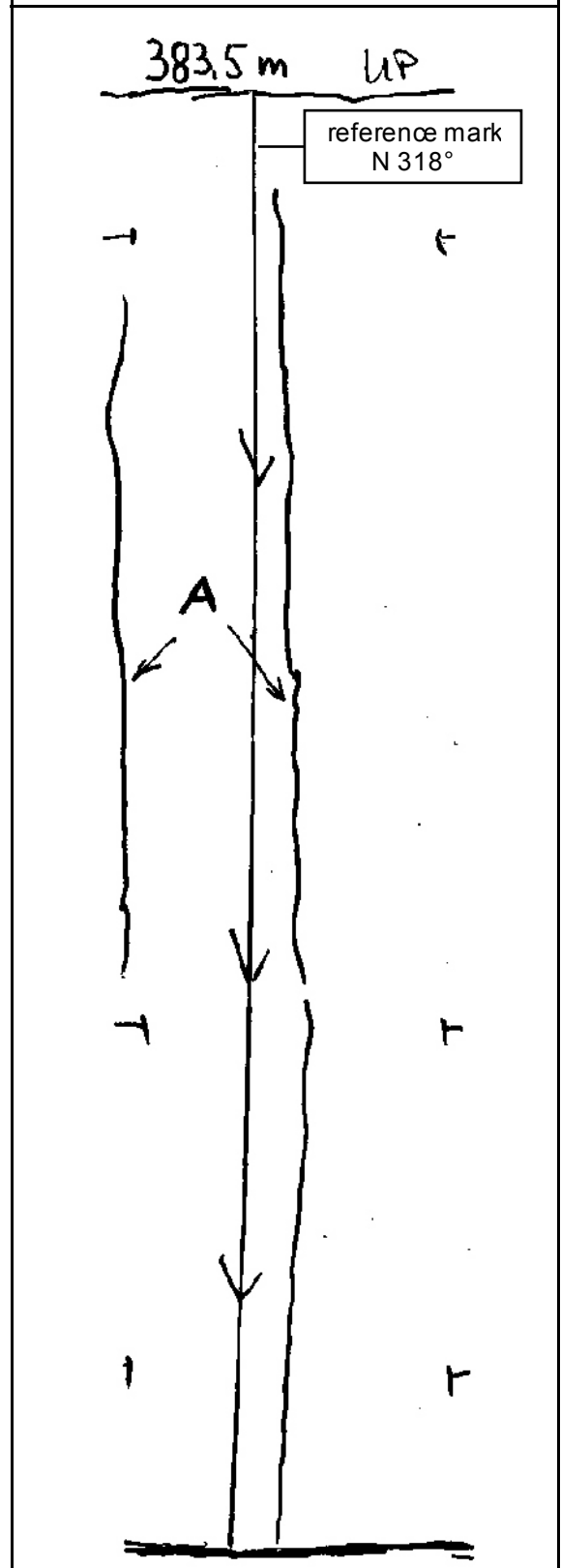
**Test at 331.5 m MD / 314.2 m TVD**

borehole dip / direction: 21° / N 206°



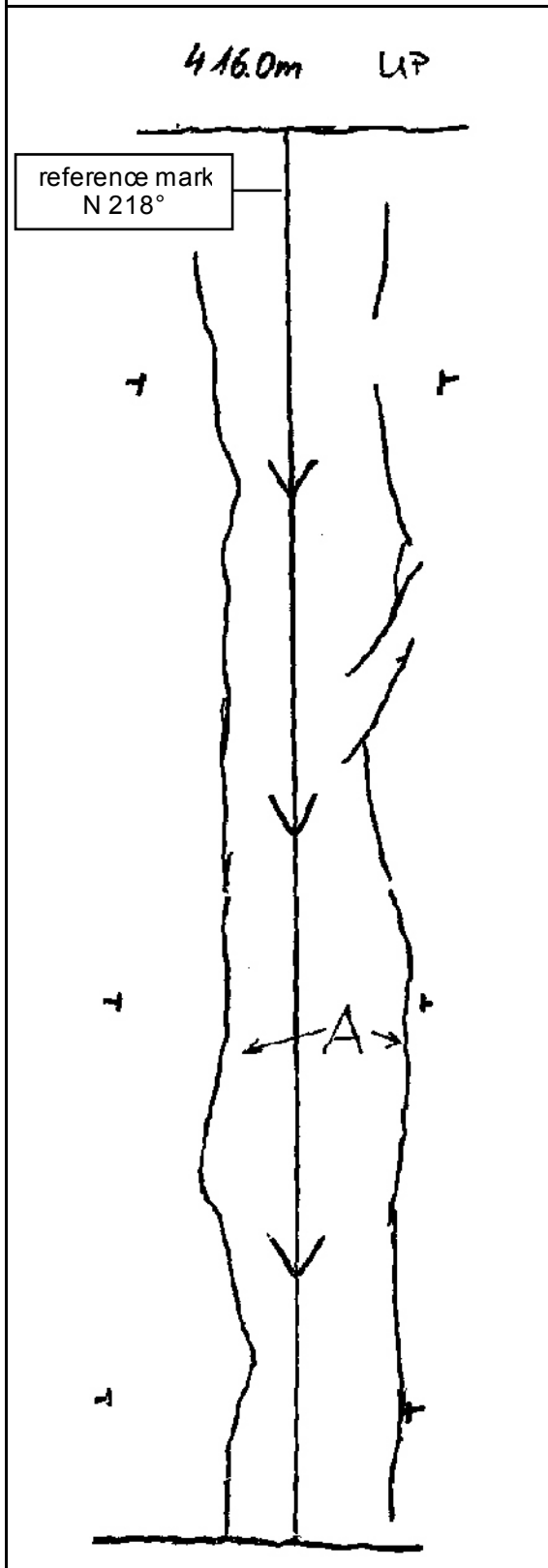
**Test at 383.5 m MD / 363.0 m TVD**

borehole dip / direction: 20° / N 210°



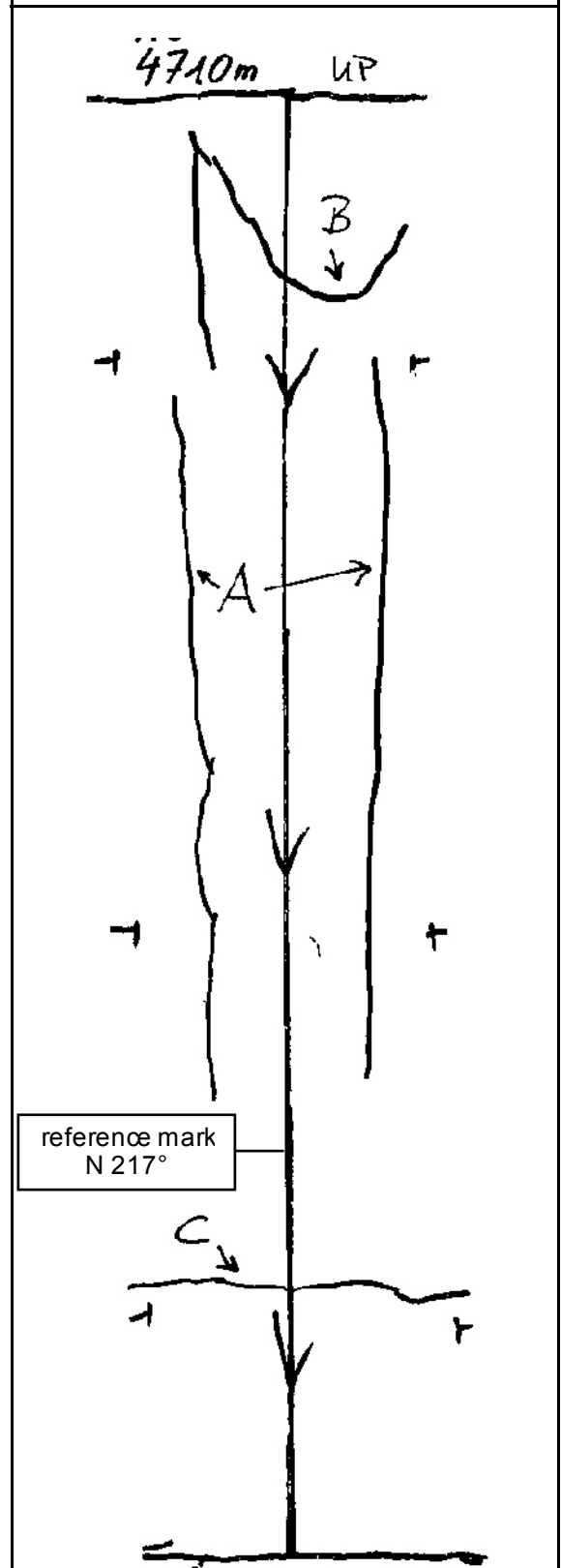
Test at 416.0 m MD / 393.8 m TVD

borehole dip / direction: 17° / N 213°



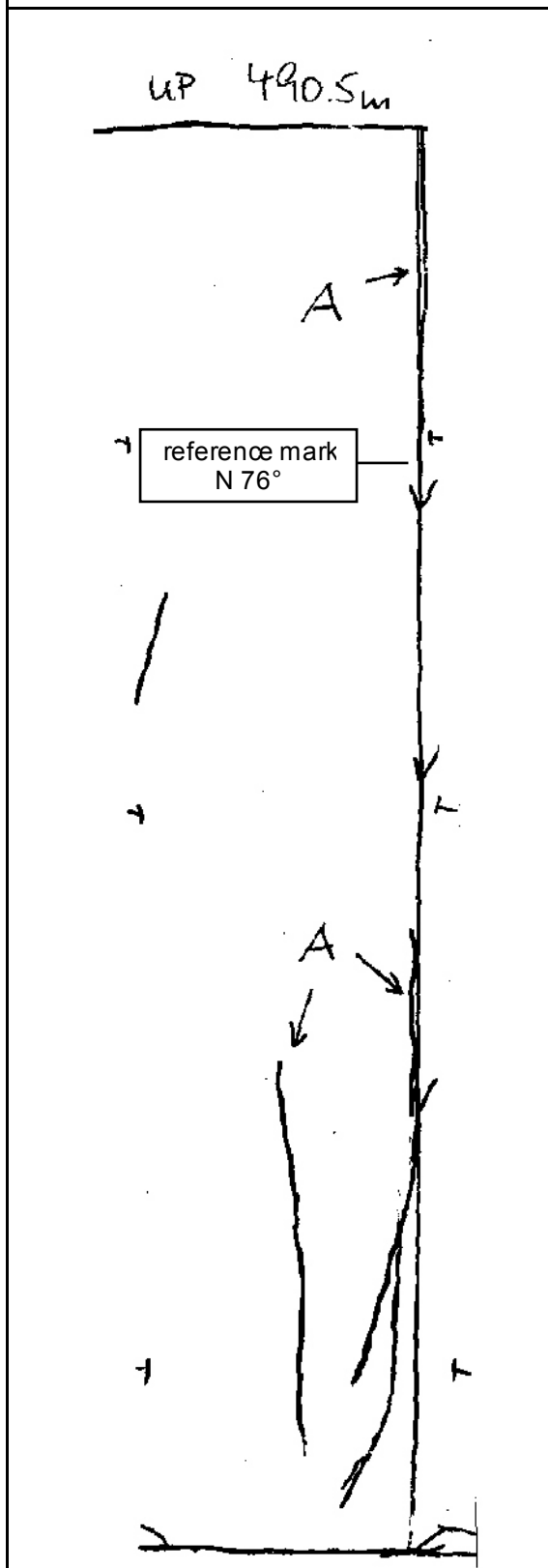
Test at 471.0 m MD / 446.9 m TVD

borehole dip / direction: 14° / N 214°



**Test at 490.5 m MD / 465.8 m TVD**

borehole dip / direction: 14° / N 216°



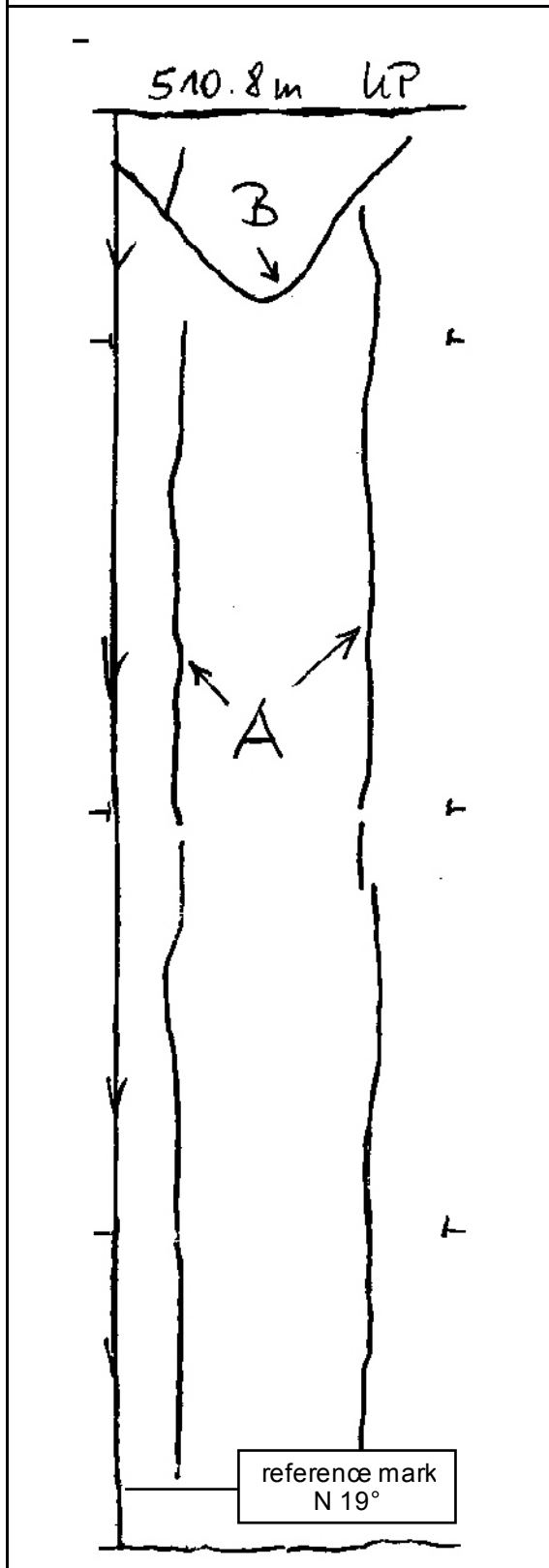
**Test at 499.8 m MD / 474.8 m TVD**

borehole dip / direction: 13° / N 216°



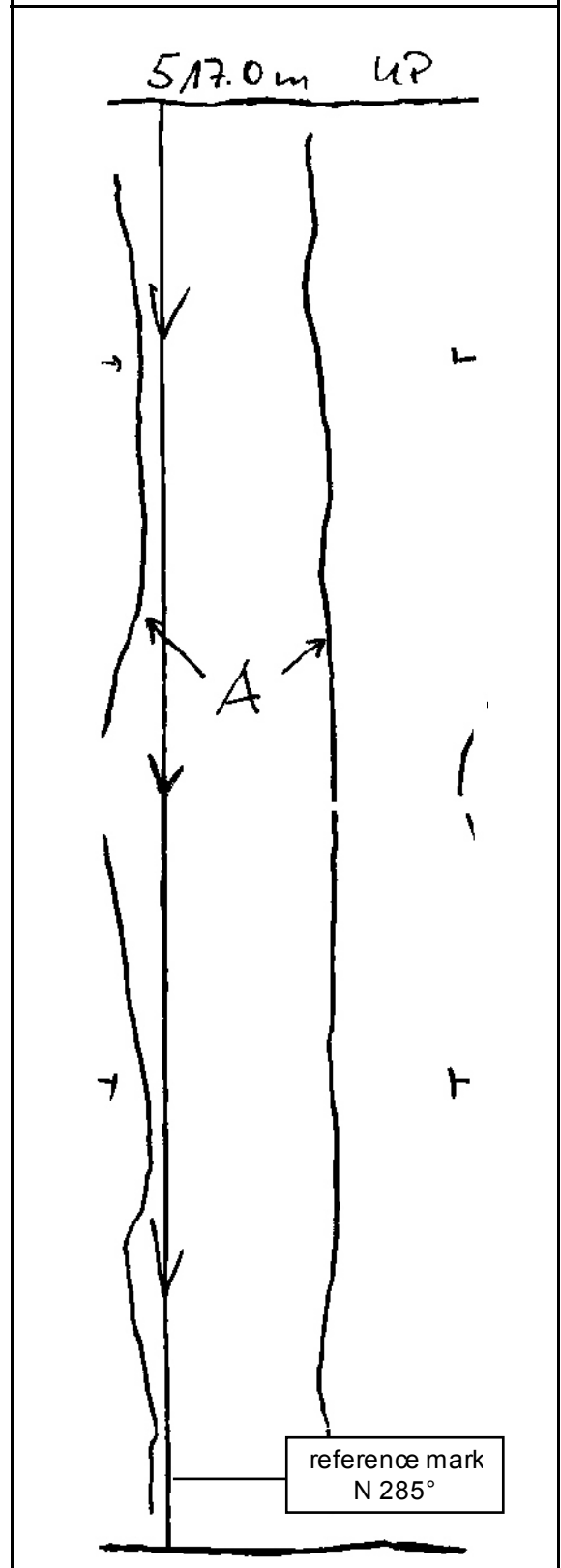
Test at 510.8 m MD / 485.6 m TVD

borehole dip / direction: 13° / N 217°



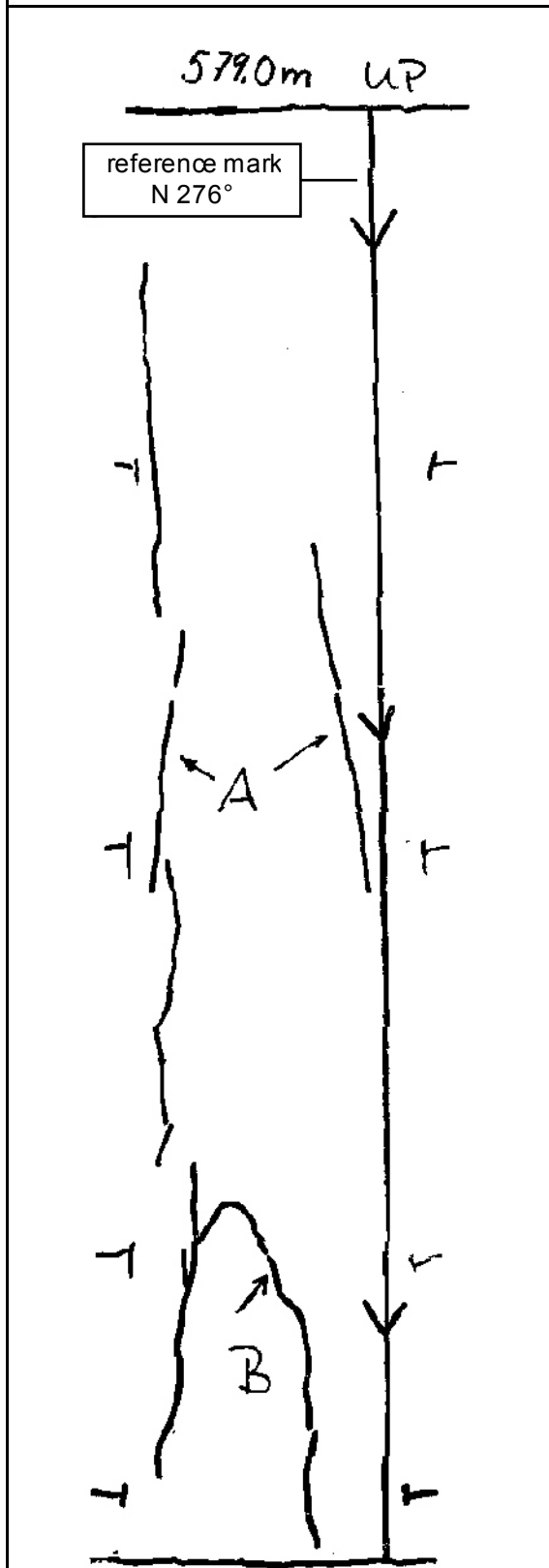
Test at 517.0 m MD / 491.6 m TVD

borehole dip / direction: 13° / N 217°



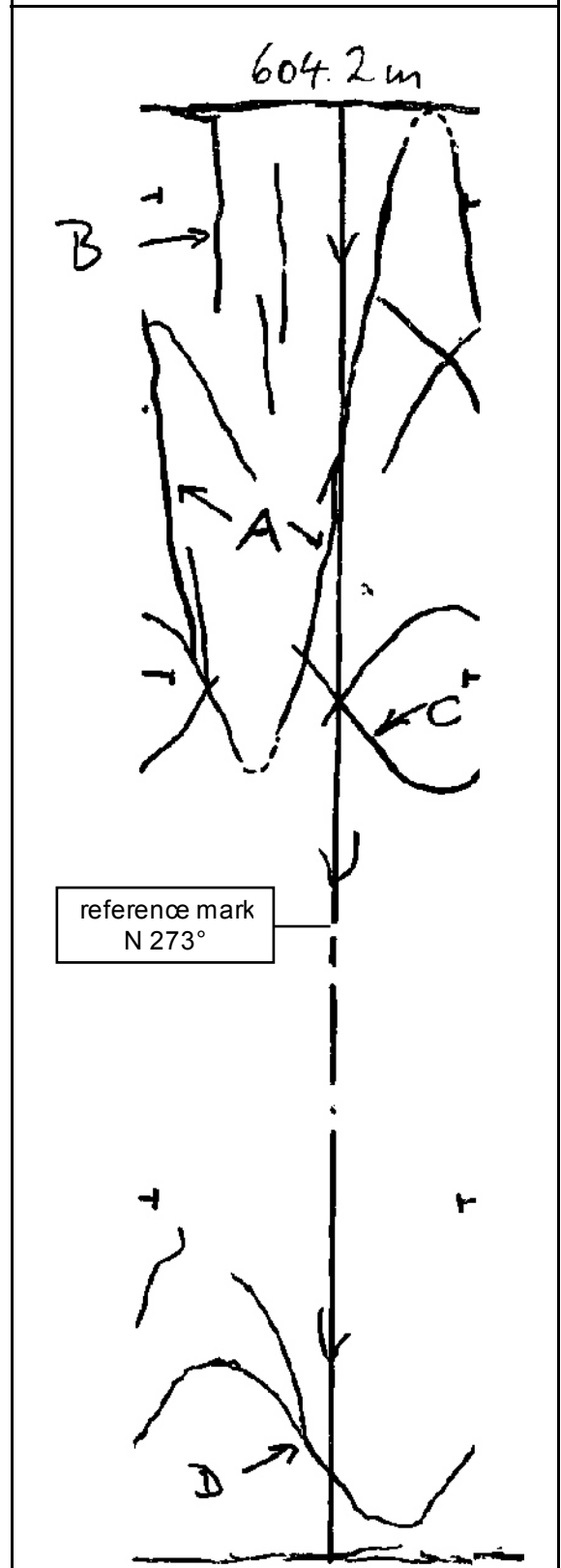
Test at 579.0 m MD / 552.2 m TVD

borehole dip / direction: 11° / N 216°



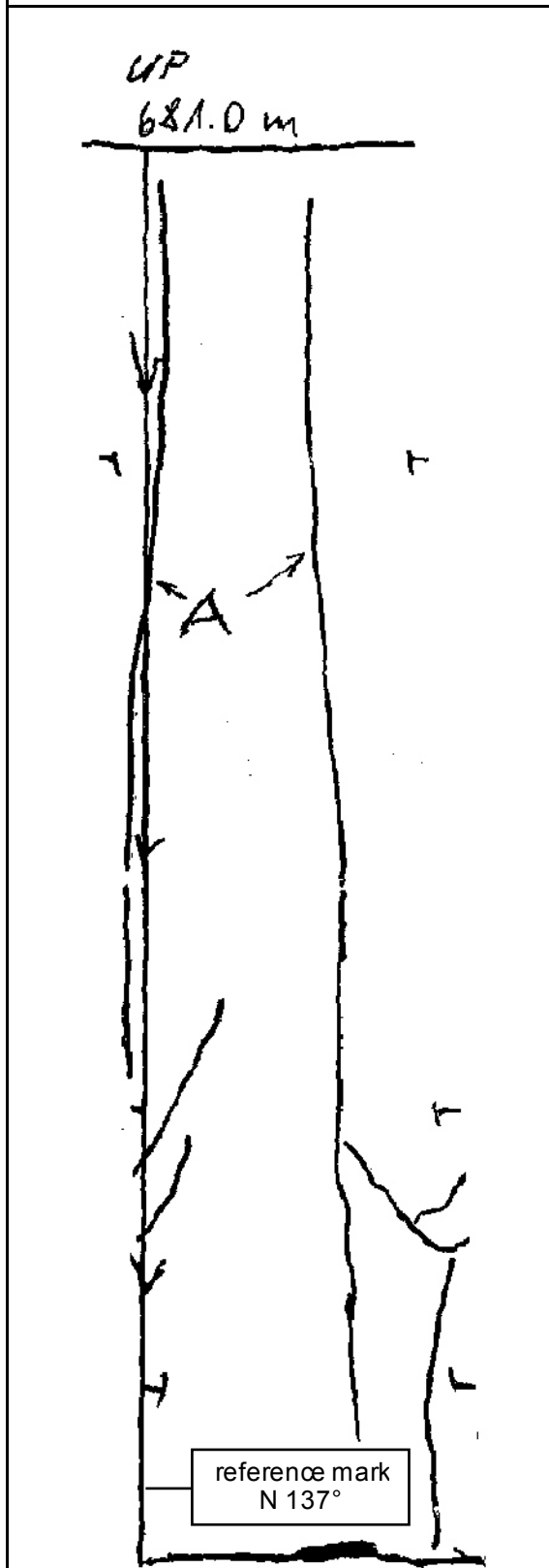
Test at 604.2 m MD / 577.0 m TVD

borehole dip / direction: 10° / N 206°



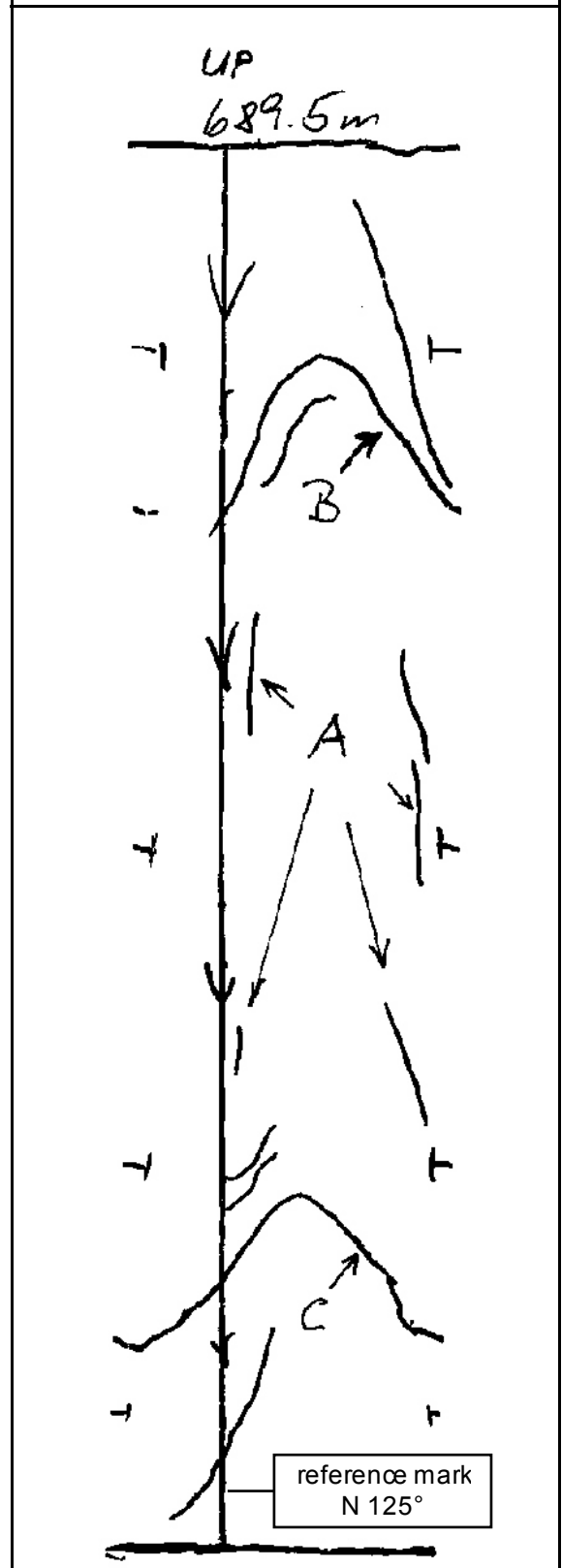
Test at 681.0 m MD / 652.9 m TVD

borehole dip / direction: 8° / N 201°



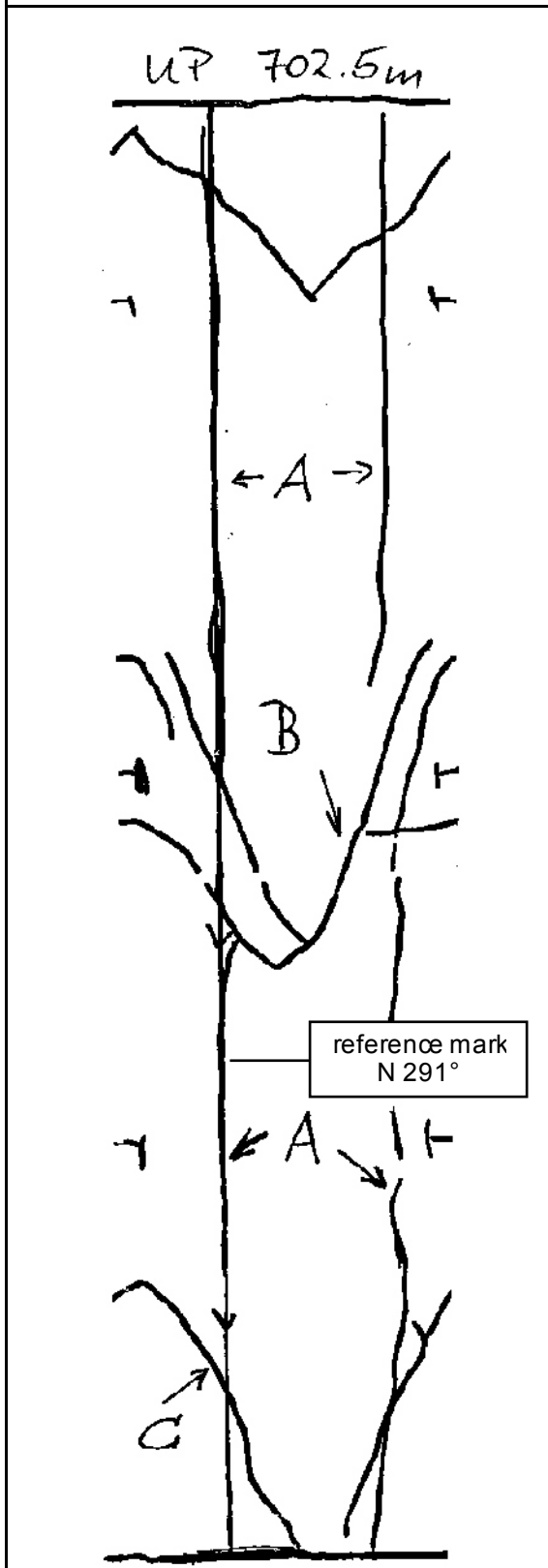
Test at 689.5 m MD / 661.3 m TVD

borehole dip / direction: 8° / N 201°



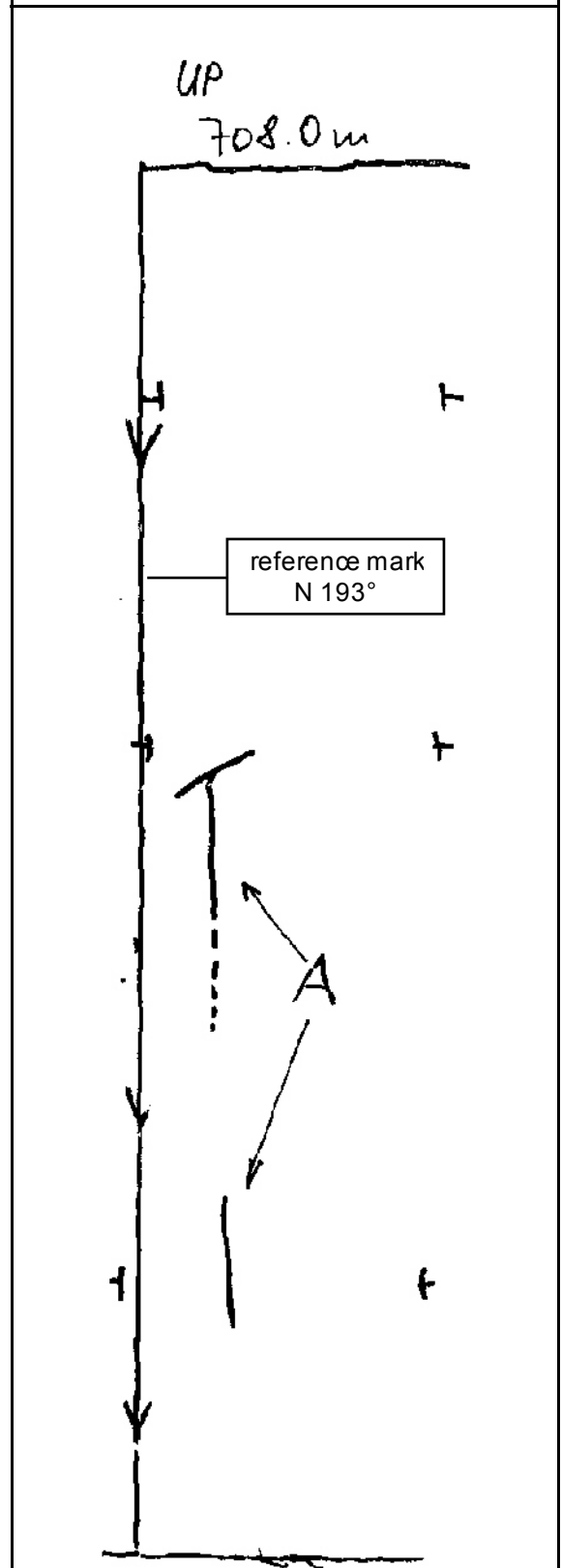
**Test at 702.5 m MD / 674.2 m TVD**

borehole dip / direction: 8° / N 202°



**Test at 708.0 m MD / 679.6 m TVD**

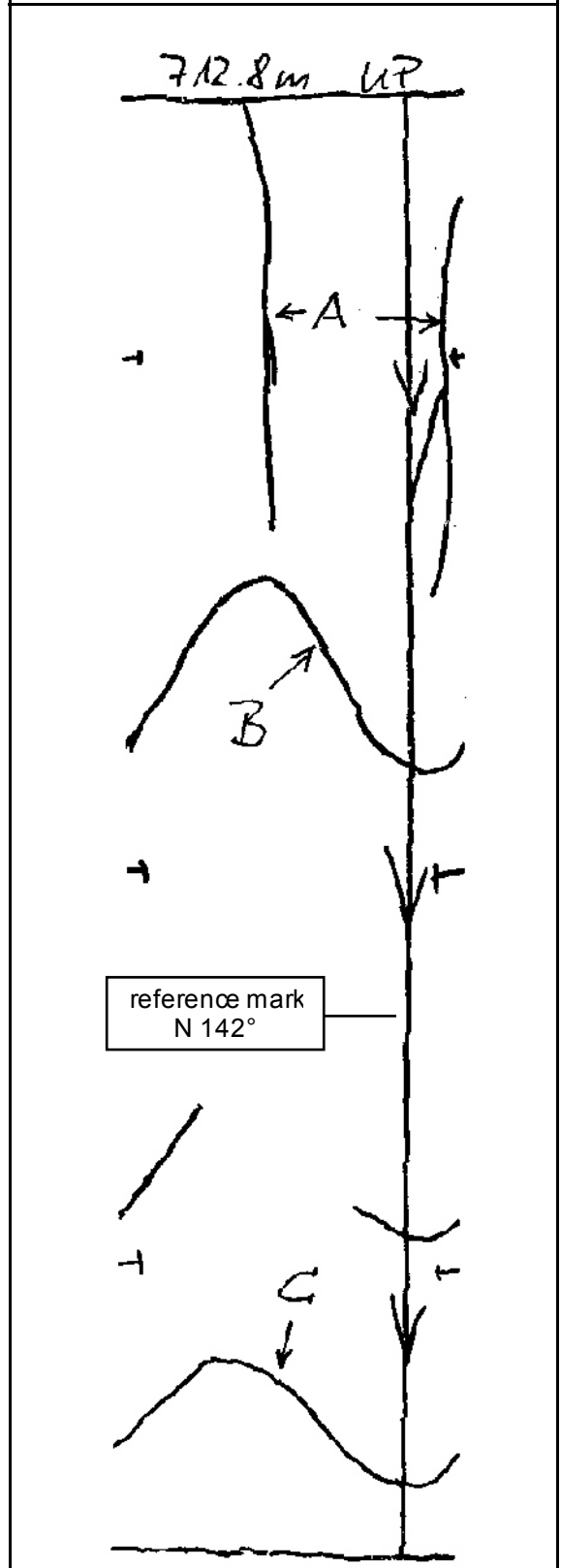
borehole dip / direction: 8° / N 203°





**Test at 712.8 m MD / 684.4 m TVD**

borehole dip / direction: 8° / N 204°



**APPENDIX C**  
**Core piece data sheets**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>		
<b>Project:</b>	<b>5561</b>		
<b>Core Container No.:</b>			
<b>SKB Core No.:</b>			
<b>MeSy Core No.:</b>	<b>1</b>		
<b>Core unconditioned from container</b>		<b>date</b>	<b>:</b>
		<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>2</b>		
<b>Core piece dimensions:</b>			
<b>length (min), mm:</b>	<b>A 300, B 168</b>	<b>diameter (min), mm:</b>	<b>47.6</b>
<b>length (max), mm:</b>	<b>A 328, B 190</b>	<b>diameter (max), mm:</b>	<b>47.6</b>
<b>Core piece(s) mass, g</b>	<b>A 1486.5 B 863.7</b>		
<b>Depth of origin:</b>	<b>min., m:</b>	<b>333.55</b>	
	<b>max., m:</b>	<b>334.03</b>	
<b>Geological Facies:</b>			

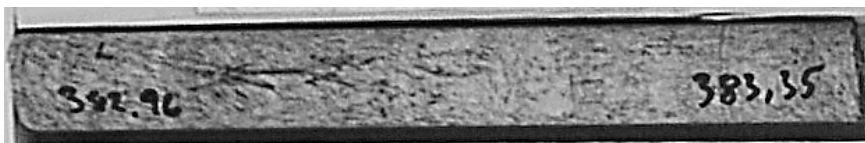
**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>	
<b>Project:</b>	<b>5561</b>	
<b>Core Container No.:</b>		
<b>SKB Core No.:</b>		
<b>MeSy Core No.:</b>	<b>2</b>	
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>
	<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>1</b>	
<b>Core piece dimensions:</b>		
length (min), mm:	<b>390</b>	diameter (min), mm: <b>47.4</b>
length (max), mm:	<b>402</b>	diameter (max), mm: <b>47.4</b>
<b>Core piece(s) mass, g</b>	<b>1880.8</b>	
<b>Depth of origin:</b>	min., m: <b>382.96</b>	
	max., m: <b>383.35</b>	
<b>Geological Facies:</b>		

**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>	
<b>Project:</b>	<b>5561</b>	
<b>Core Container No.:</b>		
<b>SKB Core No.:</b>		
<b>MeSy Core No.:</b>	<b>3</b>	
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>
	<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>3</b>	
<b>Core piece dimensions:</b>		
length (min), mm:	diameter (min), mm: 47.4	
A 140, B 145, C 105	diameter (max), mm: 47.4	
length (max), mm:		
A 150, B 165, C 120		
<b>Core piece(s) mass, g</b>	<b>A 689.5, B 729.2, C 539.2</b>	
<b>Depth of origin:</b>	min., m: 438.12	
	max., m: 438.53	
<b>Geological Facies:</b>		

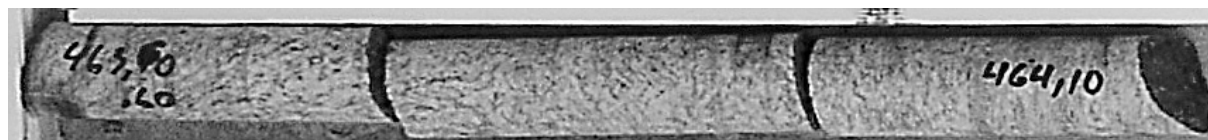
**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>		
<b>Project:</b>	<b>5561</b>		
<b>Core Container No.:</b>			
<b>SKB Core No.:</b>			
<b>MeSy Core No.:</b>	<b>4</b>		
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>	
	<b>operator</b>	<b>:</b>	
<b>Number of core pieces:</b>	<b>3</b>		
<b>Core piece dimensions:</b>			
length (min), mm:	diameter (min), mm: 47.4		
A 160, B 175, C 140	diameter (max), mm: 47.4		
length (max), mm:			
A 175, B 202, C 180			
<b>Core piece(s) mass, g</b>	<b>A 786.7, B 907.6, C 775.5</b>		
<b>Depth of origin:</b>	min., m: 463.60		
	max., m: 464.10		
<b>Geological Facies:</b>			

**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>		
<b>Project:</b>	<b>5561</b>		
<b>Core Container No.:</b>			
<b>SKB Core No.:</b>			
<b>MeSy Core No.:</b>	<b>5</b>		
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>	
	<b>operator</b>	<b>:</b>	
<b>Number of core pieces:</b>	<b>1</b>		
<b>Core piece dimensions:</b>			
length (min), mm:	390	diameter (min), mm:	51.7
length (max), mm:	425	diameter (max), mm:	51.7
<b>Core piece(s) mass, g</b>	<b>2294.9</b>		
<b>Depth of origin:</b>	<b>min., m: 500.20</b>		
	<b>max., m: 500.60</b>		
<b>Geological Facies:</b>			

**Photo**

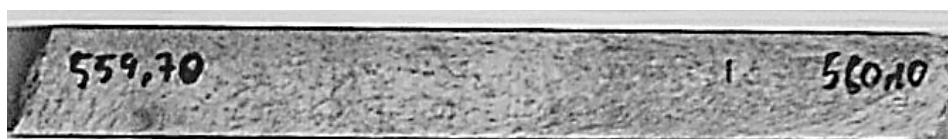




## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>	
<b>Project:</b>	<b>5561</b>	
<b>Core Container No.:</b>		
<b>SKB Core No.:</b>		
<b>MeSy Core No.:</b>	<b>6</b>	
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>
	<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>1</b>	
<b>Core piece dimensions:</b>		
length (min), mm:	<b>400</b>	diameter (min), mm: <b>47.2</b>
length (max), mm:	<b>420</b>	diameter (max), mm: <b>47.2</b>
<b>Core piece(s) mass, g</b>	<b>1961.4</b>	
<b>Depth of origin:</b>	min., m: <b>559.70</b>	
	max., m: <b>560.10</b>	
<b>Geological Facies:</b>		

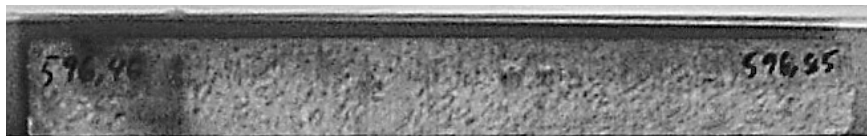
Photo



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>	
<b>Project:</b>	<b>5561</b>	
<b>Core Container No.:</b>		
<b>SKB Core No.:</b>		
<b>MeSy Core No.:</b>	<b>7</b>	
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>
	<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>1</b>	
<b>Core piece dimensions:</b>		
length (min), mm:	<b>390</b>	diameter (min), mm: <b>47.2</b>
length (max), mm:	<b>395</b>	diameter (max), mm: <b>47.2</b>
<b>Core piece(s) mass, g</b>	<b>1837.3</b>	
<b>Depth of origin:</b>	min., m: <b>595.46</b>	
	max., m: <b>596.85</b>	
<b>Geological Facies:</b>		

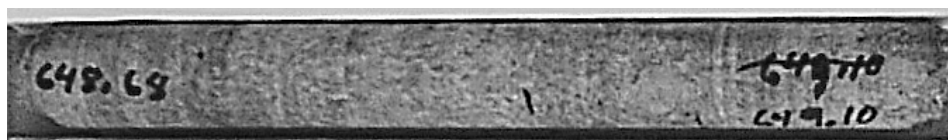
**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>		
<b>Project:</b>	<b>5561</b>		
<b>Core Container No.:</b>			
<b>SKB Core No.:</b>			
<b>MeSy Core No.:</b>	<b>8</b>		
<b>Core unconditioned from container</b>		<b>date</b>	<b>:</b>
		<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>2</b>		
<b>Core piece dimensions:</b>			
<b>length (min), mm:</b>	<b>A 75, B 305</b>	<b>diameter (min), mm:</b>	<b>47.2</b>
<b>length (max), mm:</b>	<b>A 98, B 350</b>	<b>diameter (max), mm:</b>	<b>47.2</b>
<b>Core piece(s) mass, g</b>	<b>A 404.7, B 1568.6</b>		
<b>Depth of origin:</b>	<b>min., m: 648.68</b>		
	<b>max., m: 649.10</b>		
<b>Geological Facies:</b>			

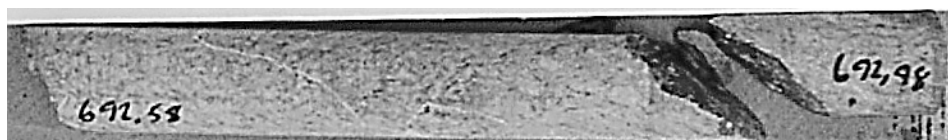
**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>		
<b>Project:</b>	<b>5561</b>		
<b>Core Container No.:</b>			
<b>SKB Core No.:</b>			
<b>MeSy Core No.:</b>	<b>9</b>		
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>	
	<b>operator</b>	<b>:</b>	
<b>Number of core pieces:</b>	<b>2</b>		
<b>Core piece dimensions:</b>			
<b>length (min), mm:</b>	<b>diameter (min), mm: 51.9</b>		
<b>A 255, B 55</b>	<b>diameter (max), mm: 51.9</b>		
<b>length (max), mm:</b>			
<b>A 380, B 120</b>			
<b>Core piece(s) mass, g</b>	<b>A 1817.8, B 504.5</b>		
<b>Depth of origin:</b>	<b>min., m: 692.58</b>		
	<b>max., m: 692.98</b>		
<b>Geological Facies:</b>			

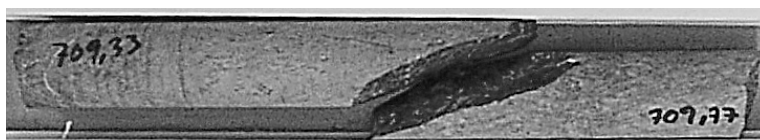
**Photo**



## Core Piece Data Sheet

<b>Client:</b>	<b>SKB</b>	
<b>Project:</b>	<b>5561</b>	
<b>Core Container No.:</b>		
<b>SKB Core No.:</b>		
<b>MeSy Core No.:</b>	<b>10</b>	
<b>Core unconditioned from container</b>	<b>date</b>	<b>:</b>
	<b>operator</b>	<b>:</b>
<b>Number of core pieces:</b>	<b>2</b>	
<b>Core piece dimensions:</b>		
length (min), mm:	diameter (min), mm:	<b>51.9</b>
<b>A 204, B 105</b>	diameter (max), mm:	<b>51.9</b>
length (max), mm:		
<b>A 308, B 240</b>		
<b>Core piece(s) mass, g</b>	<b>A 1461.3, B 1042.7</b>	
<b>Depth of origin:</b>	<b>min., m:</b>	<b>709.33</b>
	<b>max., m:</b>	<b>709.77</b>
<b>Geological Facies:</b>		

**Photo**



**APPENDIX D**  
**Rock density data sheets**



MeSy core-no./ sample-no	depth m	weight in air g	weight in water g	density $\rho$ g/cm <sup>3</sup>
1A1	333.79	465.7	293.2	2.700
1A2		547.3	343.9	2.691
1A2a				2.710
1A2b				2.690
1A3		473.5	297.6	2.692
1B1		438.6	276.9	2.712
1B2		425.1	270.0	2.741
<b>mean</b>				
2A1	383.16	452.6	285.0	2.700
2A2		960.8	605.4	2.703
2A3		467.4	294.8	2.708
2A3a				2.703
2A3b				2.635
<b>mean</b>				
3A	438.33	689.5	434.5	2.704
3B		729.2	457.8	2.687
3C		539.2	337.6	2.675
3Ca				2.669
3Cb				2.667
<b>mean</b>				
4A1	463.85	442.1	278.5	2.702
4A2		344.6	217.4	2.709
4B1		475.9	299.8	2.702
4B1a				2.701
4B1b				2.691
4B2		431.7	271.1	2.688
4C		775.5	488.9	2.706
<b>mean</b>				
5A1	500.40	642.2	405.0	2.707
5A1a				2.683
5A1b				2.700
5A2		1068.1	671.2	2.691
5A3		584.6	367.0	2.687
<b>mean</b>				
6A1	559.90	501.0	315.7	2.704
6A1a				2.697
6A1b				2.707
6A2		904.4	575.5	2.750
6A3		556.0	354.1	2.753



<b>mean</b>				<b>2.722±.027</b>
7A1	596.65	477.1	300.6	2.703
7A1a				2.706
7A1b				2.695
7A2		881.9	556.7	2.712
7A3		478.3	303.0	2.728
<b>mean</b>				<b>2.709±.012</b>
8A	648.89	404.7	254.2	2.689
8B1				2.711
8B2				2.698
8B2a				2.689
8B2b				2.710
8B3		464.8	293.2	2.709
<b>mean</b>				<b>2.701±.010</b>
9A1	692.78	889.9	558.7	2.687
9A1a				2.685
9A1b				2.672
9A2		927.9	584.4	2.701
9B		504.5	318.3	2.709
<b>mean</b>				<b>2.691±.014</b>
10A1	709.55	611.6	379.2	2.632
10A1a				2.626
10A1b				2.624
10A2		849.7	527.4	2.636
10B		1042.7	651.6	2.666
<b>mean</b>				<b>2.637±.017</b>

**APPENDIX E**  
**Ultrasonic velocity data sheets**



		radial		axial	
MeSy core-no./ sample-no	depth m	V <sub>p</sub> km/s	V <sub>s</sub> km/s	V <sub>p</sub> km/s	V <sub>s</sub> km/s
1A 1A2a 1A2b 1B	333.79	5.52	3.19	5.38 5.55	2.39 3.05
<b>mean</b>		<b>5.47±.08</b>	<b>3.14±.07</b>	<b>5.47±.12</b>	<b>2.72±.48</b>
2A 2A3a 2A3b	383.16	5.41	3.07	5.59 5.56	3.21 3.09
<b>mean</b>		<b>5.41</b>	<b>3.07</b>	<b>5.58±.02</b>	<b>3.15±.08</b>
3A 3B 3C 3Ca 3Cb	438.33	5.33 5.51 5.39	3.08 3.10 3.06	5.38 5.50	3.05 3.08
<b>mean</b>		<b>5.41±.09</b>	<b>3.08±.02</b>	<b>5.44±.08</b>	<b>3.07±.02</b>
4A 4B 4B1a 4B1b 4C	463.85	5.45 5.39	3.12 3.10	5.54 5.56	3.11 3.22
<b>mean</b>		<b>5.47±.10</b>	<b>3.11±.01</b>	<b>5.55±.01</b>	<b>3.17±.08</b>
5A 5A1a 5A1b	500.40	5.31	3.01	5.46 5.53	3.08 3.16
<b>mean</b>		<b>5.31</b>	<b>3.01</b>	<b>5.50±.05</b>	<b>3.12±.06</b>
6A 6A1a 6A1b	559.90	5.36	2.99	5.34 5.49	3.05 3.19
<b>mean</b>		<b>5.36</b>	<b>2.99</b>	<b>5.42±.11</b>	<b>3.12±.09</b>
7A 7A1a 7A1b	596.65	5.83	3.05	5.60 5.68	3.15 3.25
<b>mean</b>		<b>5.83</b>	<b>3.05</b>	<b>5.64±.06</b>	<b>3.20±.07</b>
8A 8B2a 8B2b 8B	648.89	5.19 5.32	2.99 3.03	5.50 5.53	3.14 3.17
<b>mean</b>		<b>5.26±.09</b>	<b>3.01±.03</b>	<b>5.52±.02</b>	<b>3.16±.02</b>

		radial		axial	
MeSy core-no./ sample-no	depth m	v <sub>p</sub> km/s	v <sub>s</sub> km/s	v <sub>p</sub> km/s	v <sub>s</sub> km/s
9A	692.78	5.30	2.98		
9A1a				5.52	3.14
9A1b				5.35	3.00
9B		5.46	3.04		
<b>mean</b>		<b>5.38±.11</b>	<b>3.01±.04</b>	<b>5.44±.12</b>	<b>3.07±.10</b>
10A	709.55	4.90	2.75		
10A1a				5.07	2.94
10A1b				5.02	2.89
10B		5.19	2.97		
<b>mean</b>		<b>5.05±.21</b>	<b>2.86±.16</b>	<b>5.05±.04</b>	<b>2.92±.04</b>

**APPENDIX F**  
**Elasticity data sheets**



		radial		axial	
MeSy core-no./ sample-no	depth m	E <sub>dyn</sub> GPa	V <sub>dyn</sub>	E <sub>dyn</sub> GPa	V <sub>dyn</sub>
1A	333.79	69	0.25		
1A2a				59	0.29
1A2b				64	0.28
1B		65	0.26		
<b>mean</b>		<b>67±3</b>	<b>0.26±.01</b>	<b>62±4</b>	<b>0.29±.01</b>
2A	383.16	64	0.26		
2A3a				70	0.25
2A3b				66	0.28
<b>mean</b>		<b>64</b>	<b>0.26</b>	<b>68±3</b>	<b>0.27±.02</b>
3A	438.33	64	0.25		
3B		66	0.27		
3C		64	0.26		
3Ca				63	0.26
3Cb				65	0.27
<b>mean</b>		<b>65±1</b>	<b>0.26±.01</b>	<b>64±1</b>	<b>0.27±.01</b>
4A	463.85	66	0.26		
4B		65	0.25		
4B1a				66	0.27
4B1b				70	0.25
4C		67	0.27		
<b>mean</b>		<b>66±1</b>	<b>0.26±.01</b>	<b>68±3</b>	<b>0.26±.01</b>
5A	500.40	62	0.26		
5A1a				65	0.27
5A1b				68	0.26
6A	559.90	62	0.28		
6A1a				63	0.26
6A1b				68	0.25
<b>mean</b>		<b>62</b>	<b>0.28</b>	<b>66±4</b>	<b>0.26±.01</b>
7A	596.65	64	0.26		
7A1a				68	0.27
7A1b				72	0.26
<b>mean</b>		<b>64</b>	<b>0.26</b>	<b>70±3</b>	<b>0.27±.01</b>
8A	648.89	61	0.25		
8B2a		63	0.26		
8B2b				67	0.26
8B				68	0.26
<b>mean</b>		<b>62±1</b>	<b>0.26±.01</b>	<b>68±1</b>	<b>0.26</b>



		radial		axial	
MeSy core-no./ sample-no	depth m	E <sub>dyn</sub> GPa	v <sub>dyn</sub>	E <sub>dyn</sub> GPa	v <sub>dyn</sub>
9A	692.78	61	0.27		
9A1a				67	0.26
9A1b				63	0.26
9B		64	0.28		
<b>mean</b>		<b>63±2</b>	<b>0.28±.01</b>	<b>65±3</b>	<b>0.26</b>
10A	709.55	51	0.27		
10A1a				57	0.25
10A1b				55	0.25
10B		59	0.27		
<b>mean</b>		<b>55±6</b>	<b>0.27</b>	<b>56±1</b>	<b>0.25</b>

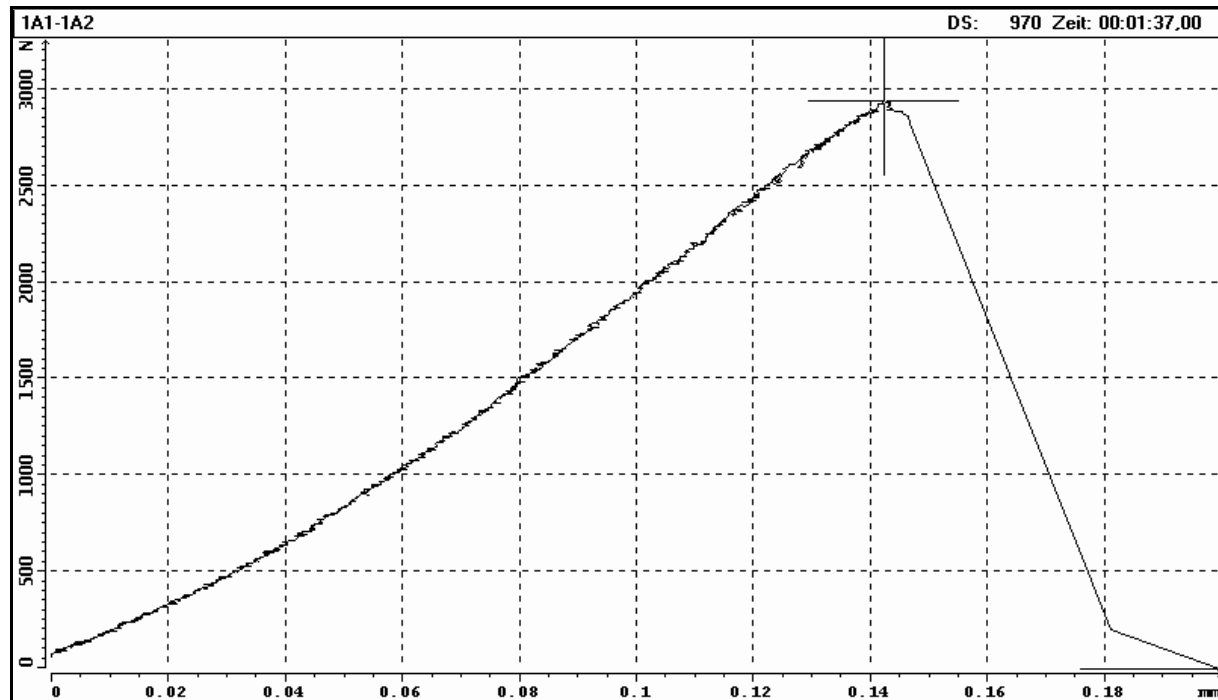
**APPENDIX G**  
**Fracture toughness data sheets**



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	1A1-1A2
mean depth, m:	333.79
diameter, mm:	47.6
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



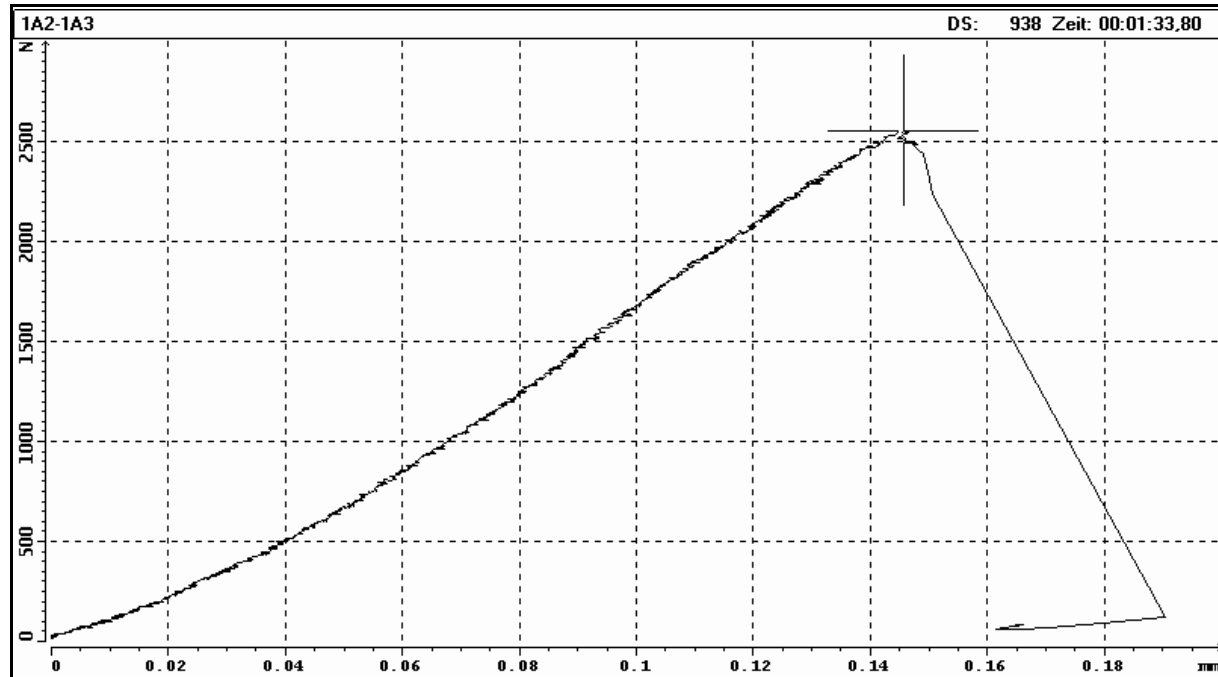
$F_{max}, N$	2927
$K_{IC}, MN \cdot m^{1/2}$	2.90



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	1A2-1A3
mean depth, m:	333.79
diameter, mm:	47.6
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



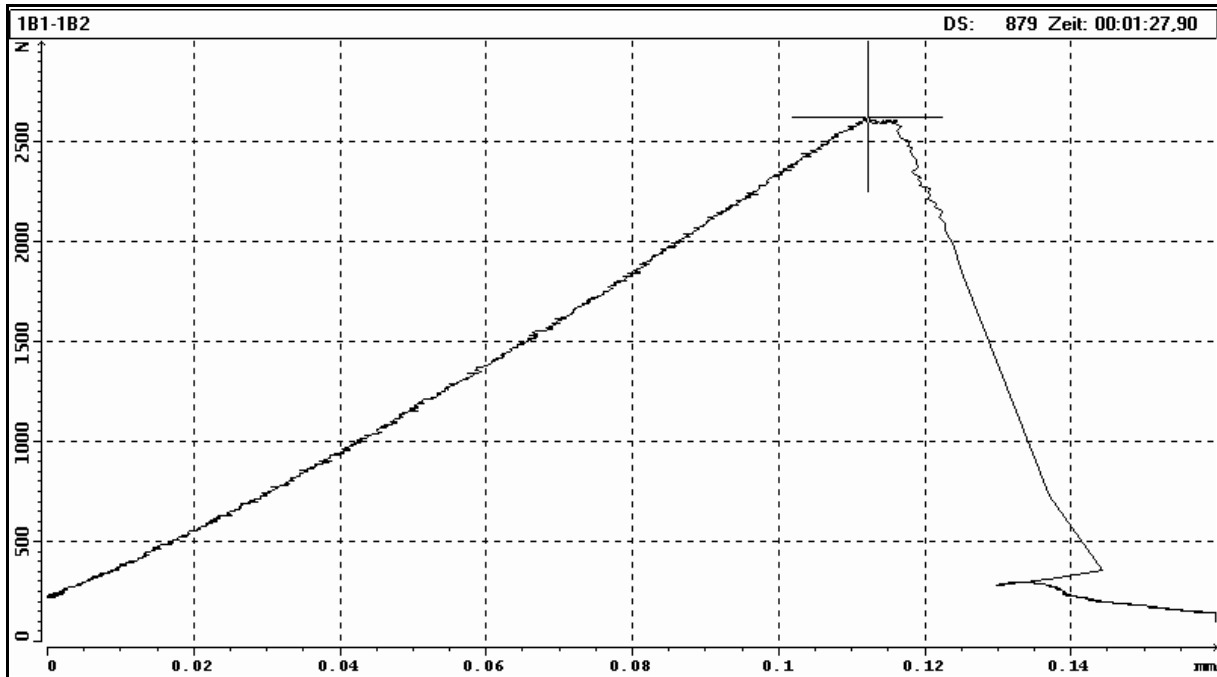
$F_{max}, N$	2547
$K_{IC}, MN \cdot m^{1/2}$	2.53



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	1B1-1B2
mean depth, m:	333.79
diameter, mm:	47.6
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



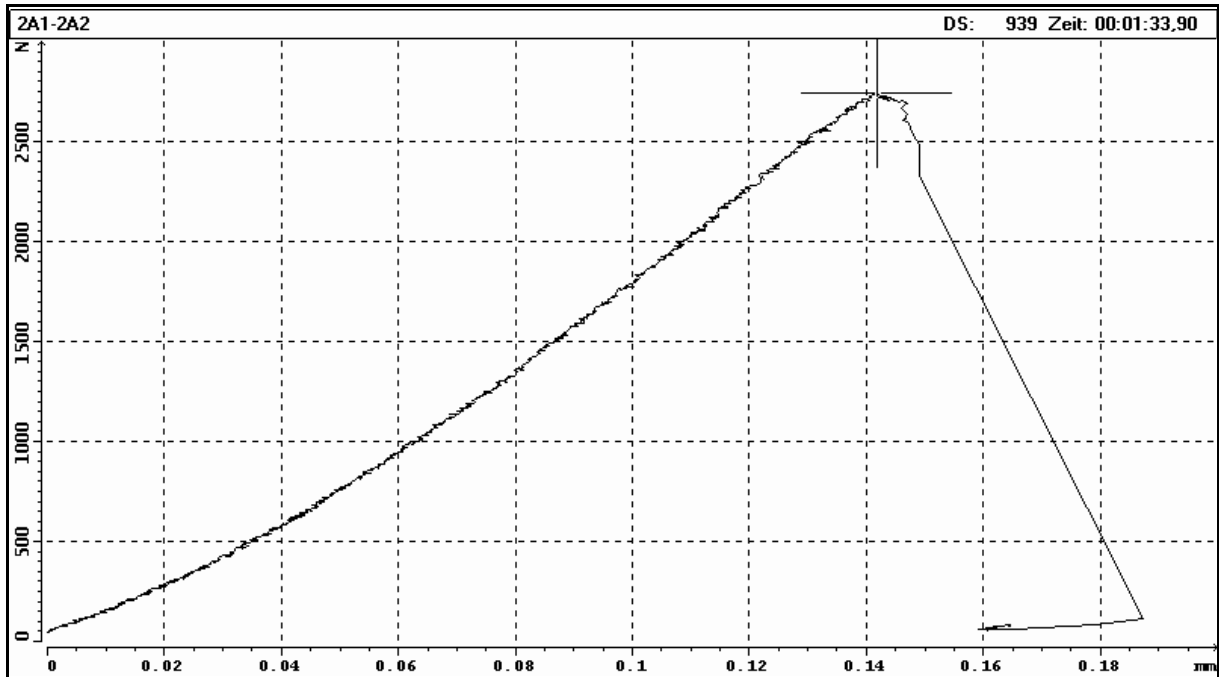
$F_{max}$ , N	2617
$K_{IC}$ , MN·m <sup>1/2</sup>	2.60



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	2A1-2A2
mean depth, m:	383.16
diameter, mm:	47.4
initial crack length, mm:	6.9
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



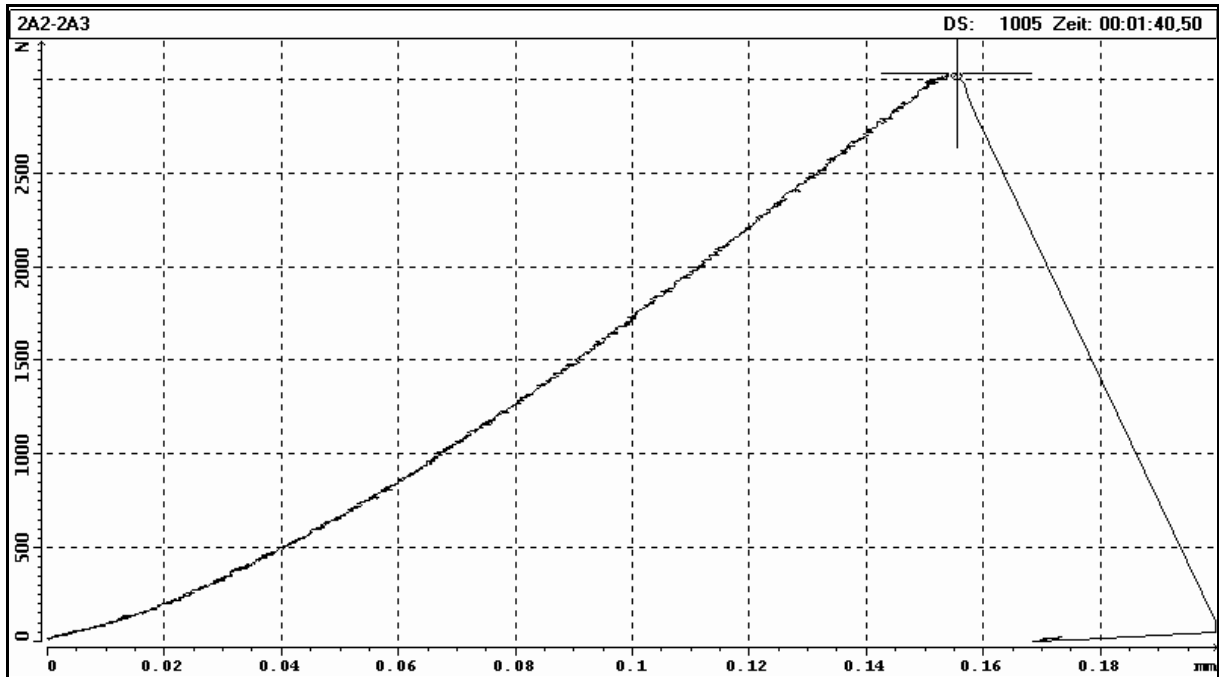
$F_{max}, N$	2736
$K_{IC}, MN \cdot m^{1/2}$	2.73



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	2A2-2A3
mean depth, m:	383.16
diameter, mm:	47.4
initial crack length, mm:	6.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



$F_{max}, N$	3023
$K_{IC}, MN \cdot m^{1/2}$	2.83

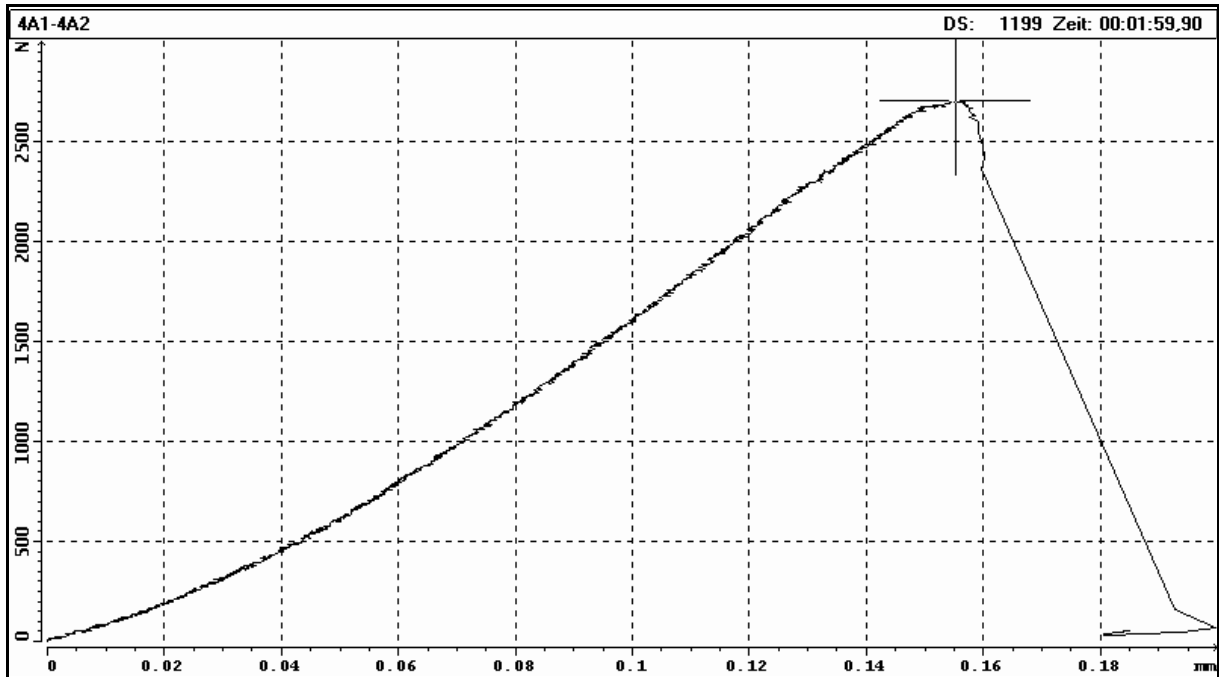




FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	4A1-4A2
mean depth, m:	463.85
diameter, mm:	47.4
initial crack length, mm:	6.8
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



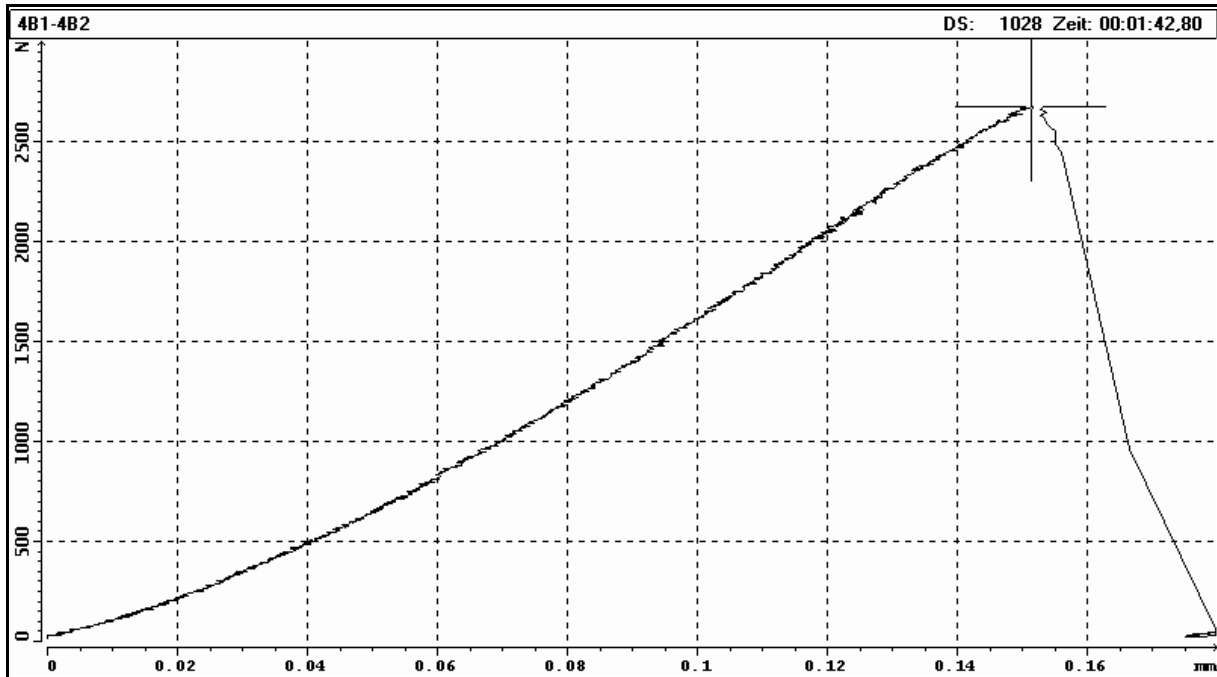
$F_{max}$ , N	2700
$K_{IC}$ , MN·m <sup>1/2</sup>	2.67



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	4B1-4B2
mean depth, m:	463.85
diameter, mm:	47.4
initial crack length, mm:	7.2
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



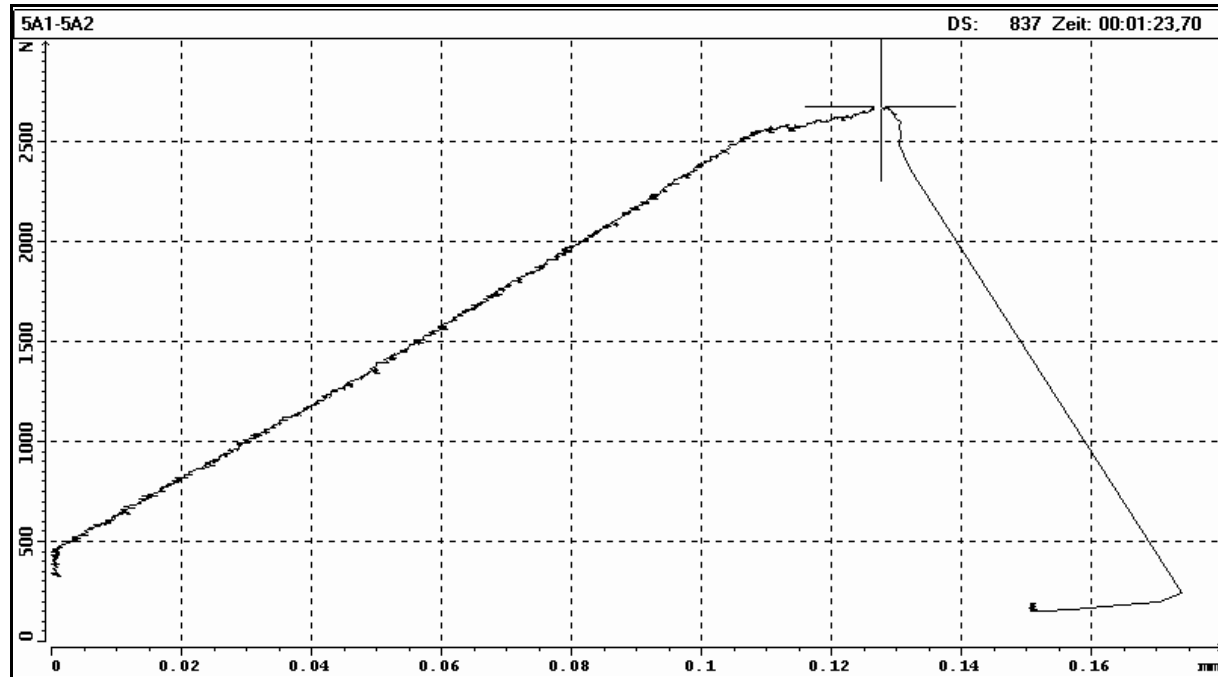
$F_{max}$ , N	2665
$K_{IC}$ , MN·m <sup>1/2</sup>	2.71



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	5A1-5A2
mean depth, m:	500.40
diameter, mm:	51.7
initial crack length, mm:	7.3
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



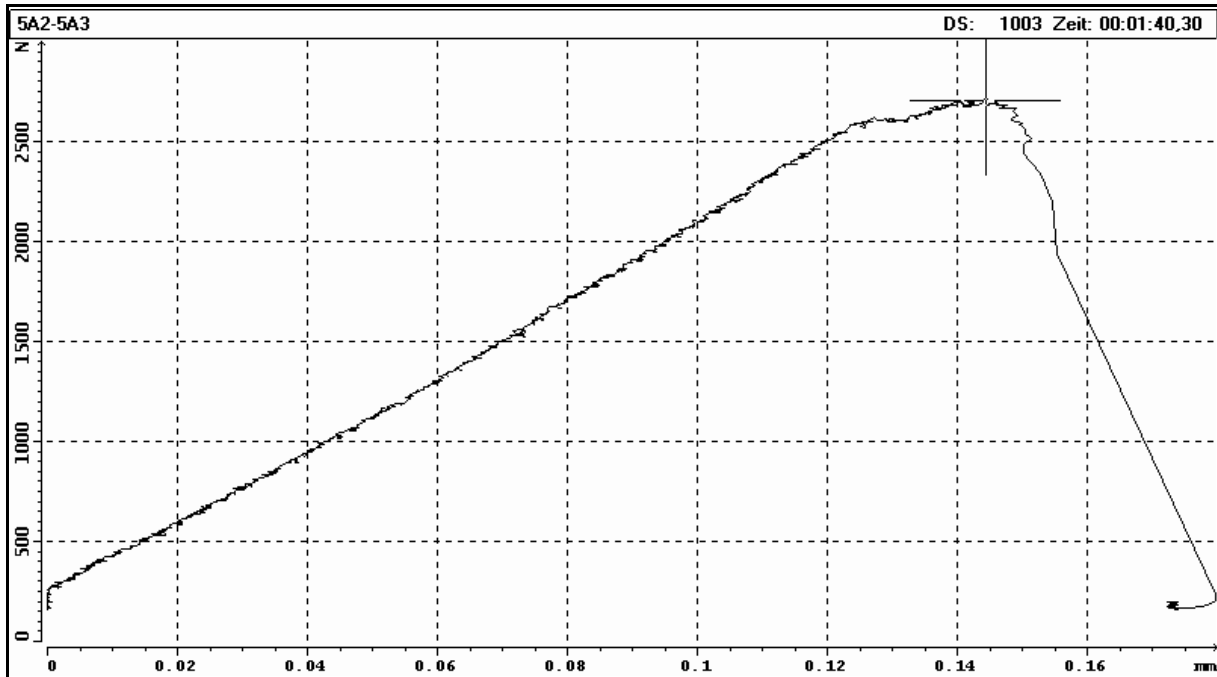
$F_{max}, N$	2672
$K_{IC}, MN \cdot m^{1/2}$	2.30



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	5A2-5A3
mean depth, m:	500.40
diameter, mm:	51.7
initial crack length, mm:	7.1
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



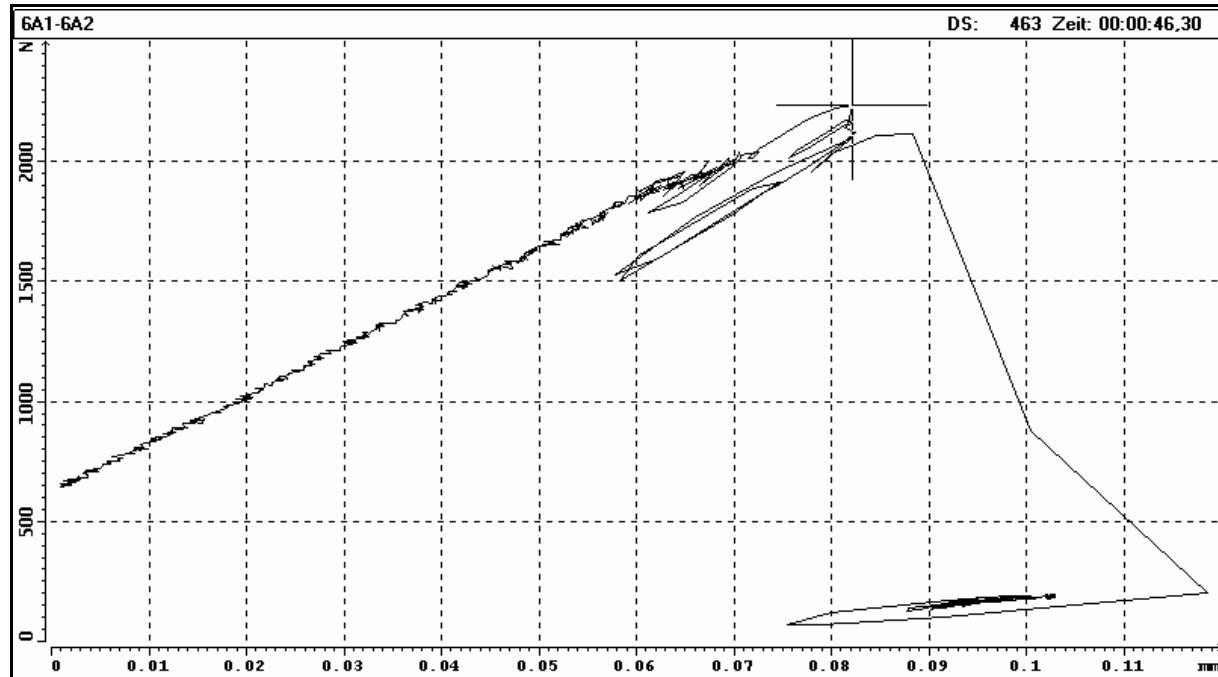
$F_{max}, N$	2698
$K_{IC}, MN \cdot m^{1/2}$	2.30



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	6A1-6A2
mean depth, m:	559.90
diameter, mm:	47.2
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



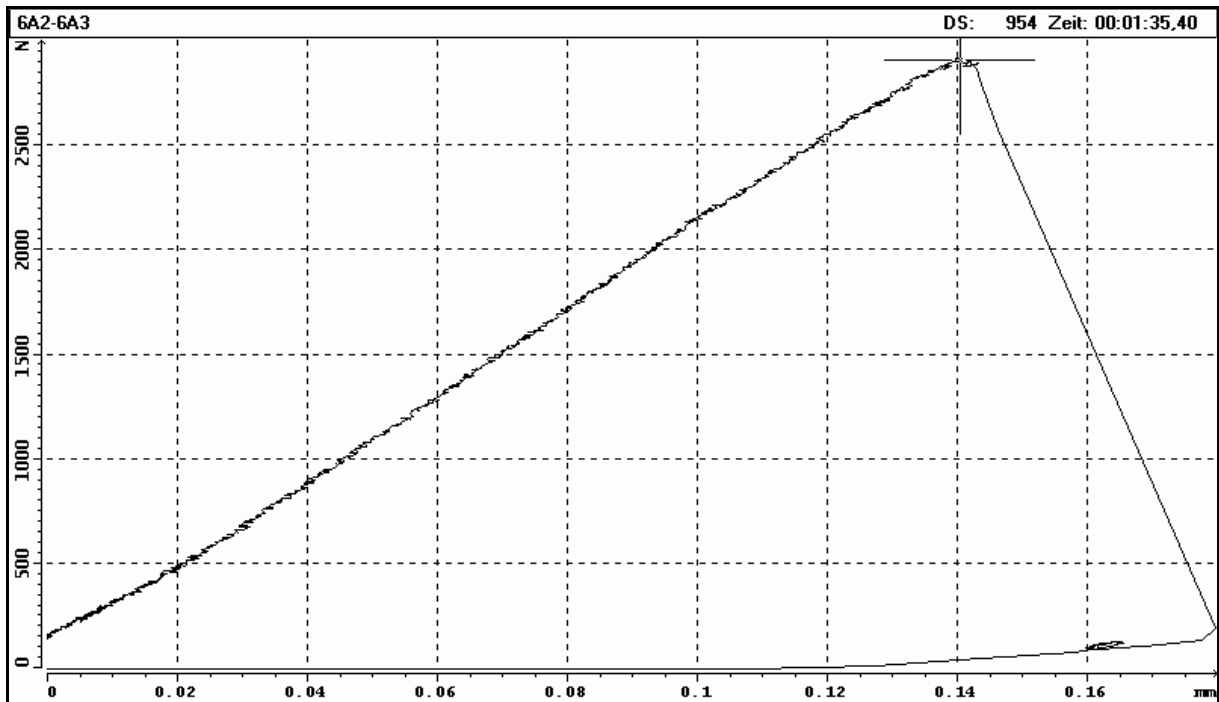
$F_{max}$ , N	2228
$K_{IC}$ , MN·m <sup>1/2</sup>	2.25



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	6A2-6A3
mean depth, m:	559.90
diameter, mm:	47.2
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



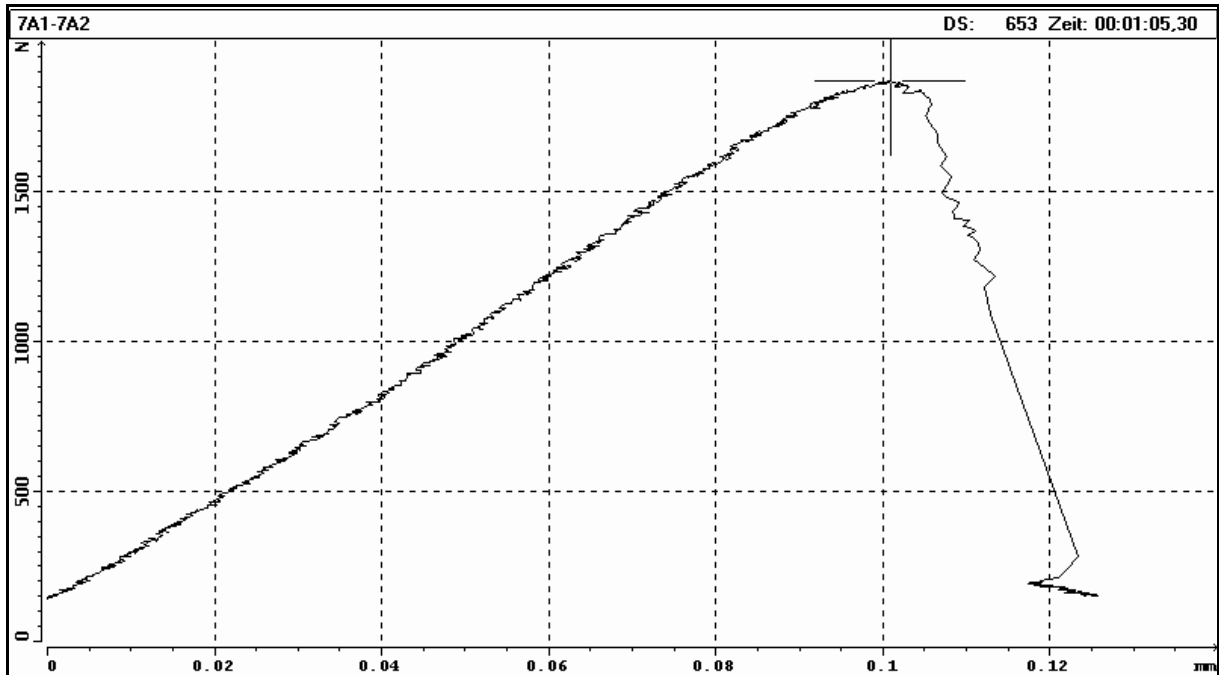
$F_{max}, N$	2900
$K_{IC}, MN \cdot m^{1/2}$	2.93



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	7A1-7A2
mean depth, m:	596.65
diameter, mm:	47.2
initial crack length, mm:	7.1
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



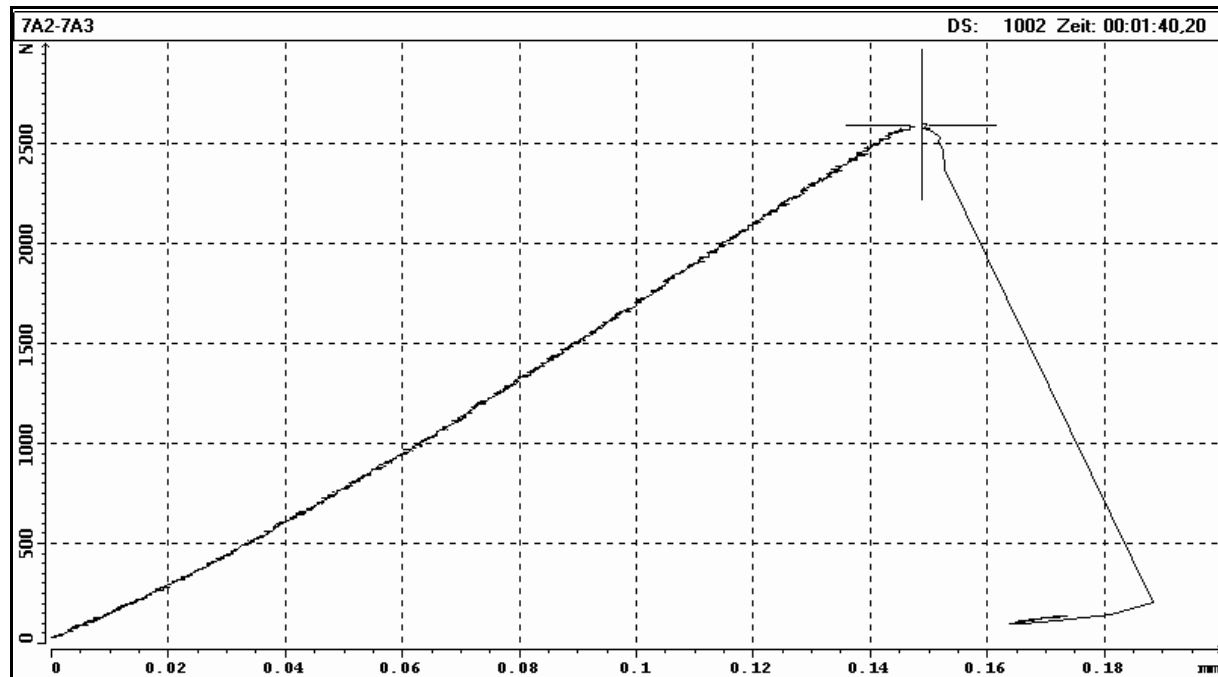
$F_{max}$ , N	1866
$K_{IC}$ , $MN \cdot m^{1/2}$	1.90



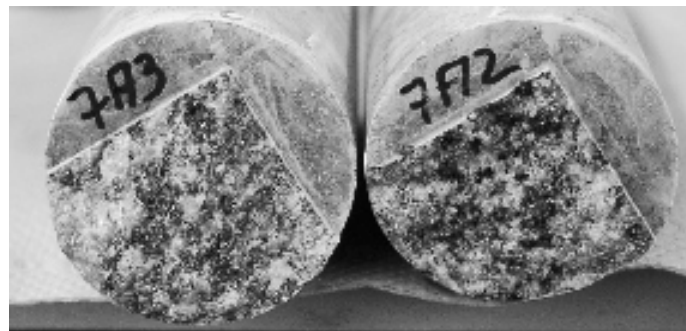
FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	7A2-7A3
mean depth, m:	596.65
diameter, mm:	47.2
initial crack length, mm:	7.3
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



$F_{max}$ , N	2585
$K_{IC}$ , MN·m <sup>1/2</sup>	2.67

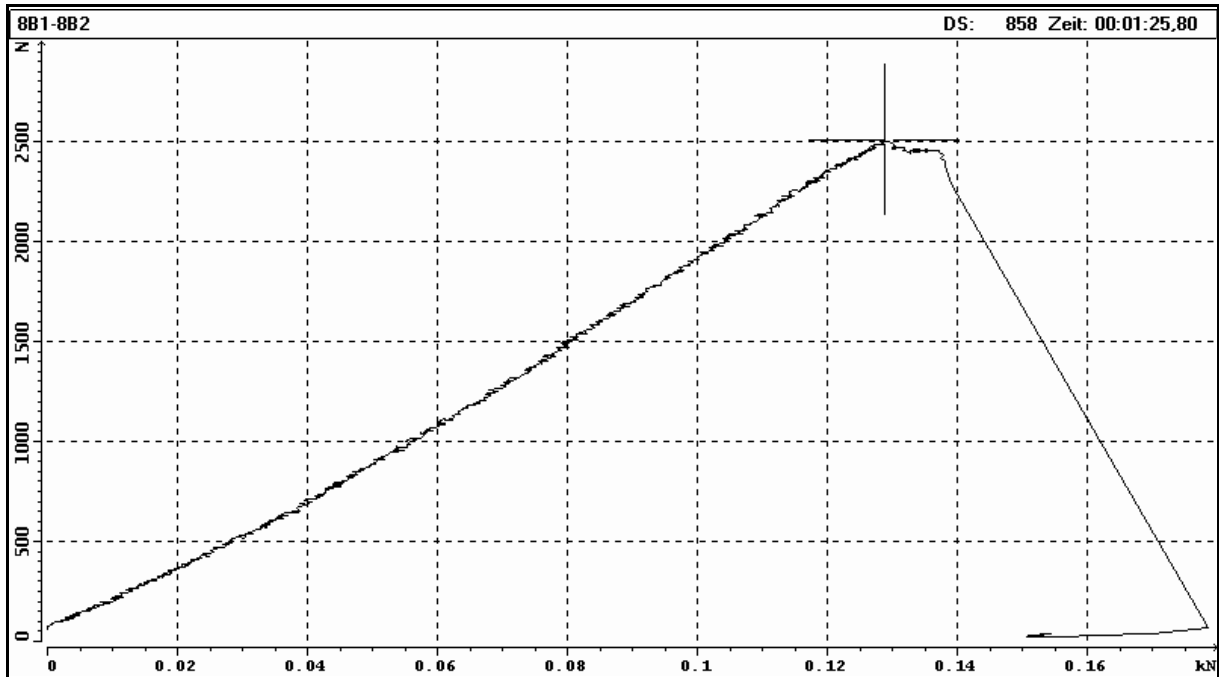




FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	8B1-8B2
mean depth, m:	648.89
diameter, mm:	47.3
initial crack length, mm:	7.2
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



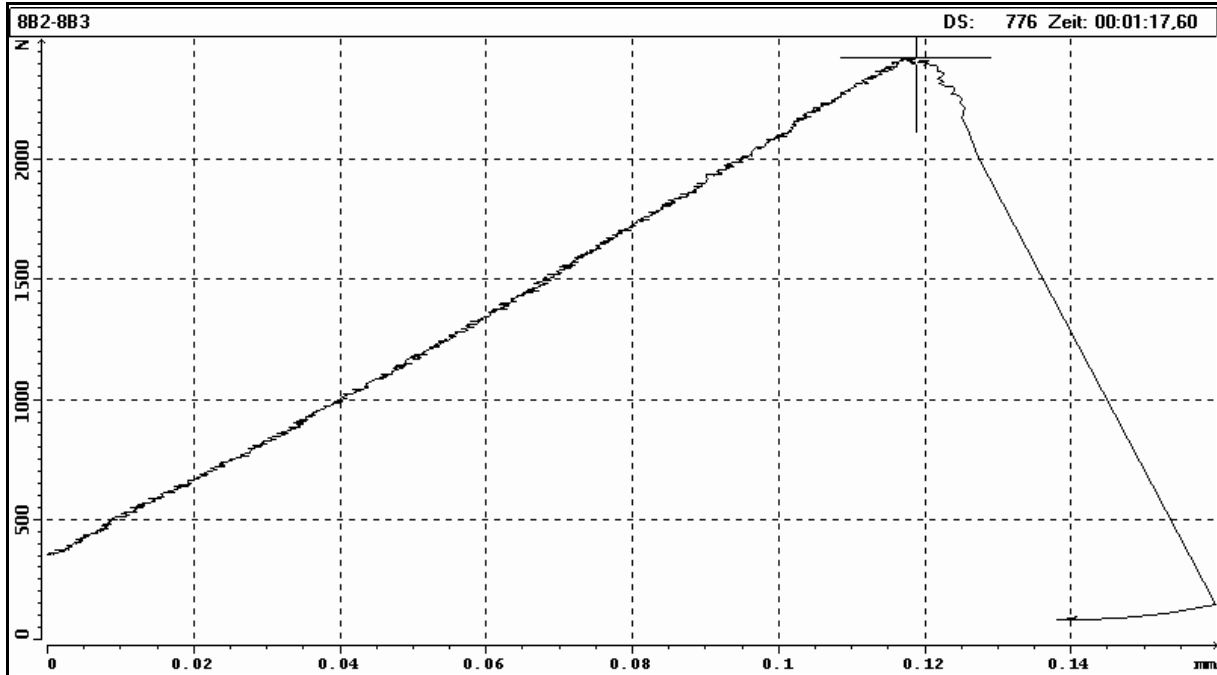
$F_{max}, N$	2500
$K_{IC}, MN \cdot m^{1/2}$	2.55



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	8B2-8B3
mean depth, m:	648.89
diameter, mm:	47.3
initial crack length, mm:	6.6
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



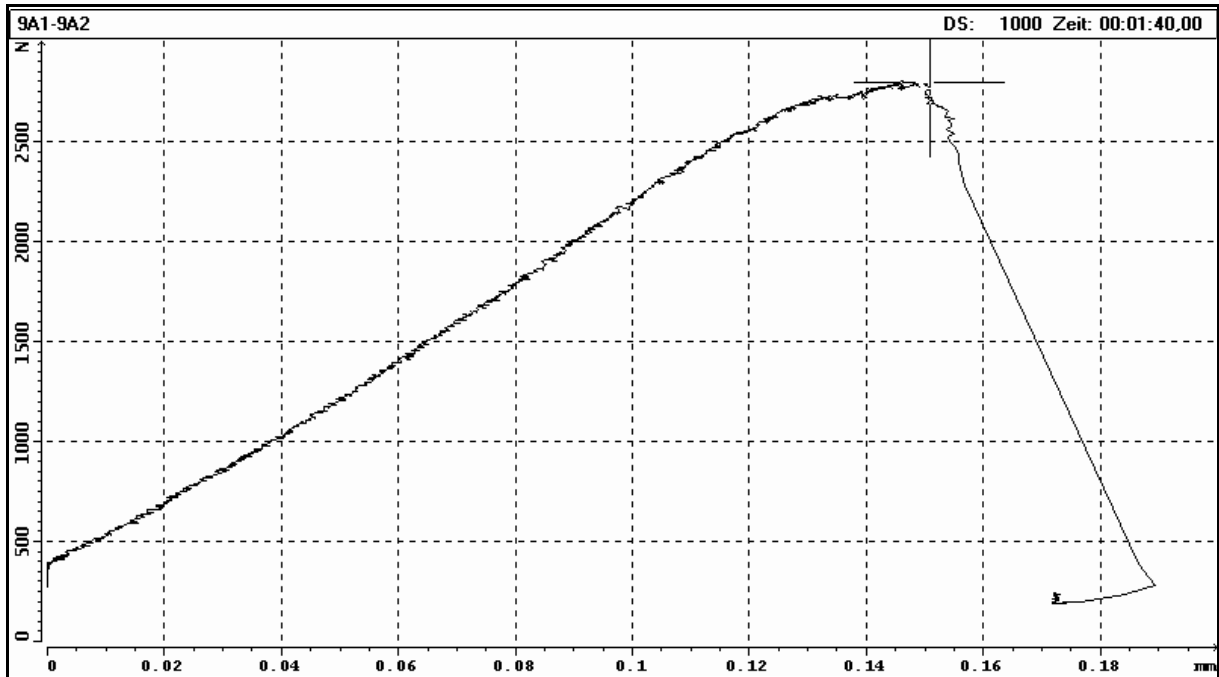
$F_{max}, N$	2416
$K_{IC}, MN \cdot m^{1/2}$	2.37



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	9A1-9A2
mean depth, m:	692.78
diameter, mm:	51.9
initial crack length, mm:	7.0
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



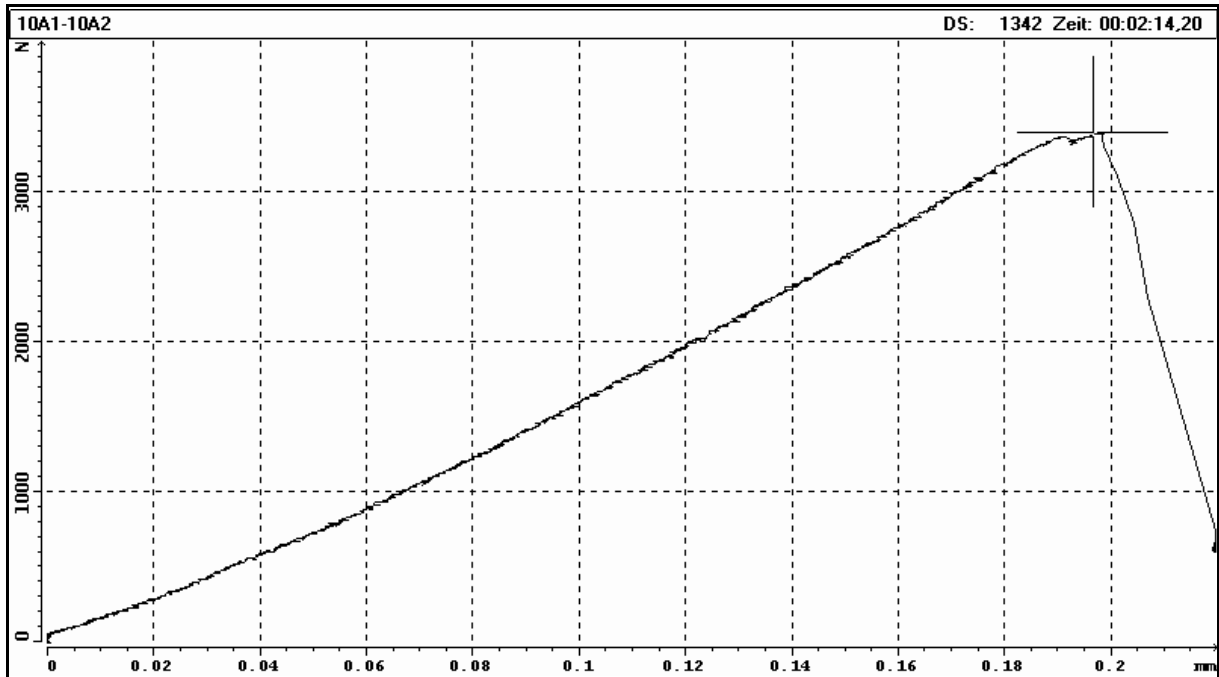
$F_{max}, N$	2789
$K_{IC}, MN \cdot m^{1/2}$	2.34



FRACTURE TOUGHNESS TEST

ISRM standard 1988

borehole:	KOV01
sample - no.:	10A1-10A2
mean depth, m:	709.55
diameter, mm:	51.9
initial crack length, mm:	5.2
date of unpacking / sampling:	08.11.01
sample ready for testing:	09.11.01
date of testing:	09.11.01
operated by	U. Weber
checked by	U. Weber



$F_{max}, N$	3385
$K_{IC}, MN \cdot m^{1/2}$	2.53

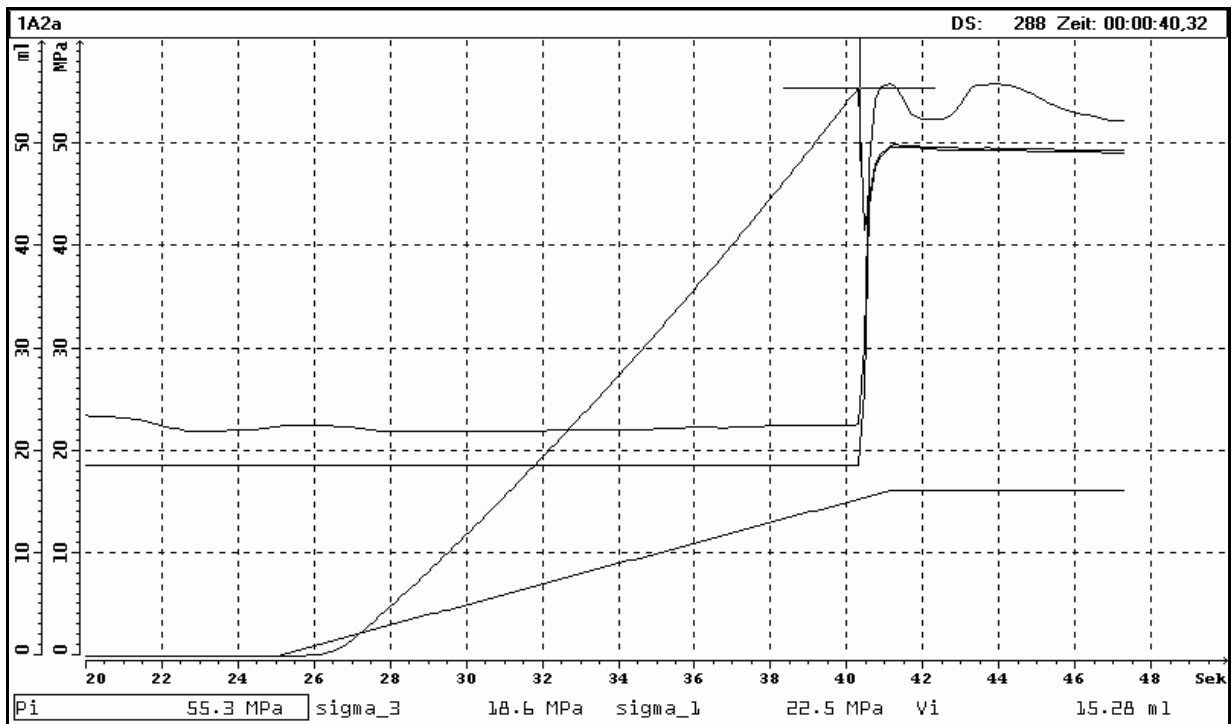


**APPENDIX H**  
**Laboratory hydrofrac data sheets**

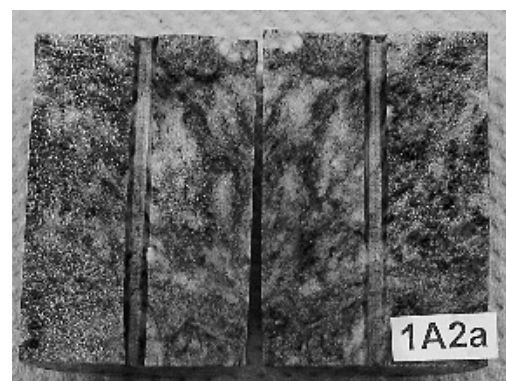


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	1A2a
mean depth, m:	333.79
diameter, mm:	29.9
length, mm:	44.5
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

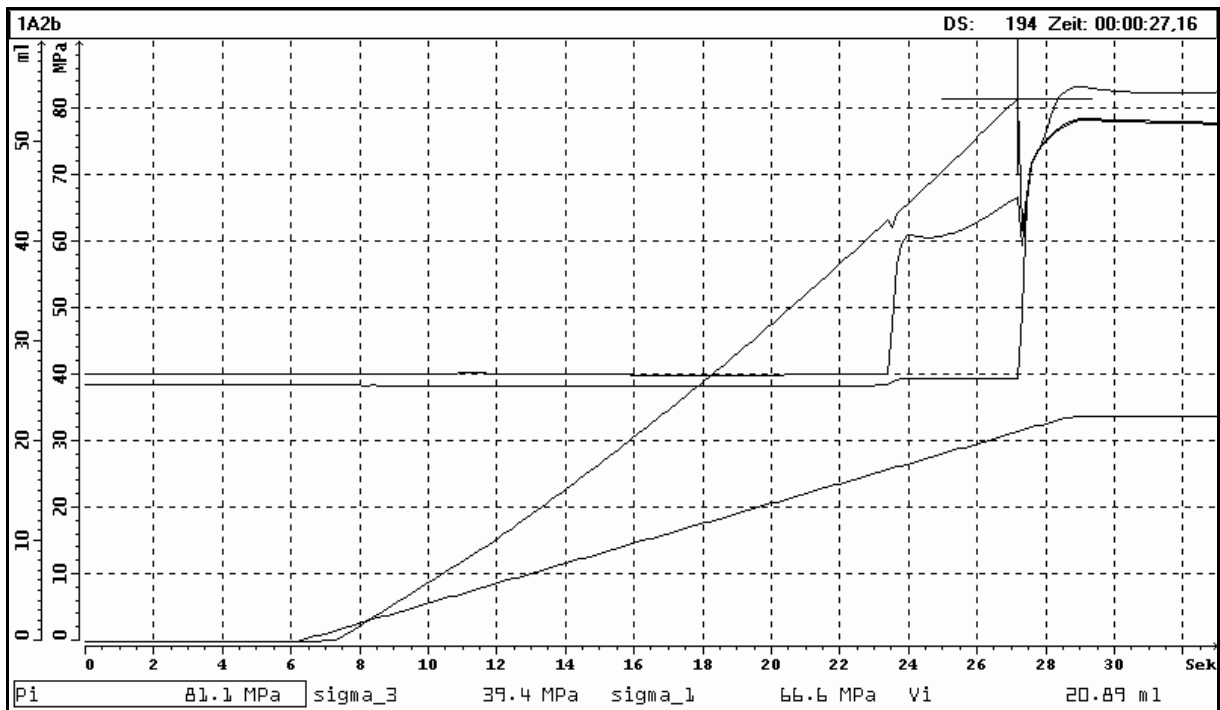


Initial conditions	
$\sigma_1$ , MPa	22.5
$\sigma_3$ , MPa	18.6
Injection Characteristics at failure	
$P_i$ , MPa	55.3
$V_i$ , ml	15.28

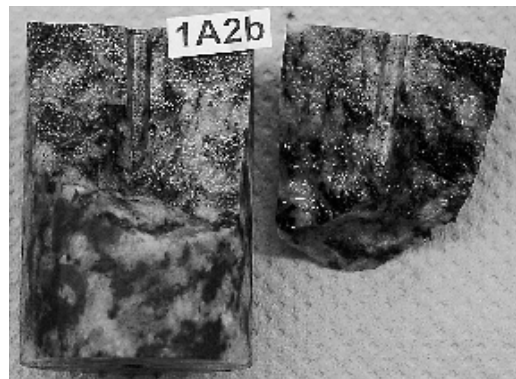


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	1A2b
mean depth, m:	333.79
diameter, mm:	29.9
length, mm:	44.5
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber



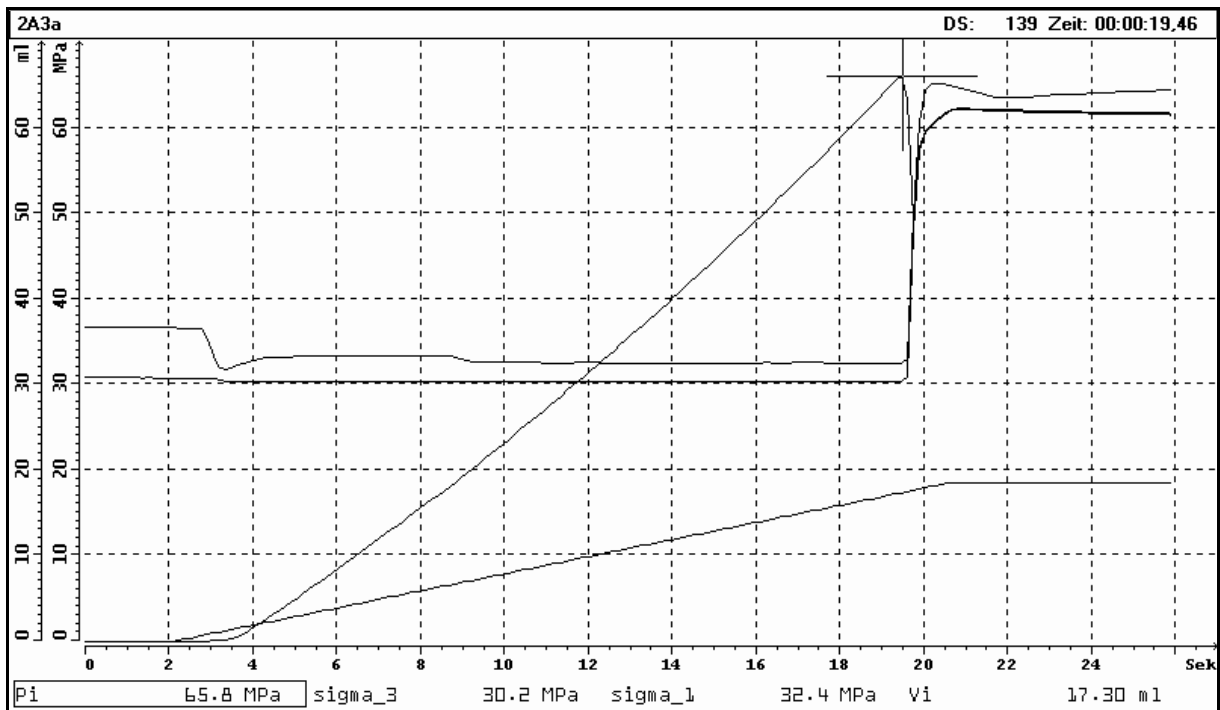
Initial conditions	
$\sigma_1$ , MPa	66.6
$\sigma_3$ , MPa	39.4
Injection Characteristics at failure	
$P_i$ , MPa	81.1
$V_i$ , ml	20.89



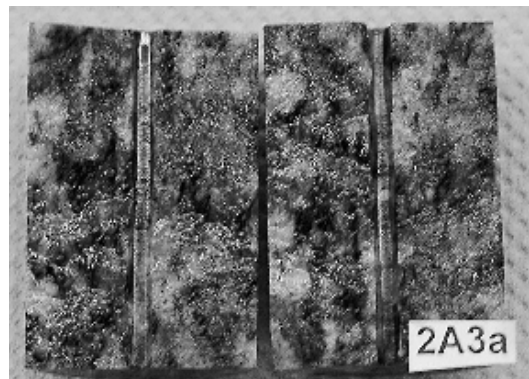


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	2A3a
mean depth, m:	383.16
diameter, mm:	29.9
length, mm:	44.2
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

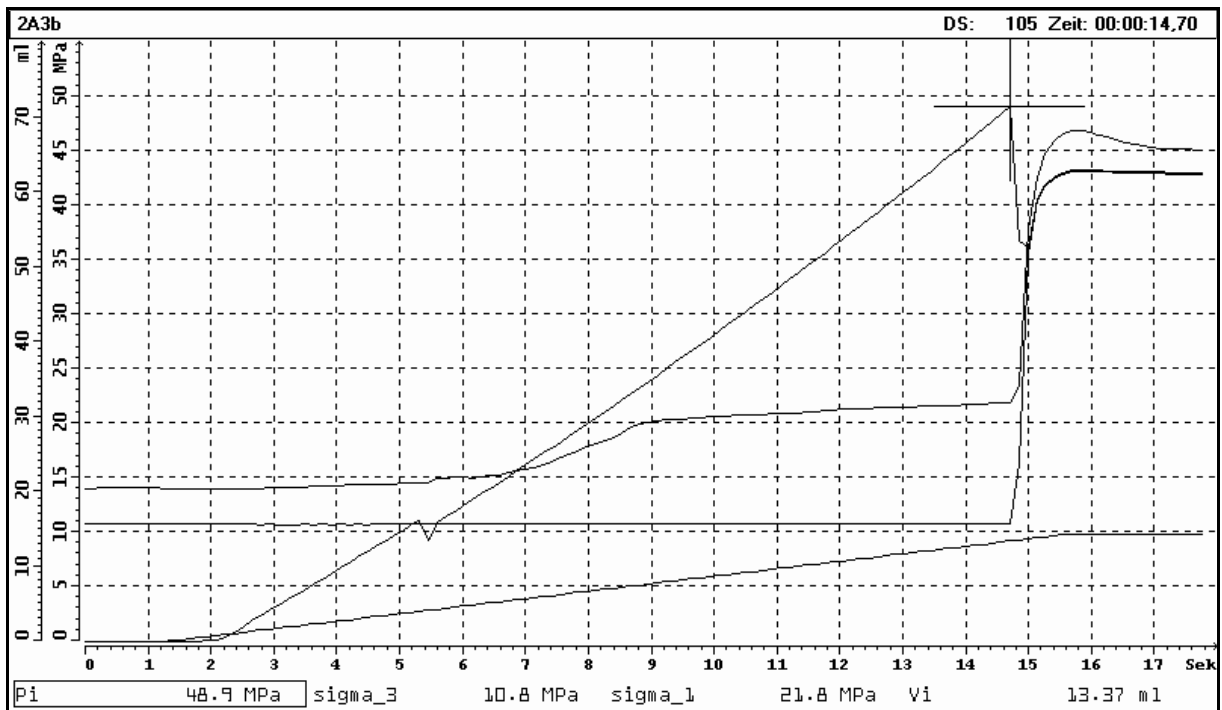


Initial conditions	
$\sigma_1$ , MPa	32.4
$\sigma_3$ , MPa	30.2
Injection Characteristics at failure	
P <sub>i</sub> , MPa	65.8
V <sub>i</sub> , ml	17.30



### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	2A3b
mean depth, m:	383.16
diameter, mm:	29.9
length, mm:	45.0
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

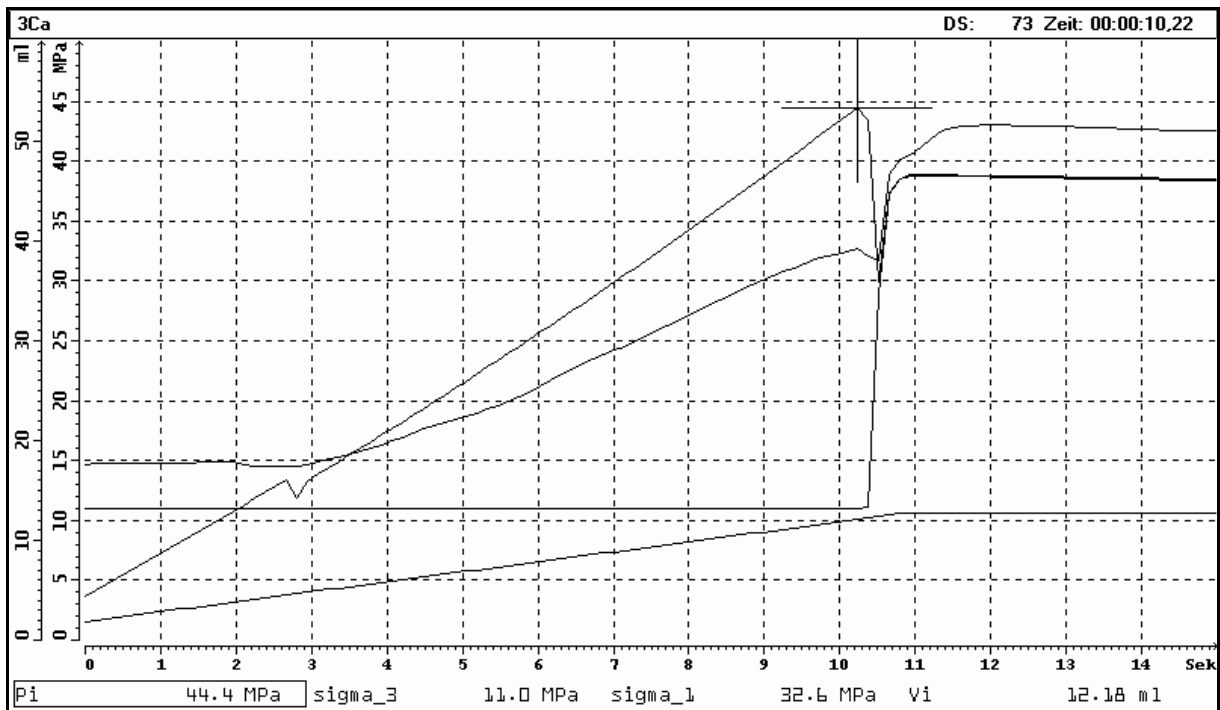


Initial conditions	
$\sigma_1$ , MPa	21.8
$\sigma_3$ , MPa	10.8
Injection Characteristics at failure	
P <sub>i</sub> , MPa	48.9
V <sub>i</sub> , ml	13.37

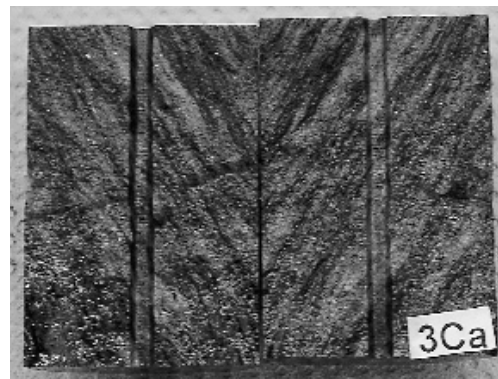


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	3Ca
mean depth, m:	438.33
diameter, mm:	29.9
length, mm:	43.8
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

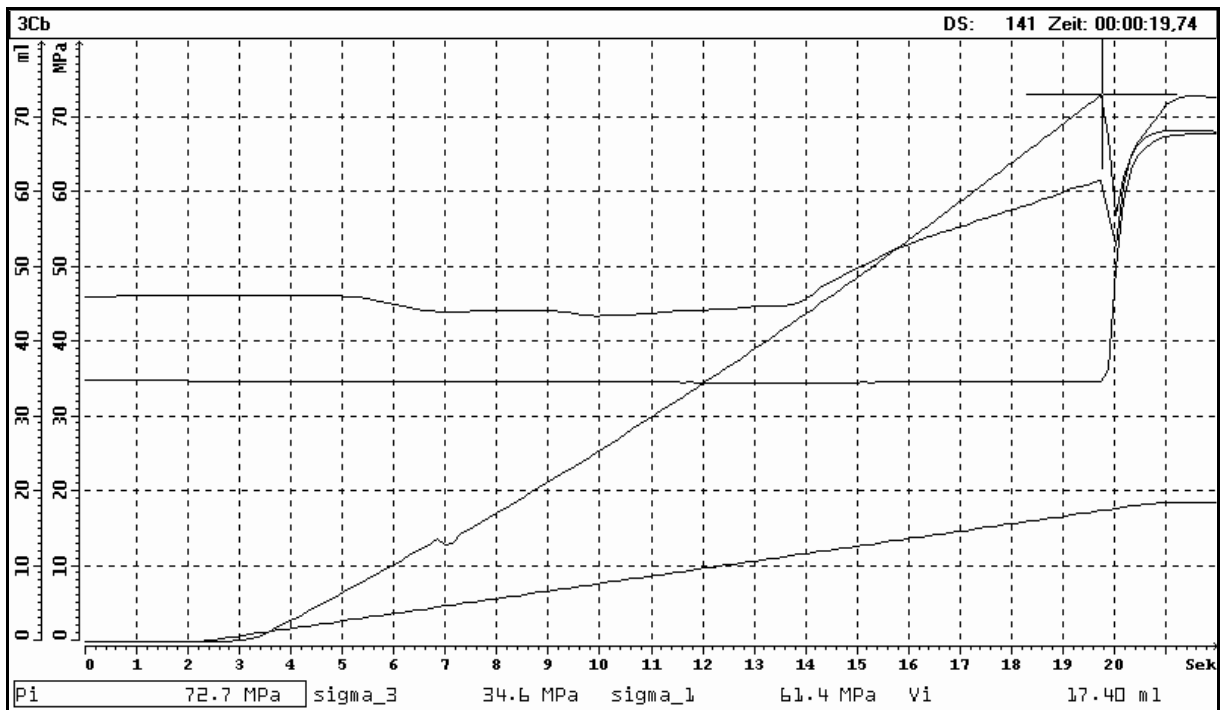


Initial conditions	
$\sigma_1$ , MPa	32.6
$\sigma_3$ , MPa	11.0
Injection Characteristics at failure	
$P_i$ , MPa	44.4
$V_i$ , ml	12.18

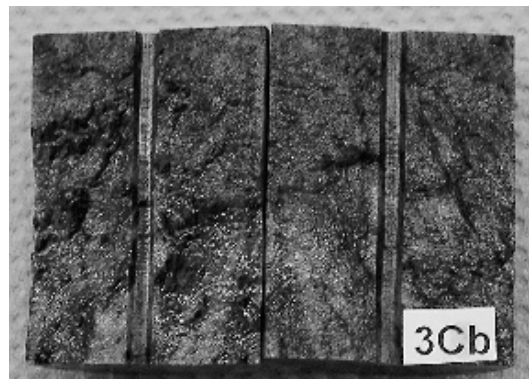


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	3Cb
mean depth, m:	438.33
diameter, mm:	29.9
length, mm:	40.8
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

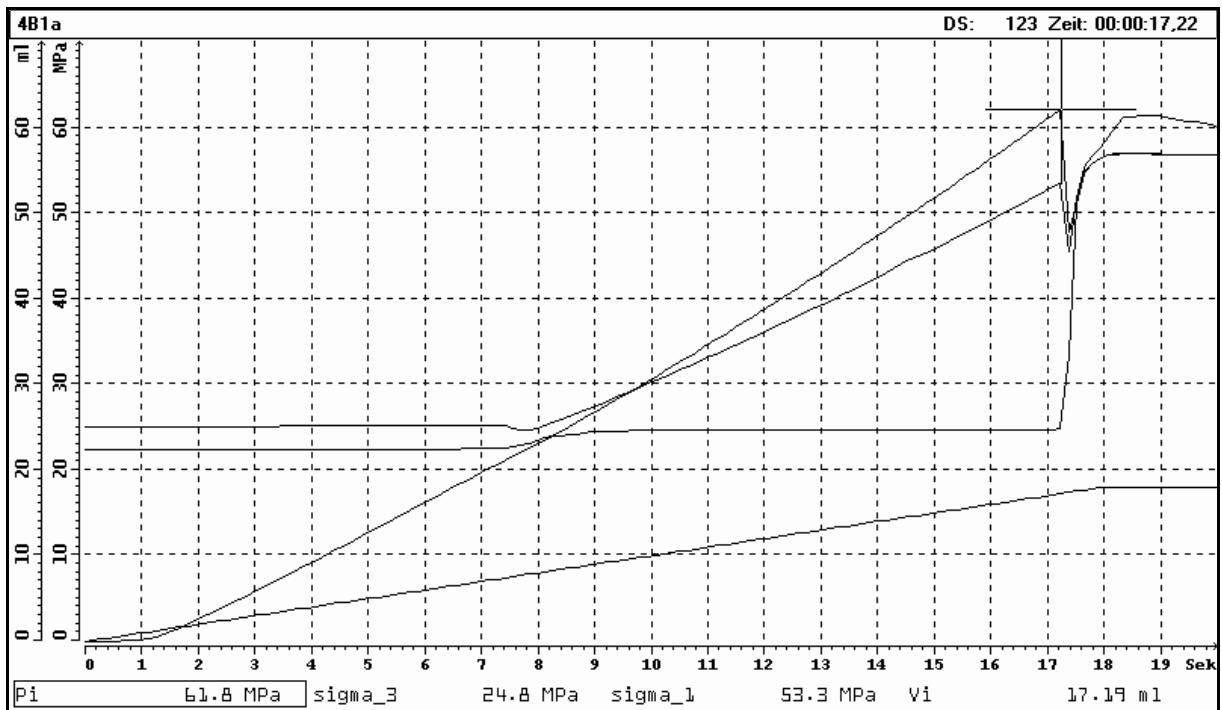


Initial conditions	
$\sigma_1$ , MPa	61.4
$\sigma_3$ , MPa	34.6
Injection Characteristics at failure	
$P_i$ , MPa	72.7
$V_i$ , ml	17.40

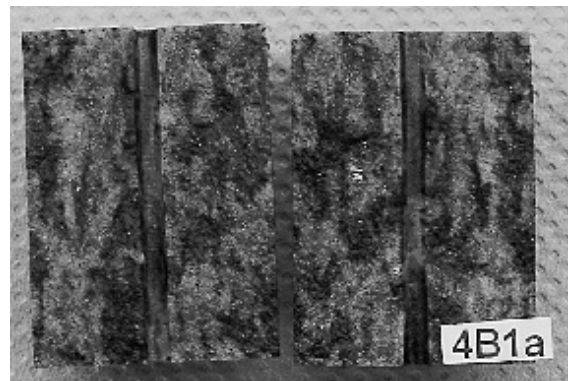


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	4B1a
mean depth, m:	463.85
diameter, mm:	29.9
length, mm:	40.8
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

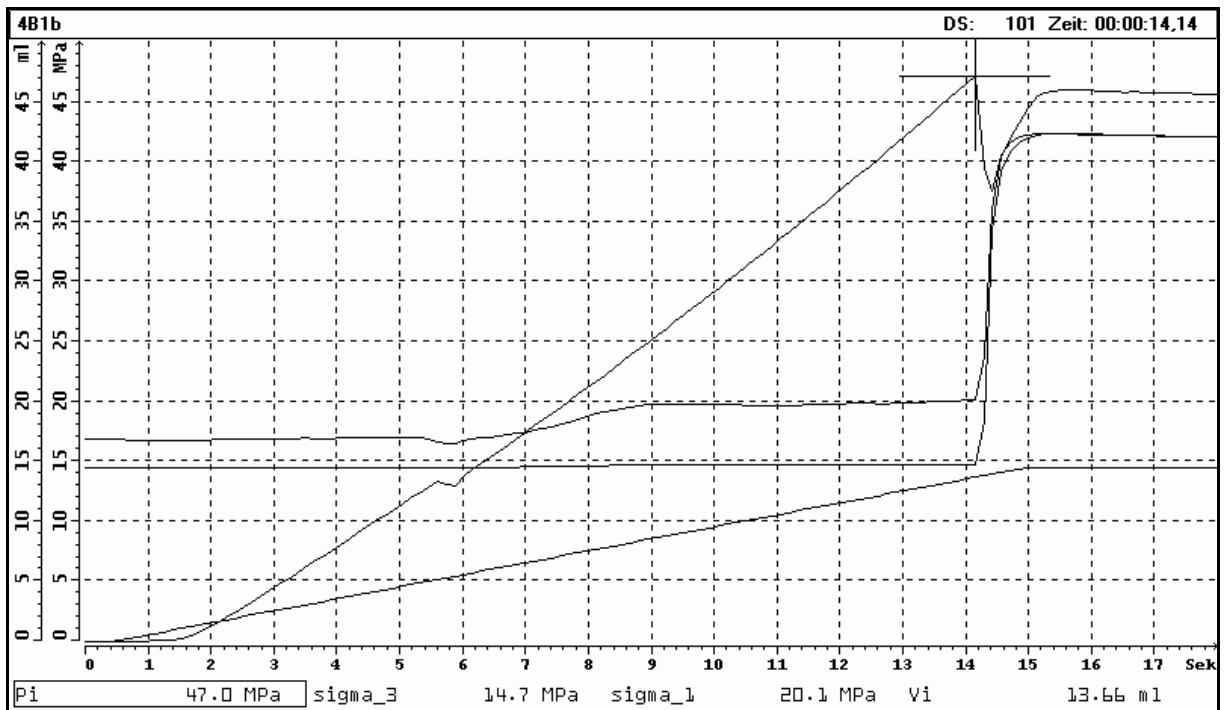


Initial conditions	
$\sigma_1$ , MPa	53.3
$\sigma_3$ , MPa	24.8
Injection Characteristics at failure	
$P_i$ , MPa	61.8
$V_i$ , ml	17.19



### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	4B1b
mean depth, m:	463.85
diameter, mm:	29.9
length, mm:	45.6
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

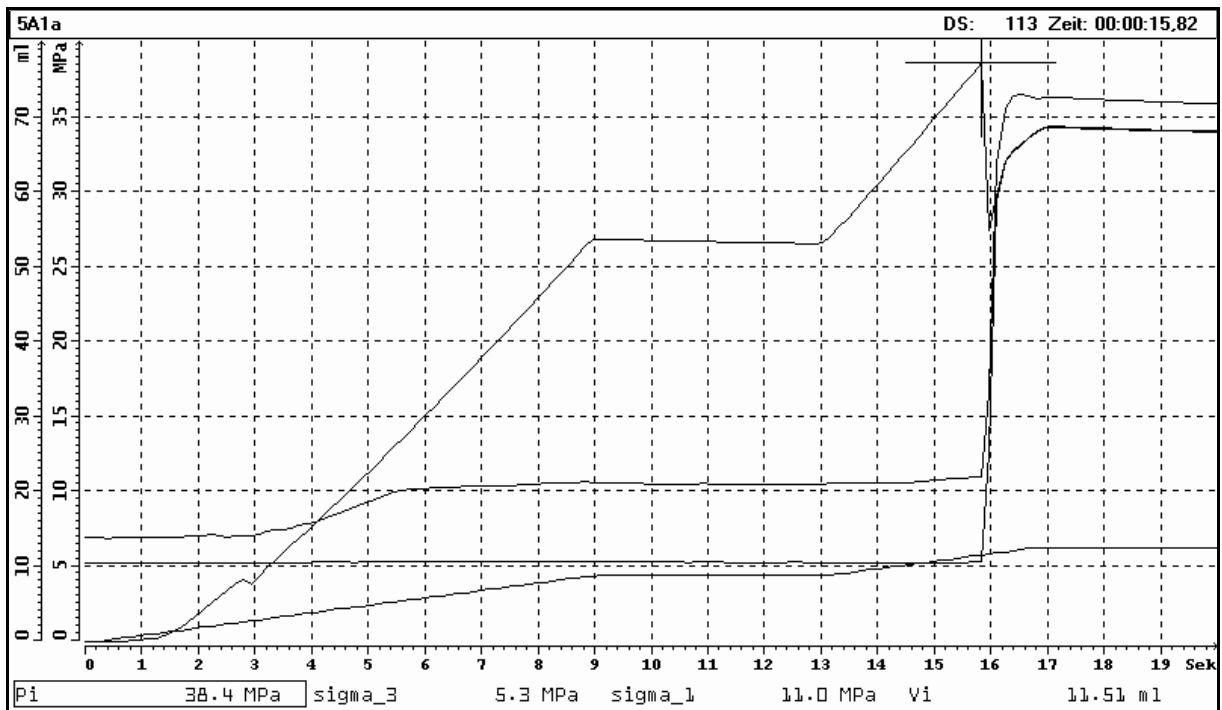


Initial conditions	
$\sigma_1$ , MPa	20.1
$\sigma_3$ , MPa	14.7
Injection Characteristics at failure	
$P_i$ , MPa	47.0
$V_i$ , ml	13.66

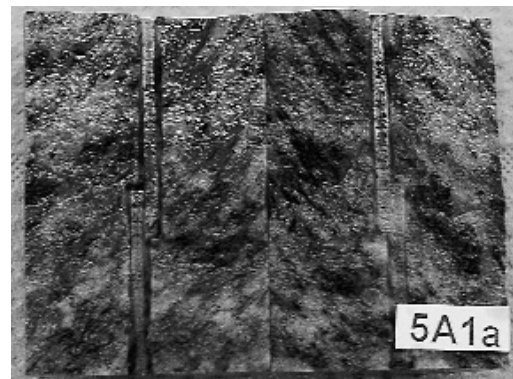


## MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	5A1a
mean depth, m:	500.40
diameter, mm:	29.9
length, mm:	44.5
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

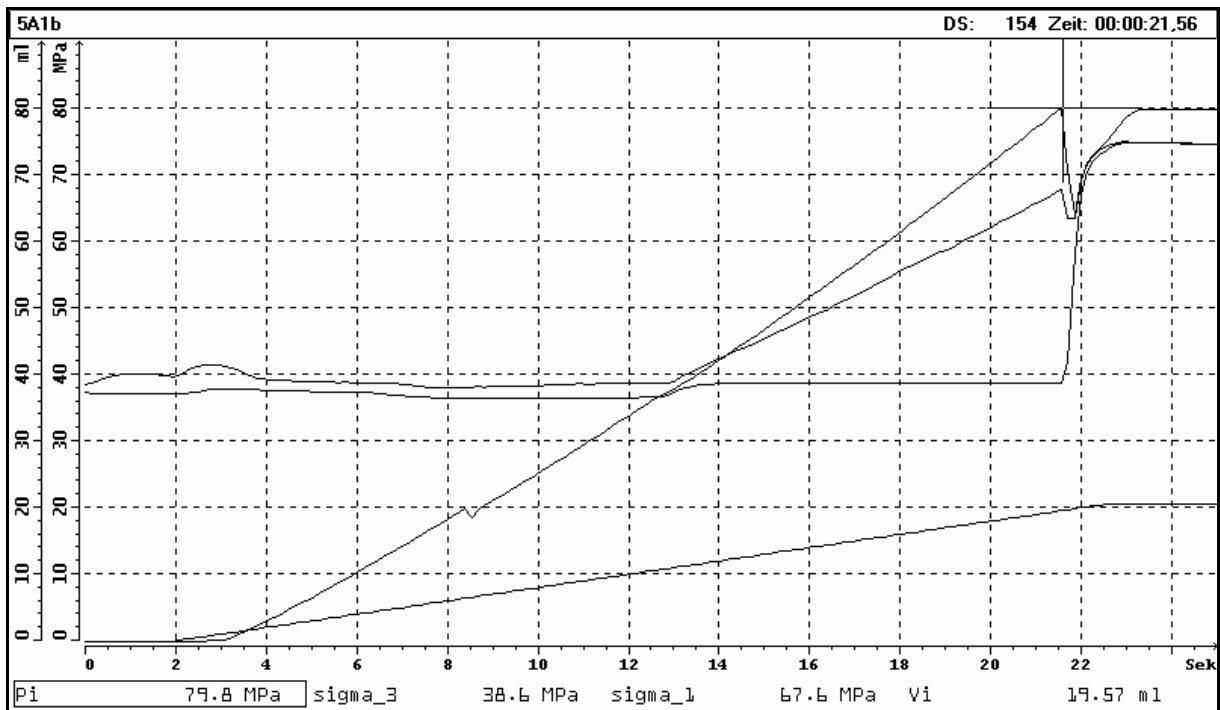


Initial conditions	
$\sigma_1$ , MPa	11.0
$\sigma_3$ , MPa	5.3
Injection Characteristics at failure	
$P_i$ , MPa	38.4
$V_i$ , ml	11.51

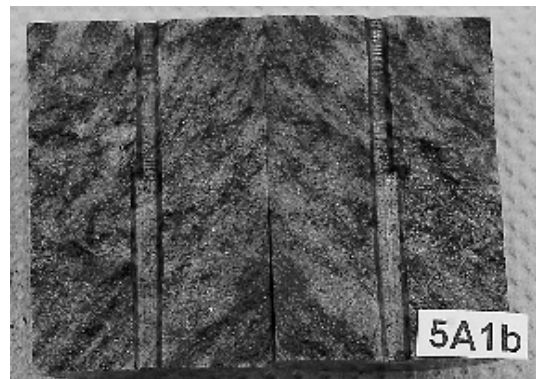


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	5A1b
mean depth, m:	500.40
diameter, mm:	29.9
length, mm:	44.1
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber



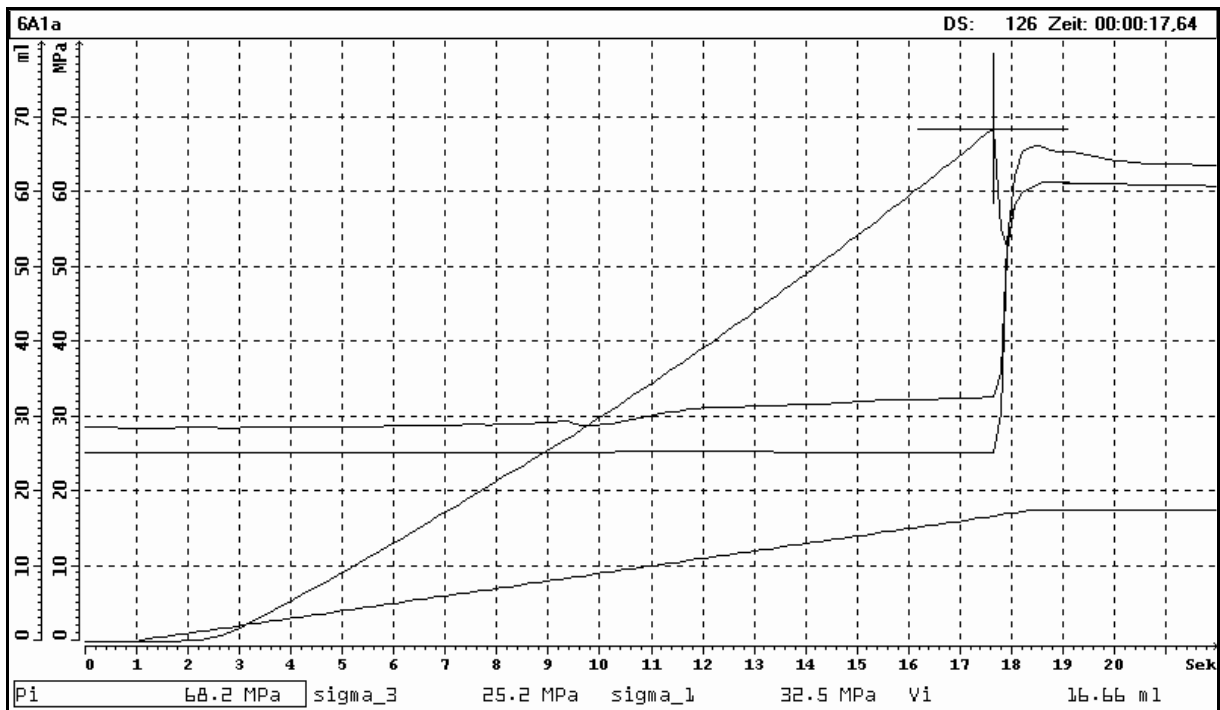
Initial conditions	
$\sigma_1$ , MPa	67.6
$\sigma_3$ , MPa	38.6
Injection Characteristics at failure	
P <sub>i</sub> , MPa	79.8
V <sub>i</sub> , ml	19.57



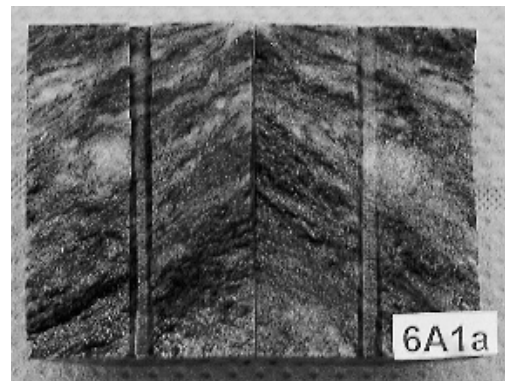


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	6A1a
mean depth, m:	559.90
diameter, mm:	29.9
length, mm:	43.9
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

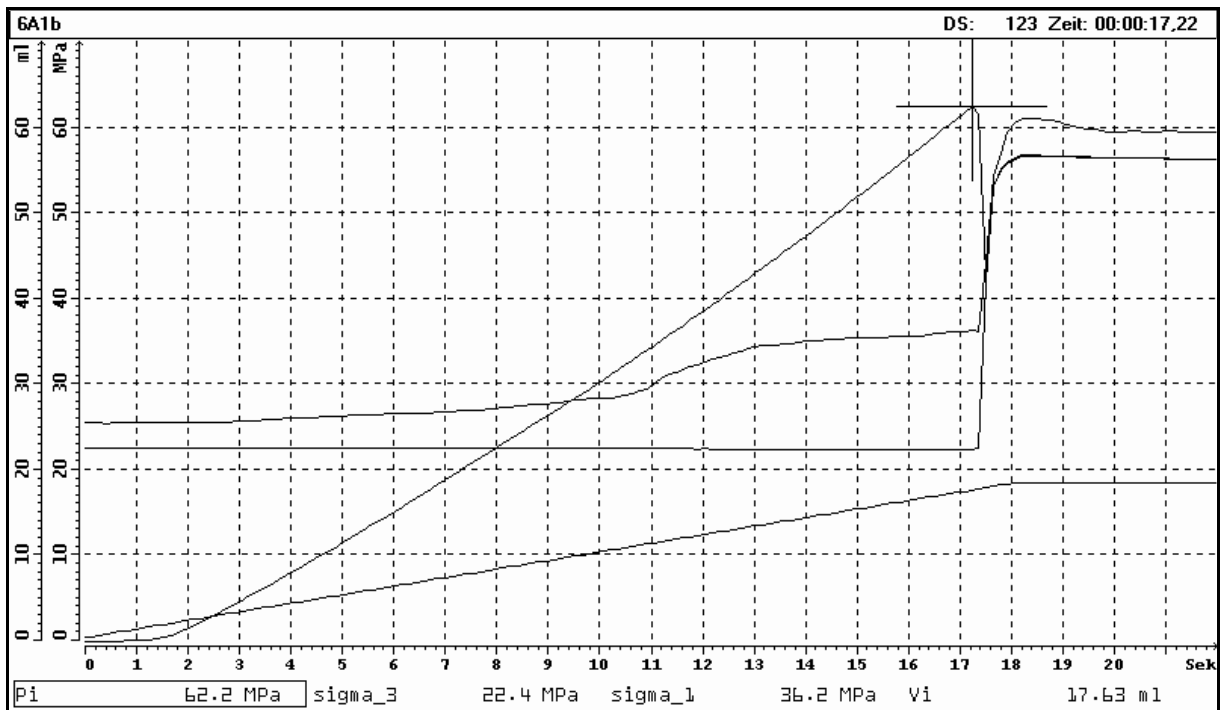


Initial conditions	
$\sigma_1$ , MPa	32.5
$\sigma_3$ , MPa	25.2
Injection Characteristics at failure	
$P_i$ , MPa	68.2
$V_i$ , ml	16.66

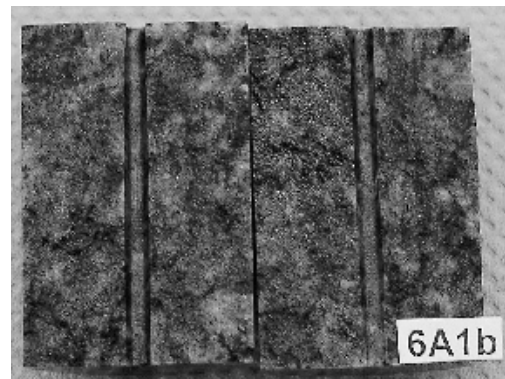


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	6A1b
mean depth, m:	559.9
diameter, mm:	29.9
length, mm:	45.0
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

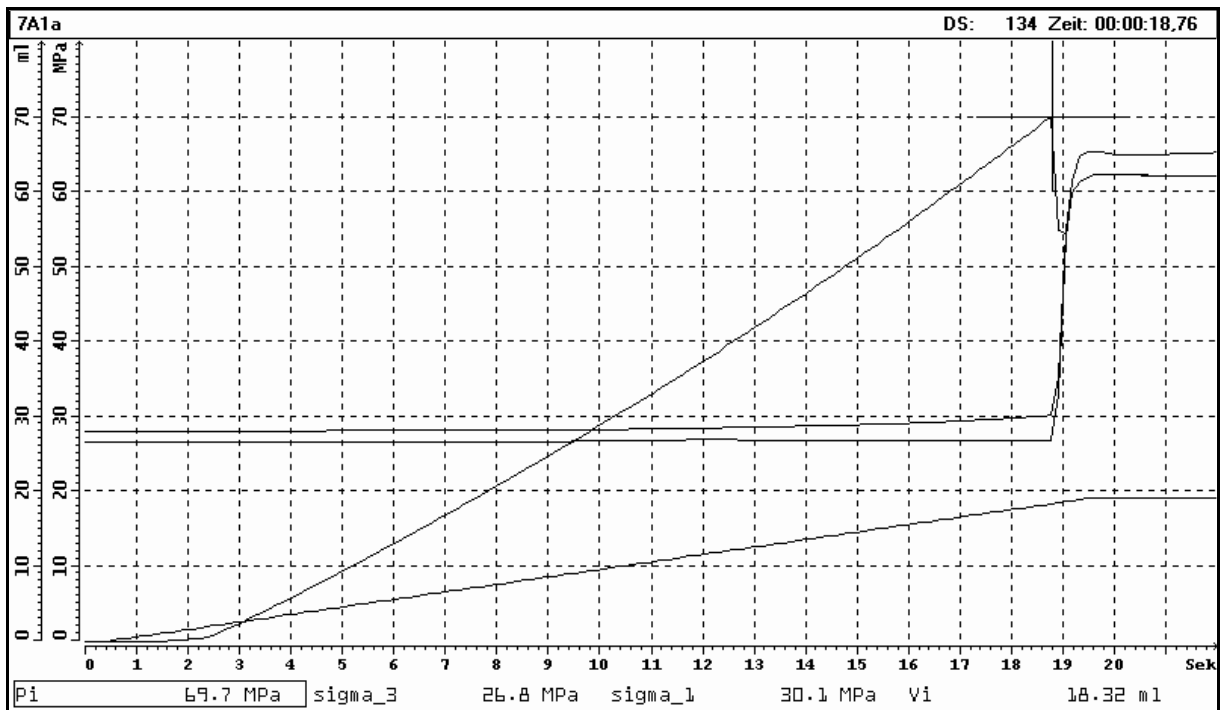


Initial conditions	
$\sigma_1$ , MPa	36.2
$\sigma_3$ , MPa	22.4
Injection Characteristics at failure	
$P_i$ , MPa	62.2
$V_i$ , ml	17.63

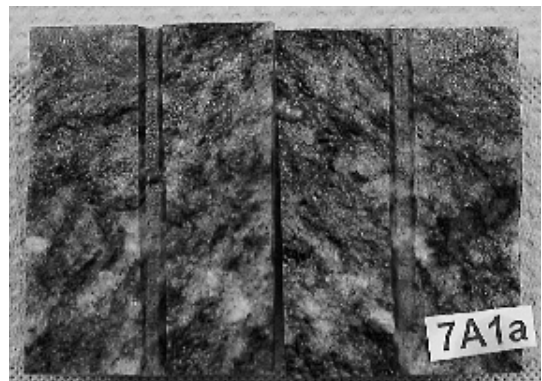


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	7A1a
mean depth, m:	596.65
diameter, mm:	29.9
length, mm:	42.4
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

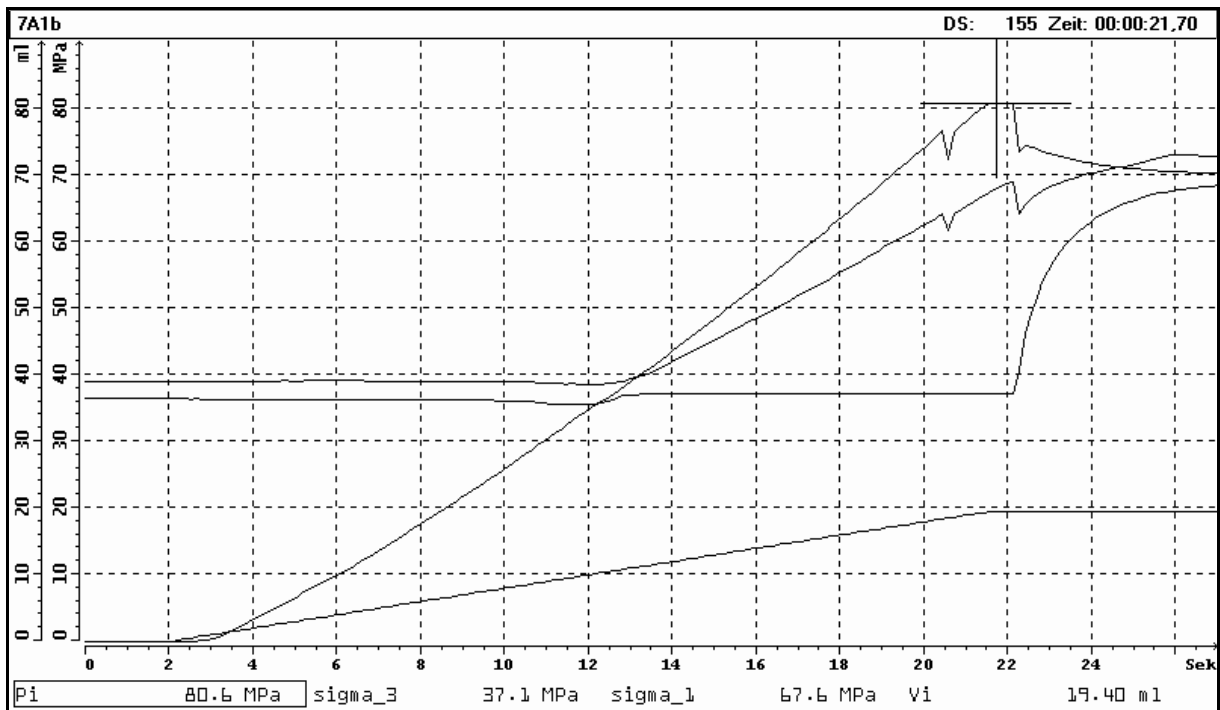


Initial conditions	
$\sigma_1$ , MPa	30.1
$\sigma_3$ , MPa	26.8
Injection Characteristics at failure	
$P_i$ , MPa	69.7
$V_i$ , ml	18.32



### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	7A1b
mean depth, m:	596.65
diameter, mm:	29.9
length, mm:	45.3
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

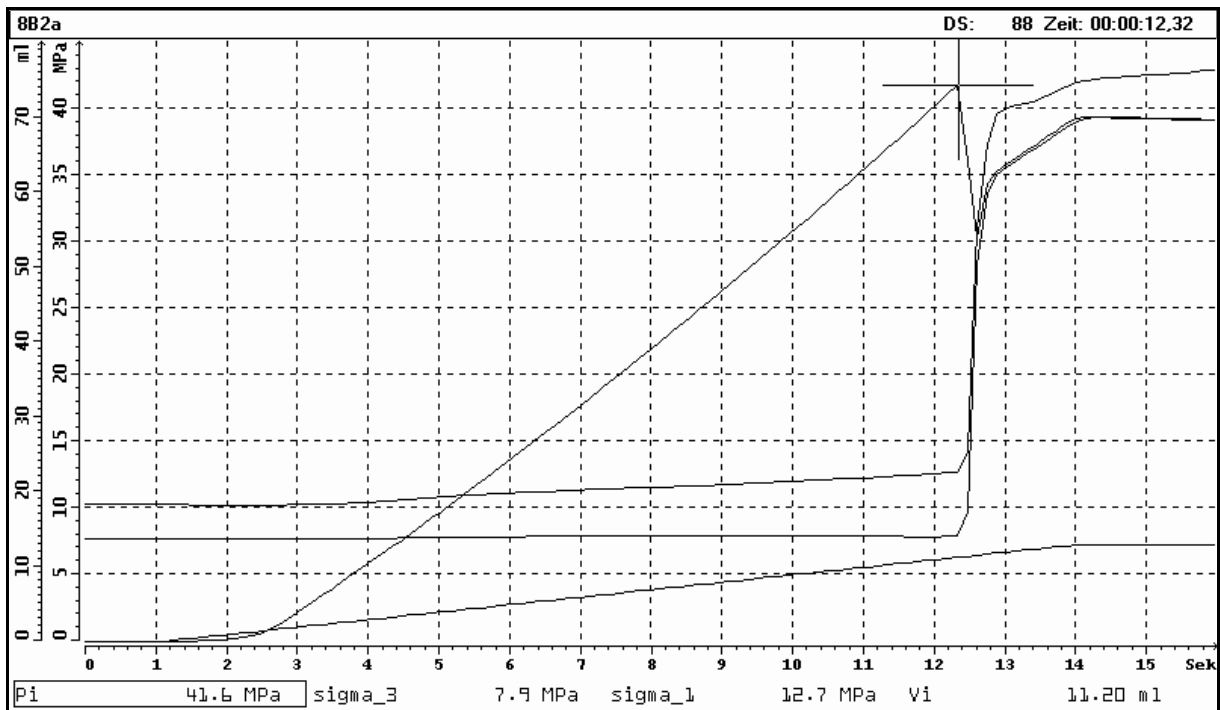


Initial conditions	
$\sigma_1$ , MPa	67.6
$\sigma_3$ , MPa	37.1
Injection Characteristics at failure	
P <sub>i</sub> , MPa	80.6
V <sub>i</sub> , ml	19.40

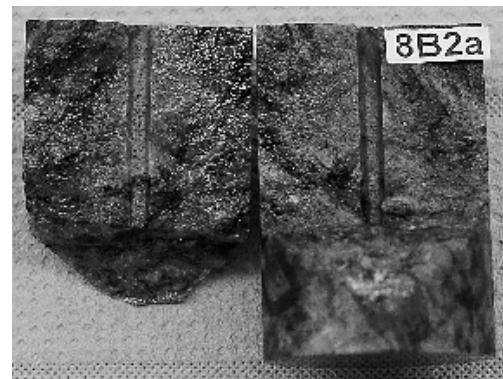


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	8B2a
mean depth, m:	648.89
diameter, mm:	29.9
length, mm:	44.1
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

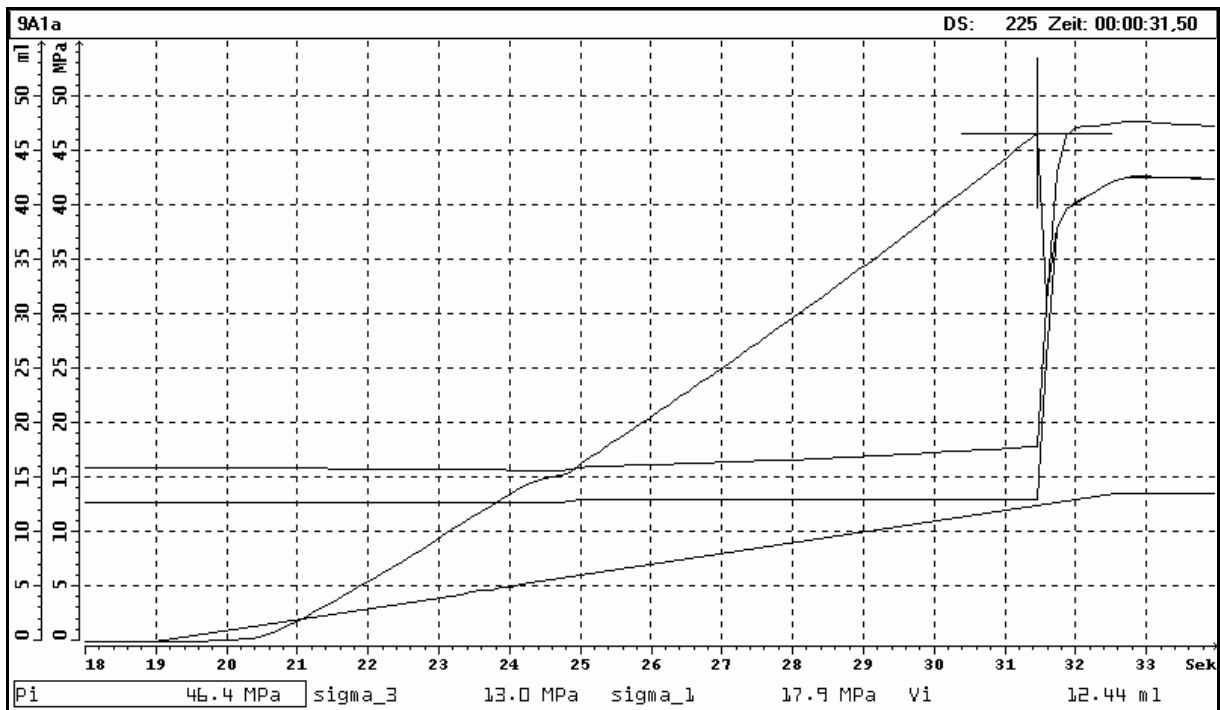


Initial conditions	
$\sigma_1$ , MPa	12.7
$\sigma_3$ , MPa	7.9
Injection Characteristics at failure	
Pi, MPa	41.6
Vi, ml	11.20

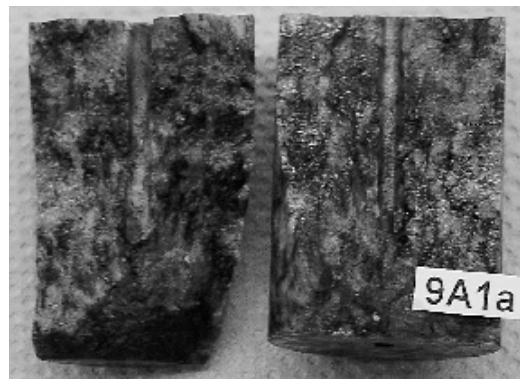


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	9A1a
mean depth, m:	692.78
diameter, mm:	29.9
length, mm:	44.0
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

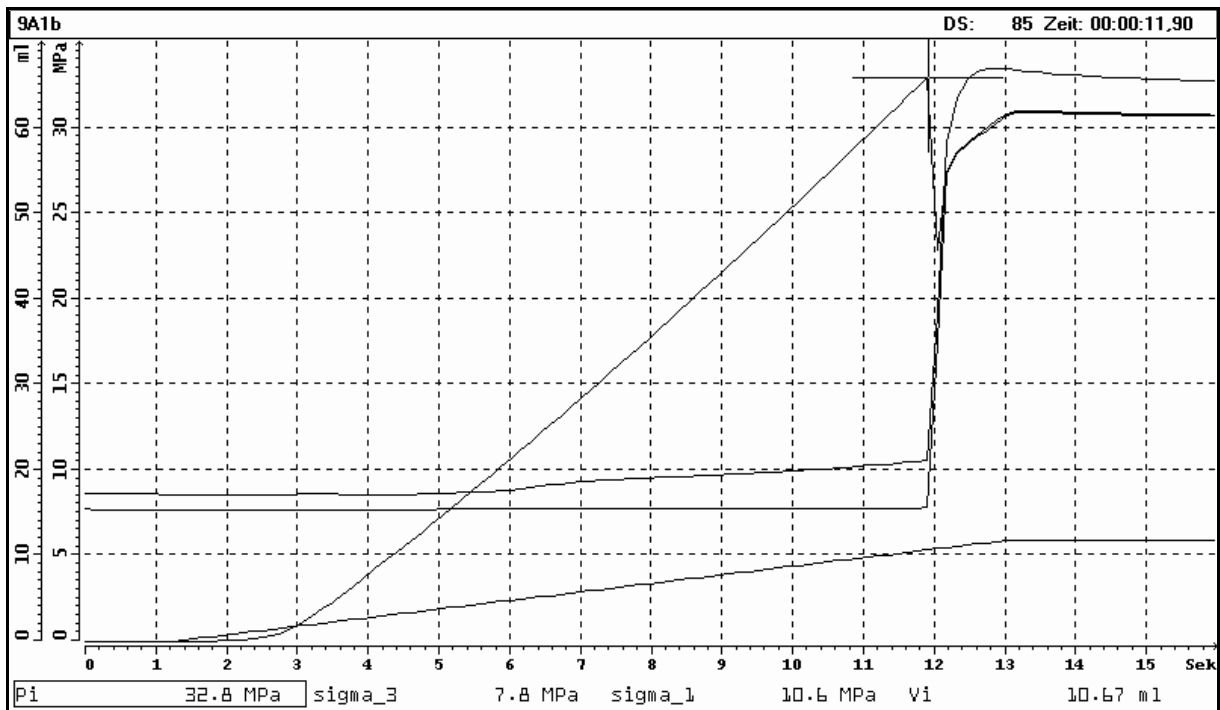


Initial conditions	
$\sigma_1$ , MPa	17.9
$\sigma_3$ , MPa	13.0
Injection Characteristics at failure	
$P_i$ , MPa	46.4
$V_i$ , ml	12.44

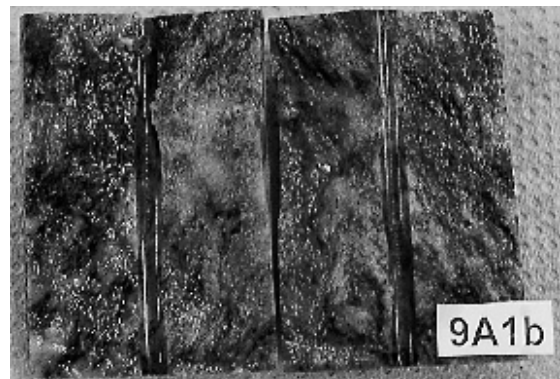


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	9A1b
mean depth, m:	692.78
diameter, mm:	29.9
length, mm:	43.9
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber

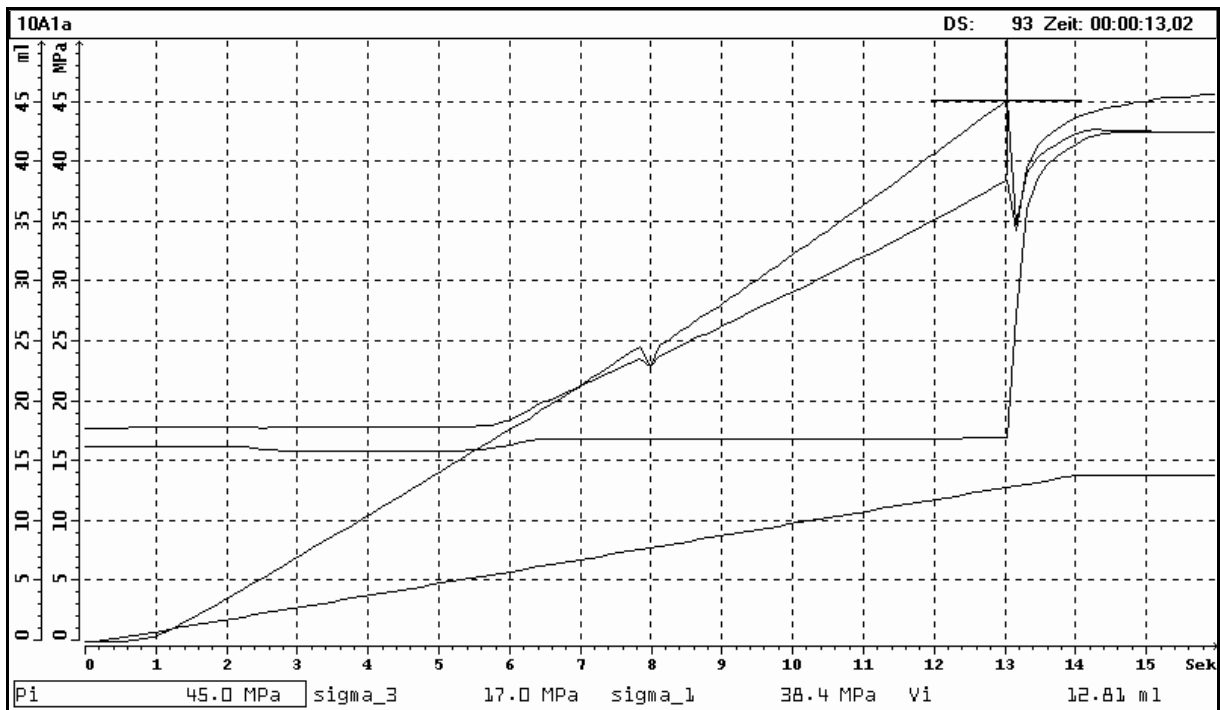


Initial conditions	
$\sigma_1$ , MPa	10.6
$\sigma_3$ , MPa	7.8
Injection Characteristics at failure	
$P_i$ , MPa	32.8
$V_i$ , ml	10.67

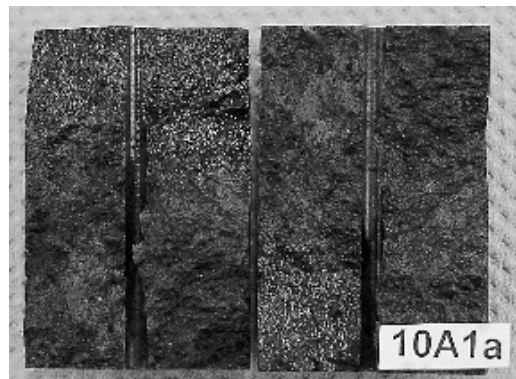


### MINI-FRAC TEST DATA SHEET

borehole:	KOV01
sample - no.:	10A1a
mean depth, m:	709.55
diameter, mm:	29.9
length, mm:	45.0
date of unpacking / sampling:	12.11.01
mini - sample ready for testing:	14.11.01
date of testing:	16.11.01
operated by	J. Orzol
checked by	U. Weber



Initial conditions	
$\sigma_1$ , MPa	38.4
$\sigma_3$ , MPa	17.0
Injection Characteristics at failure	
$P_i$ , MPa	45.0
$V_i$ , ml	12.81



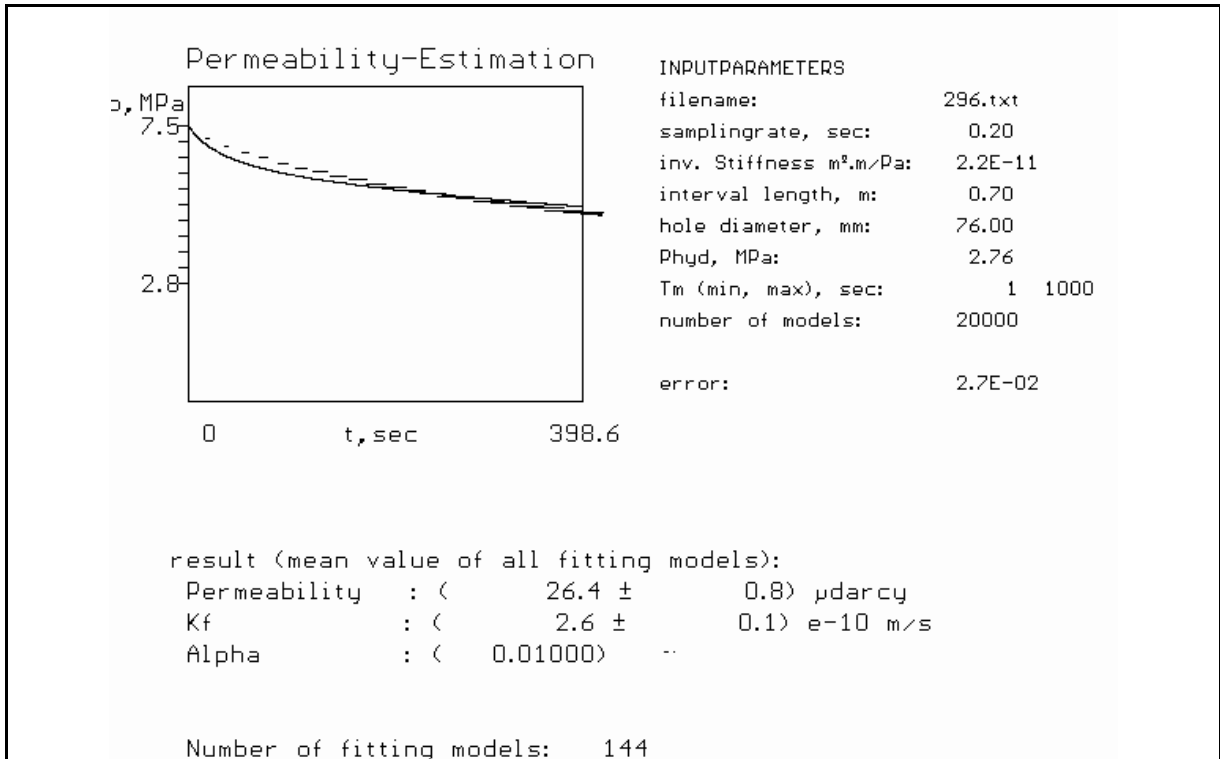


## **APPENDIX I**

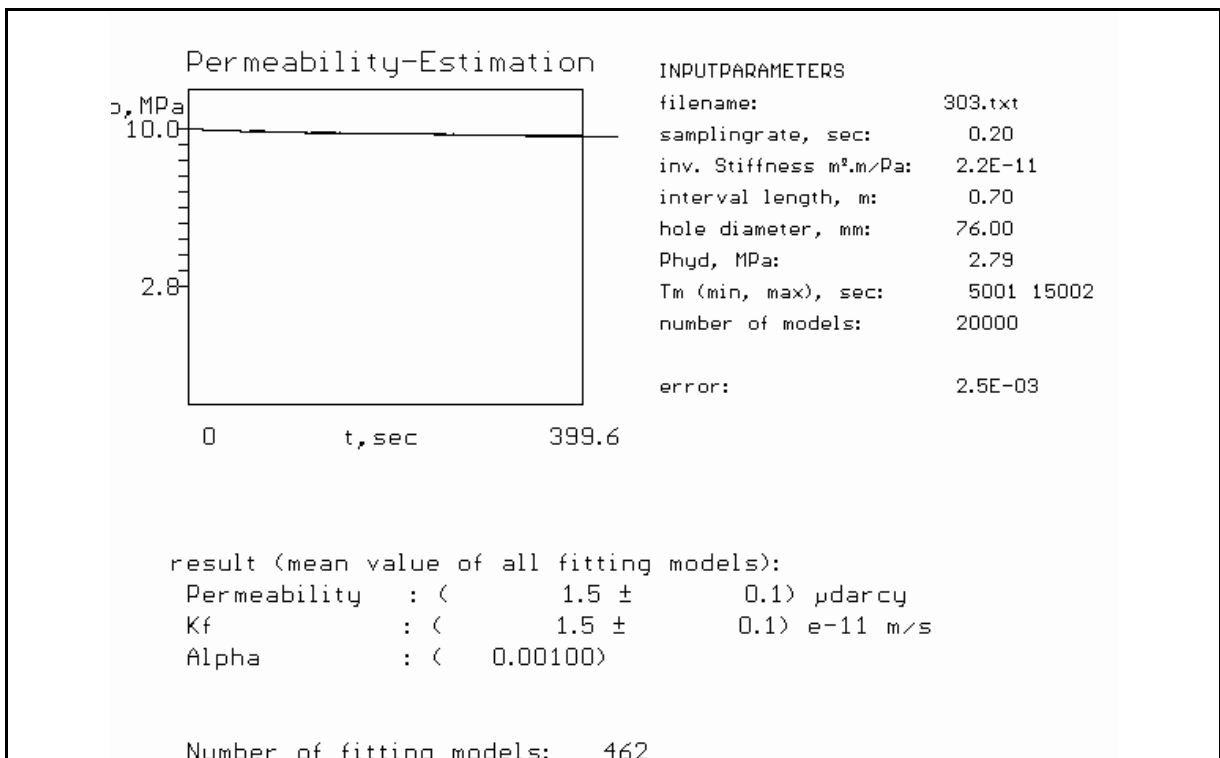
### **Pressure Pulse Test Records and Permeability Analysis**



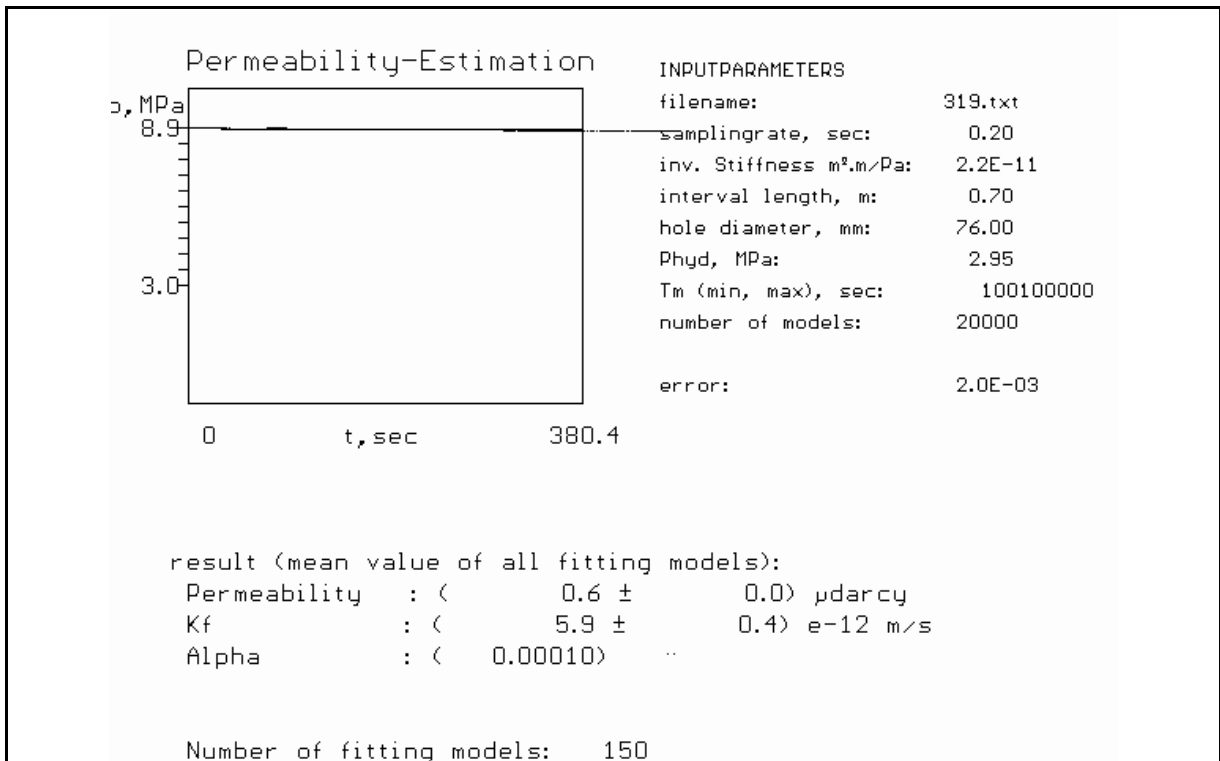
## TEST AT 296.0 m MD / 280.9 m TVD



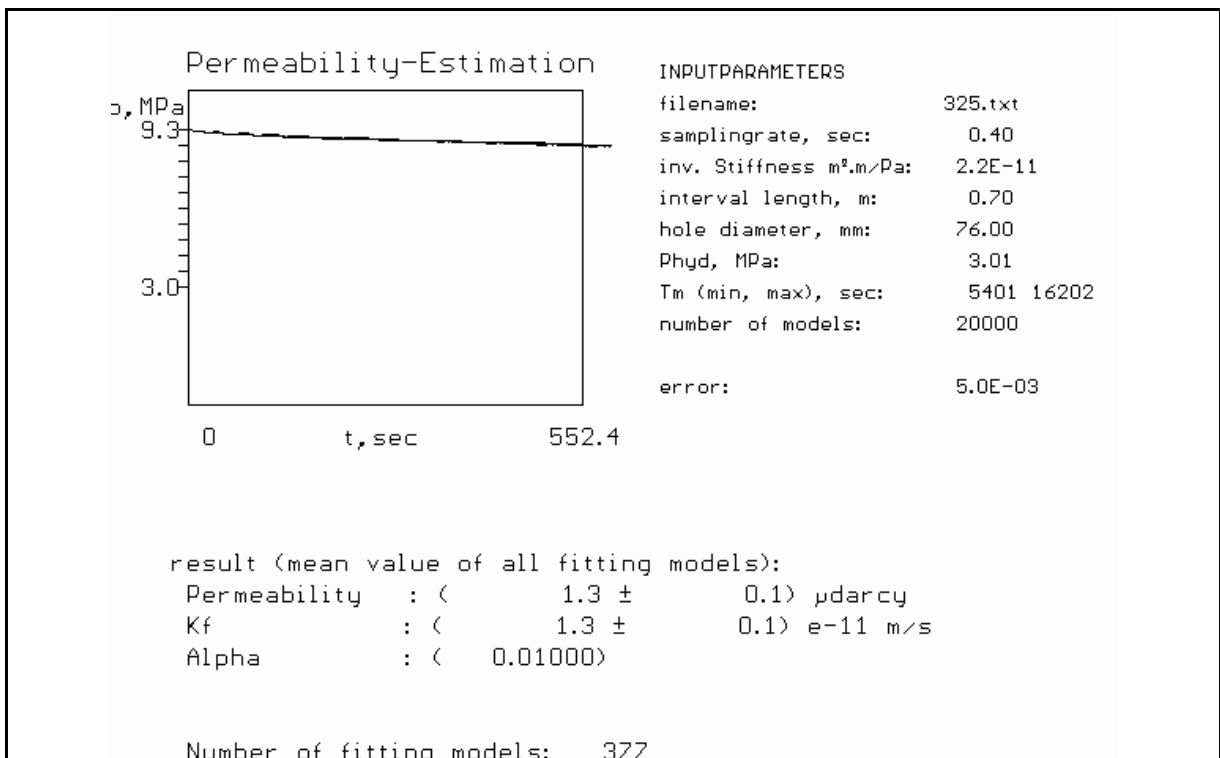
## TEST AT 303.5 m MD / 288.0 m TVD



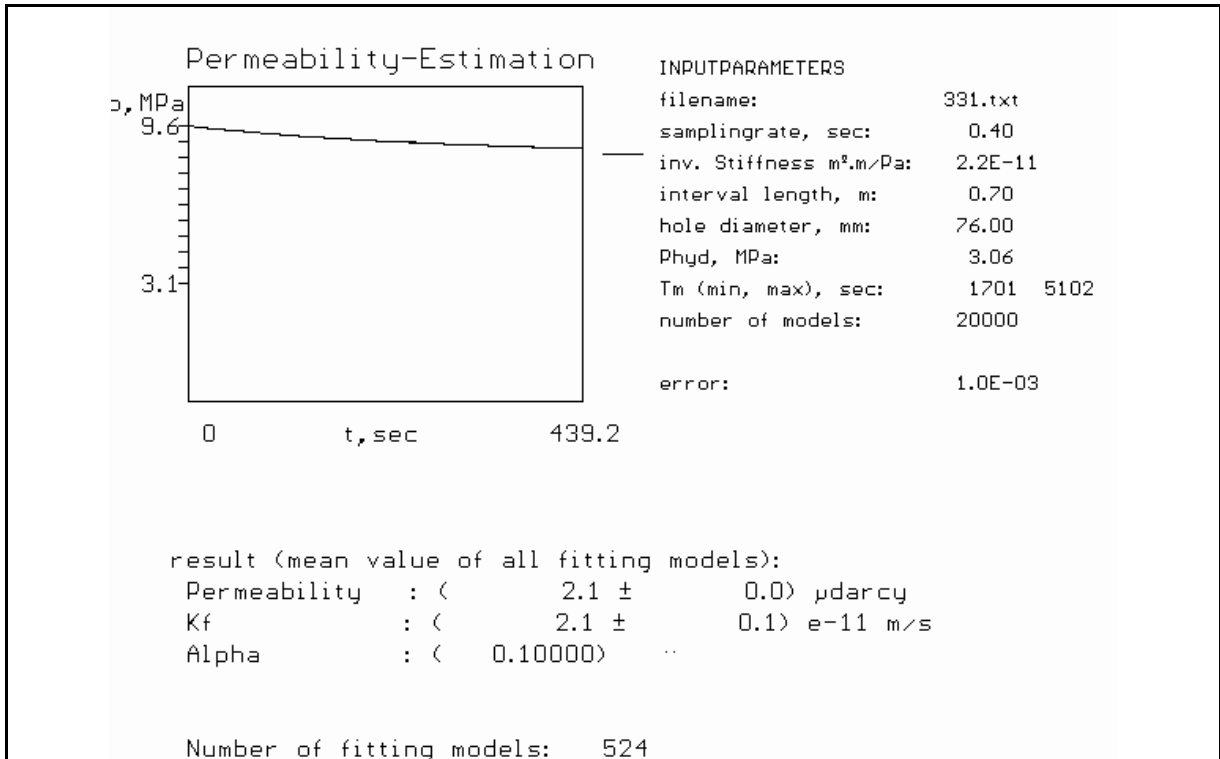
## TEST AT 319.3 m MD / 302.8 m TVD



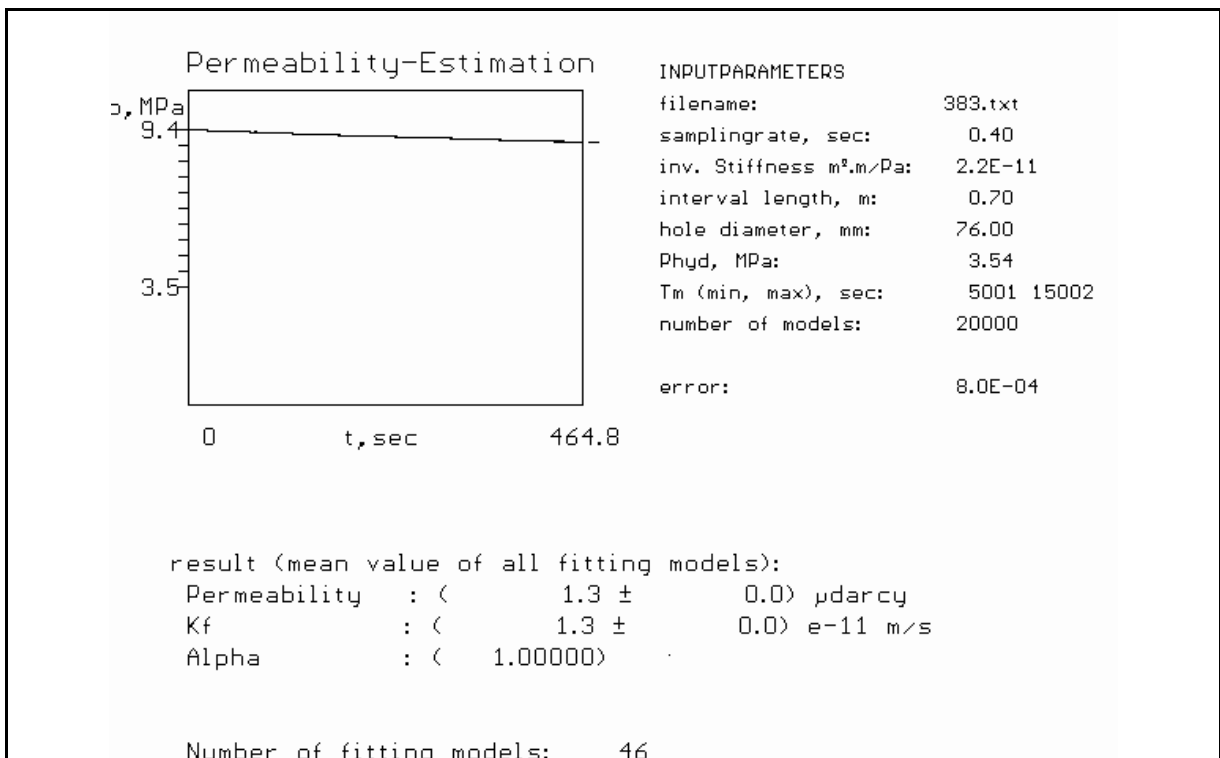
## TEST AT 325.4 m MD / 308.5 m TVD



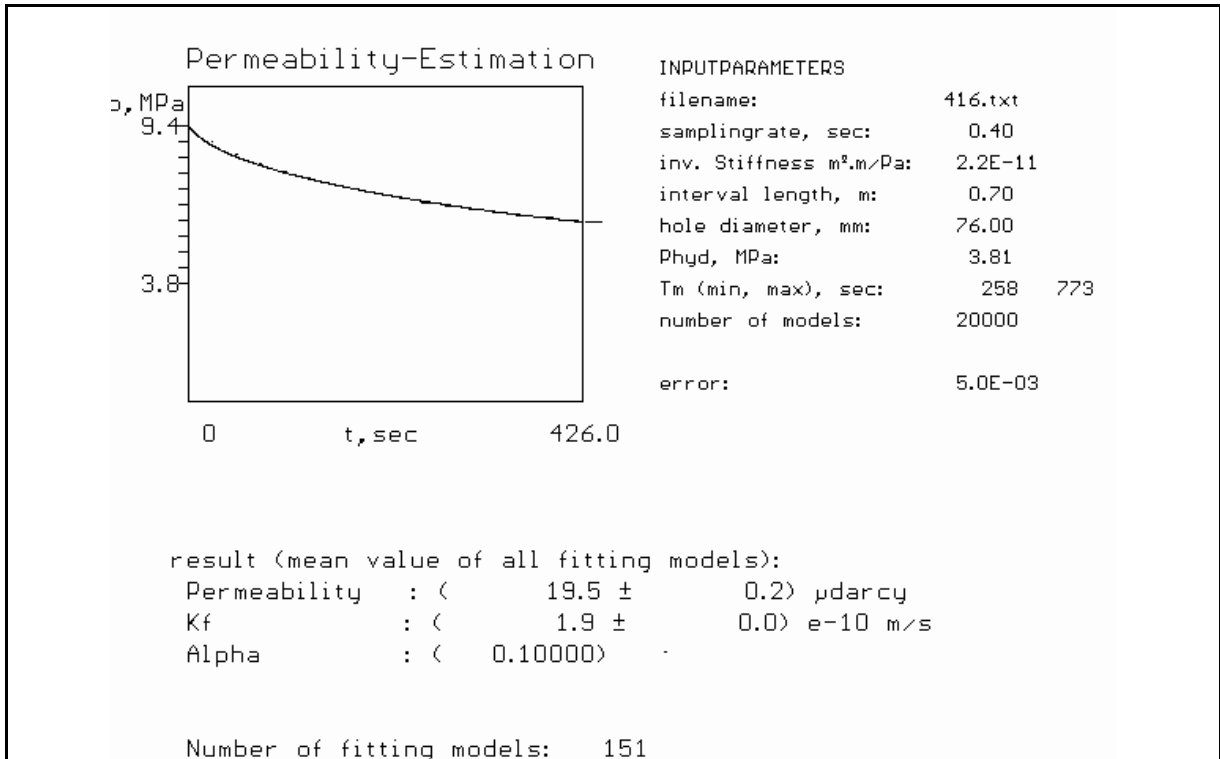
## TEST AT 331.5 m MD / 314.2 m TVD



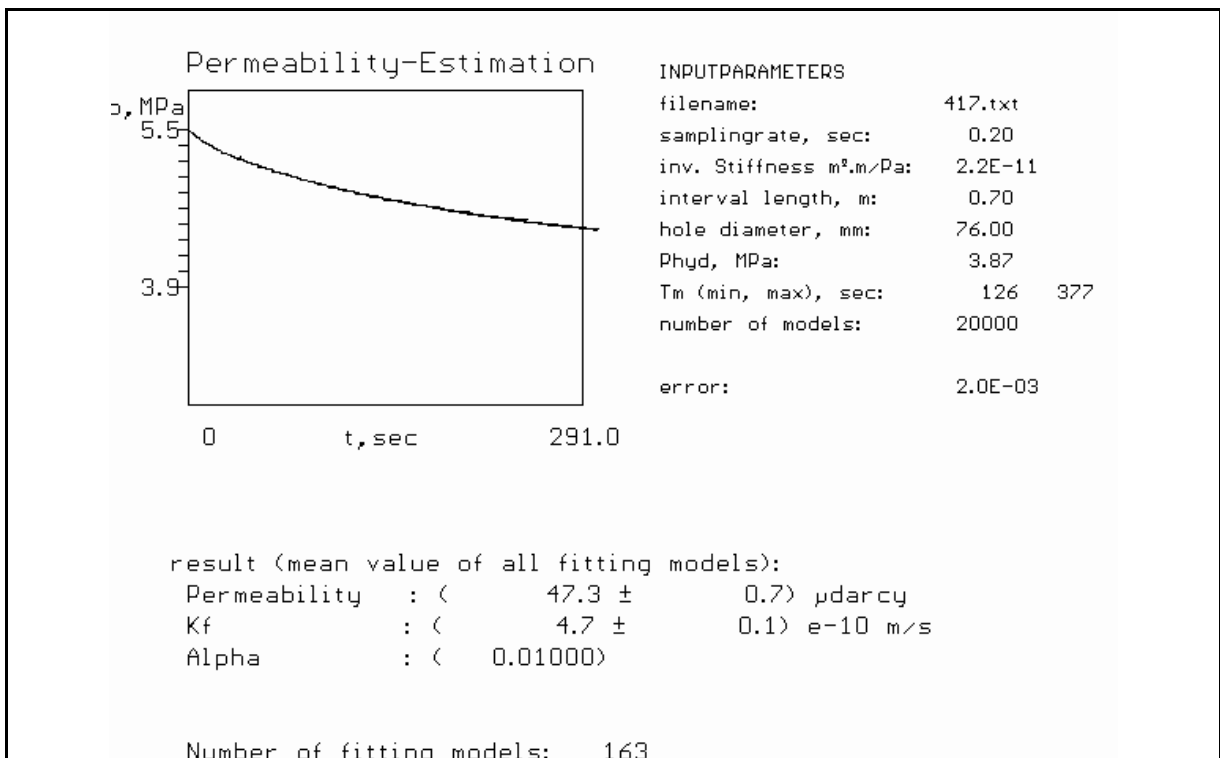
## TEST AT 383.5 m MD / 363.0 m TVD



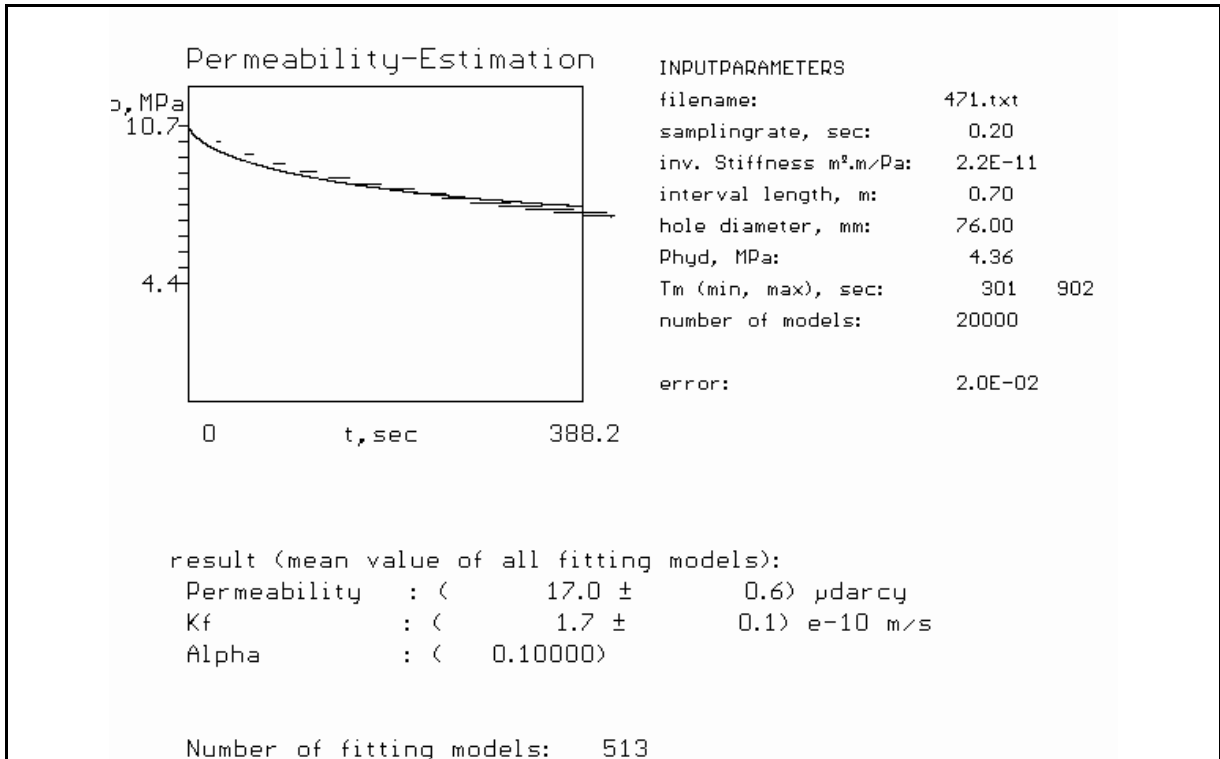
## TEST AT 416.0 m MD / 393.8 m TVD



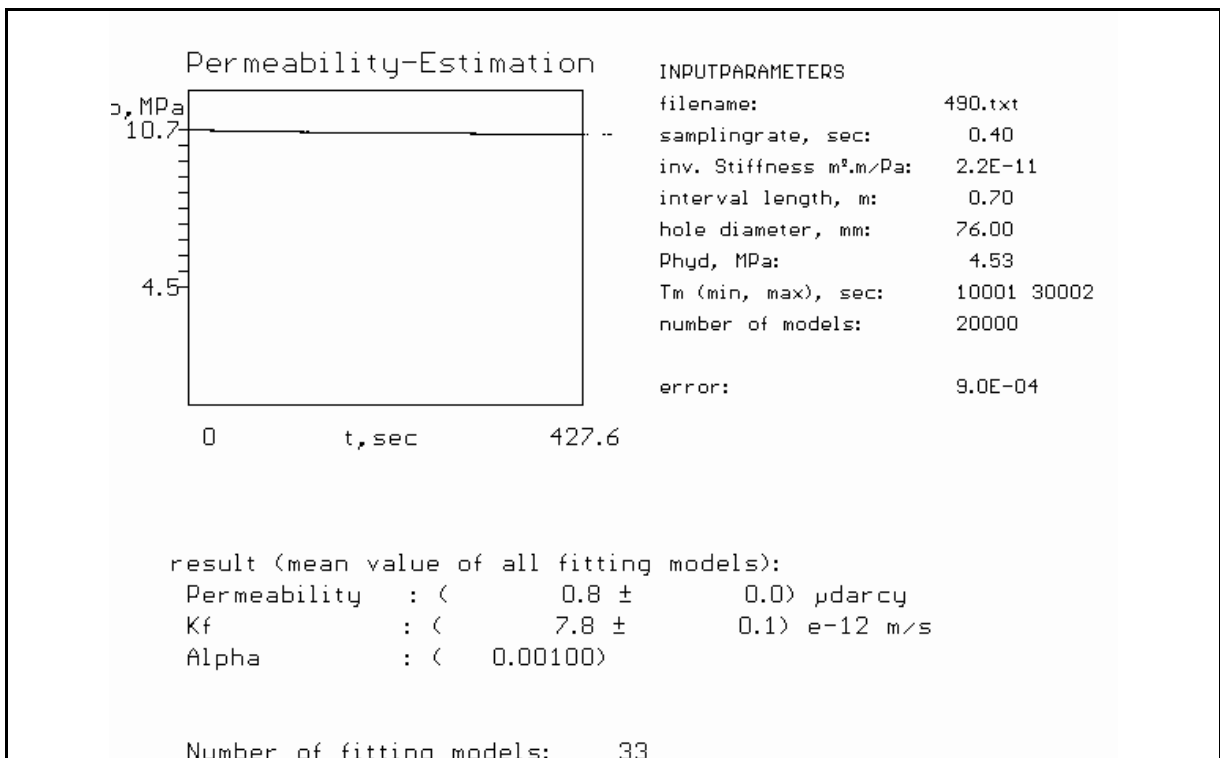
## TEST AT 417.7 m MD / 395.5 m TVD



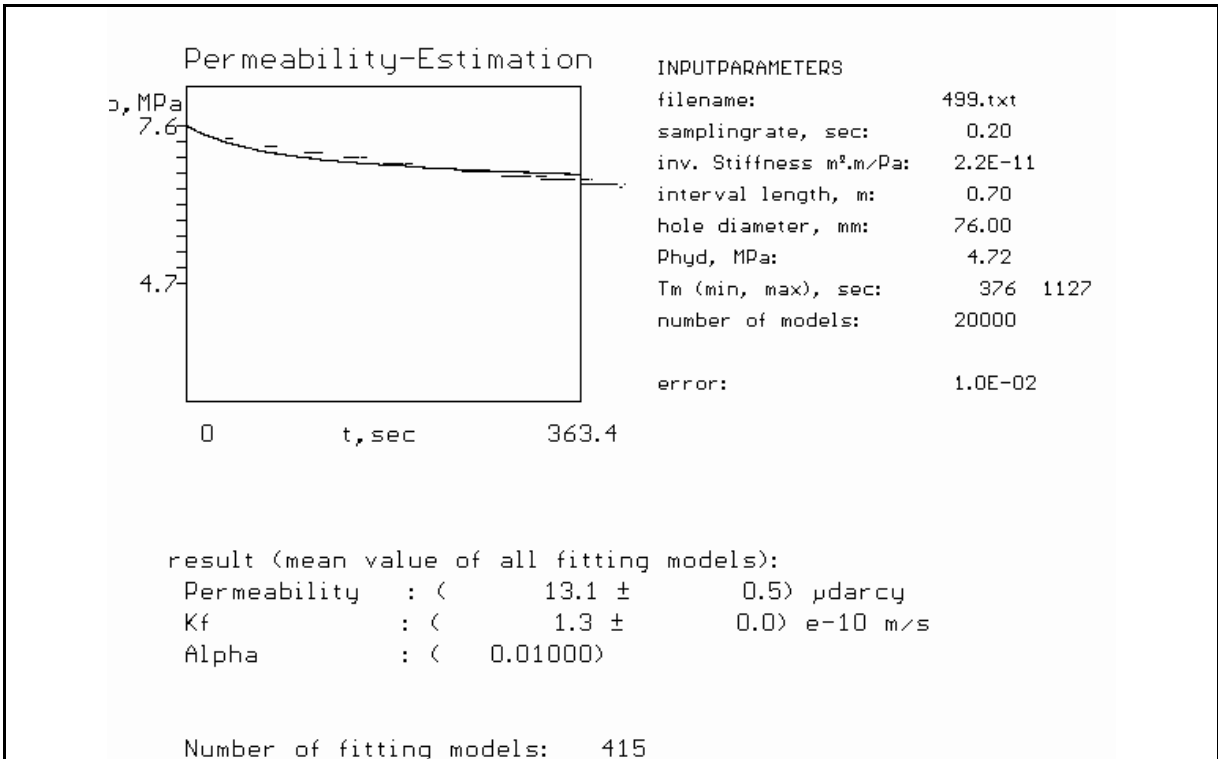
## TEST AT 471.0 m MD / 446.9 m TVD



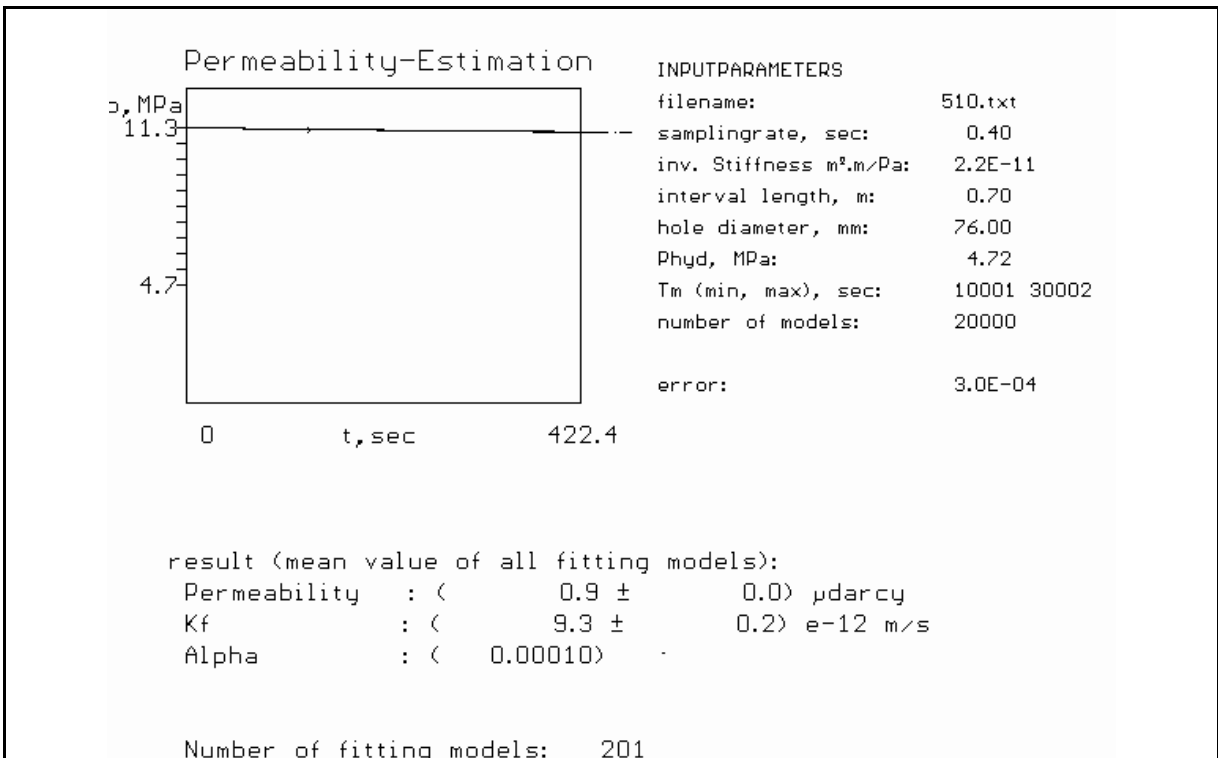
## TEST AT 490.5 m MD / 465.8 m TVD



## TEST AT 499.8 m MD / 474.8 m TVD

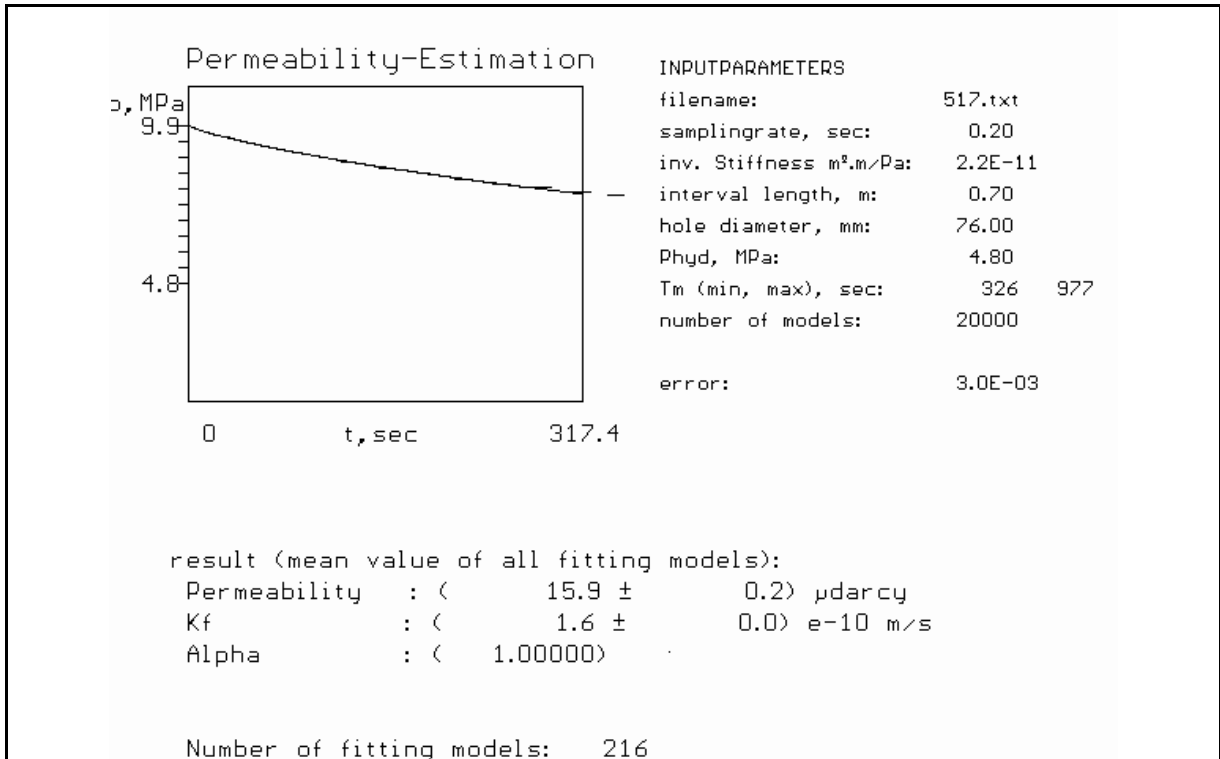


## TEST AT 510.8 m MD / 485.6 m TVD

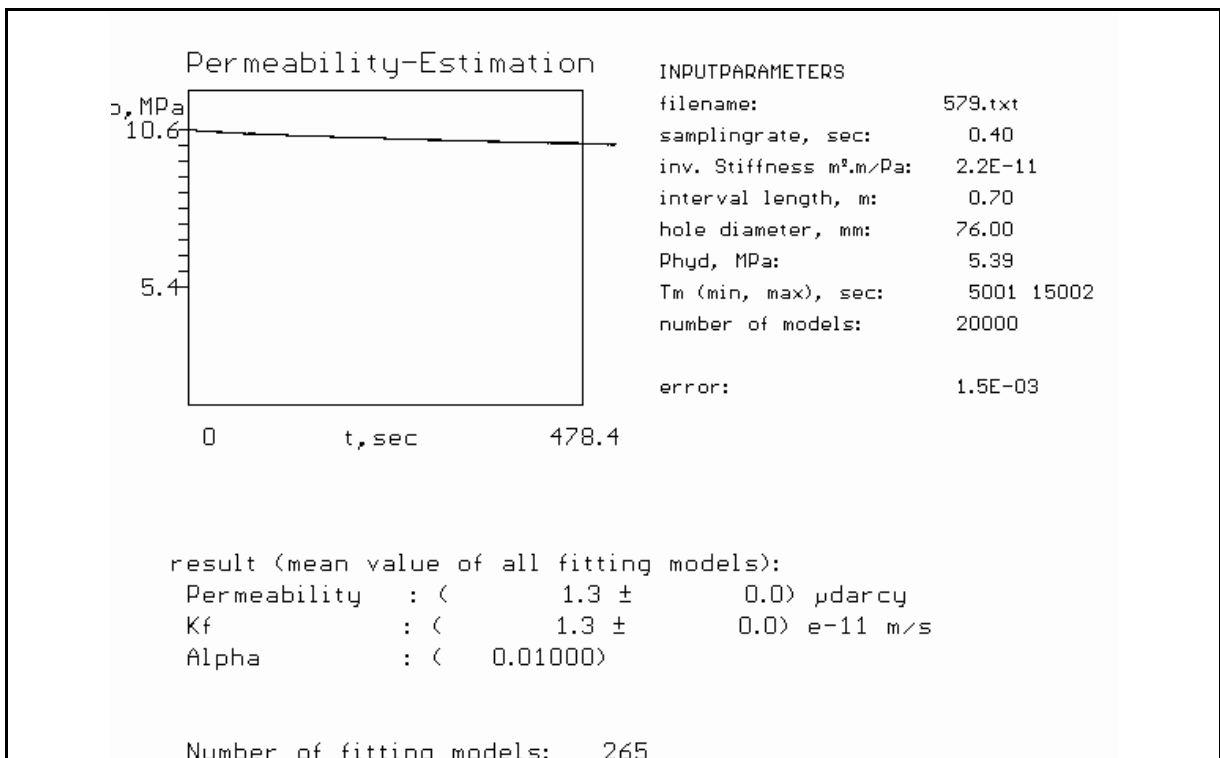




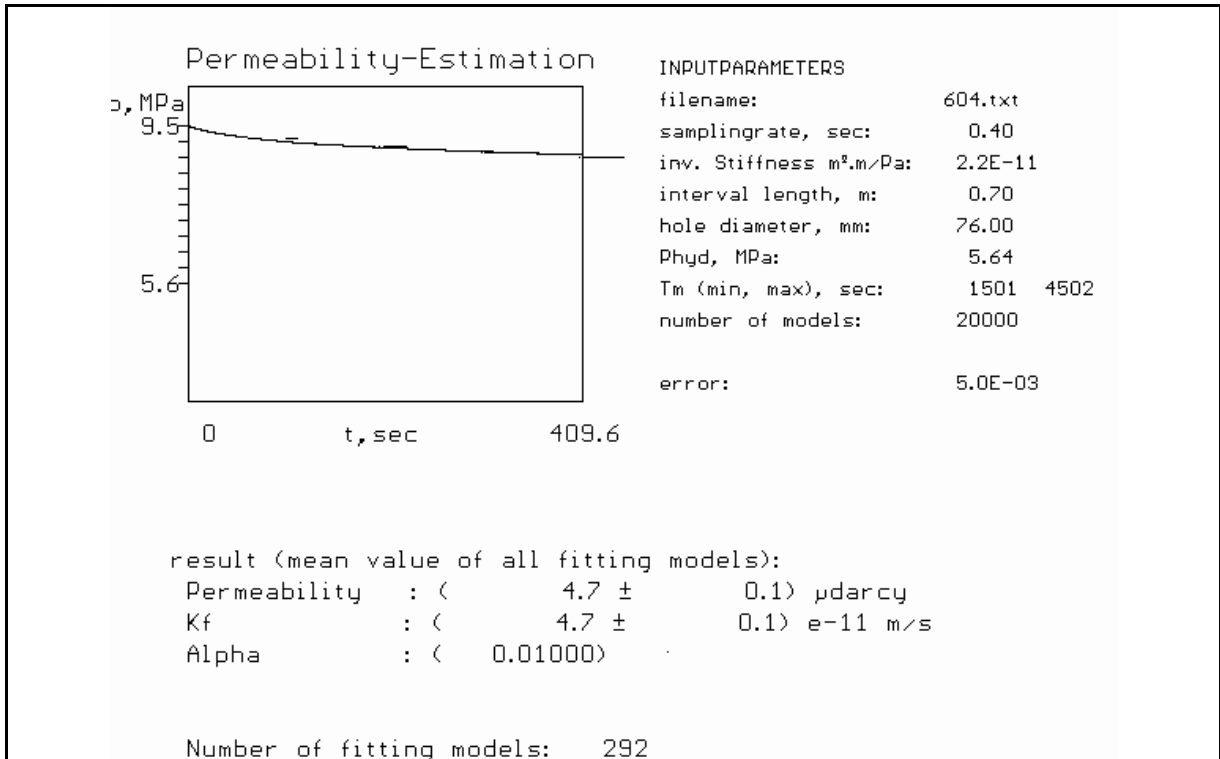
## TEST AT 517.0 m MD / 491.6 m TVD



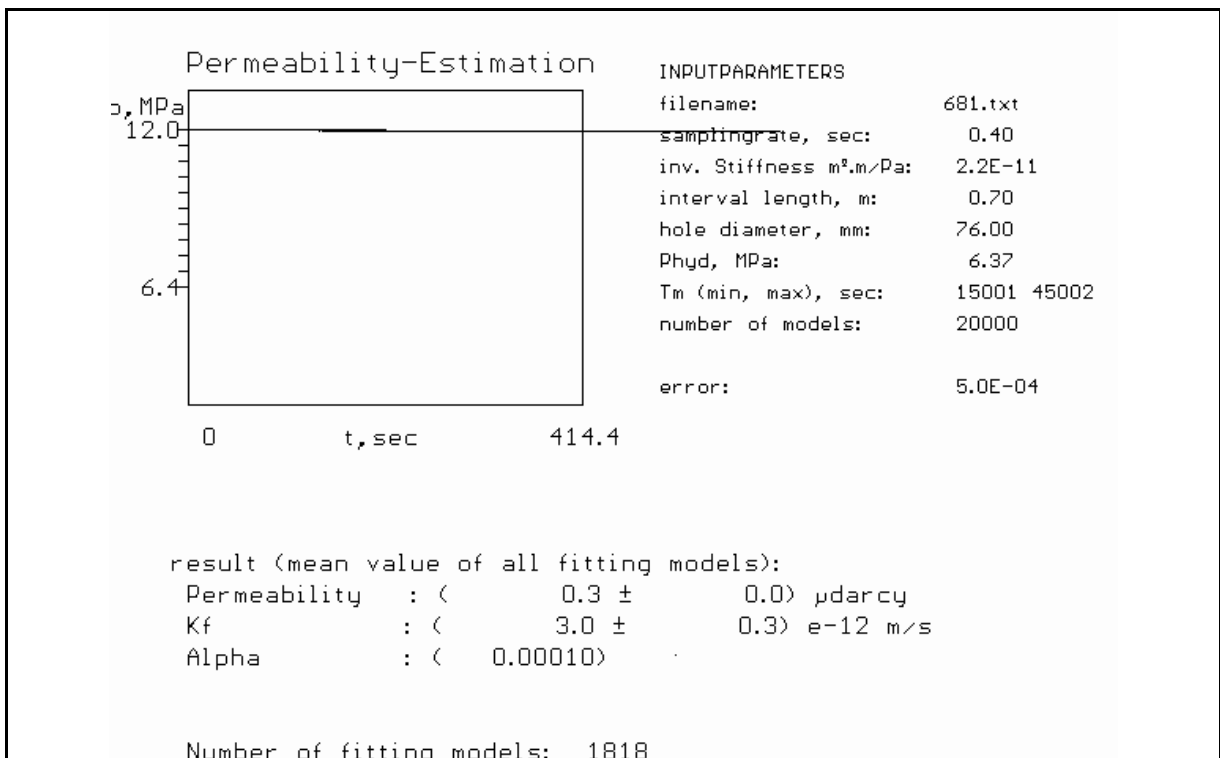
## TEST AT 579.0 m MD / 552.2 m TVD



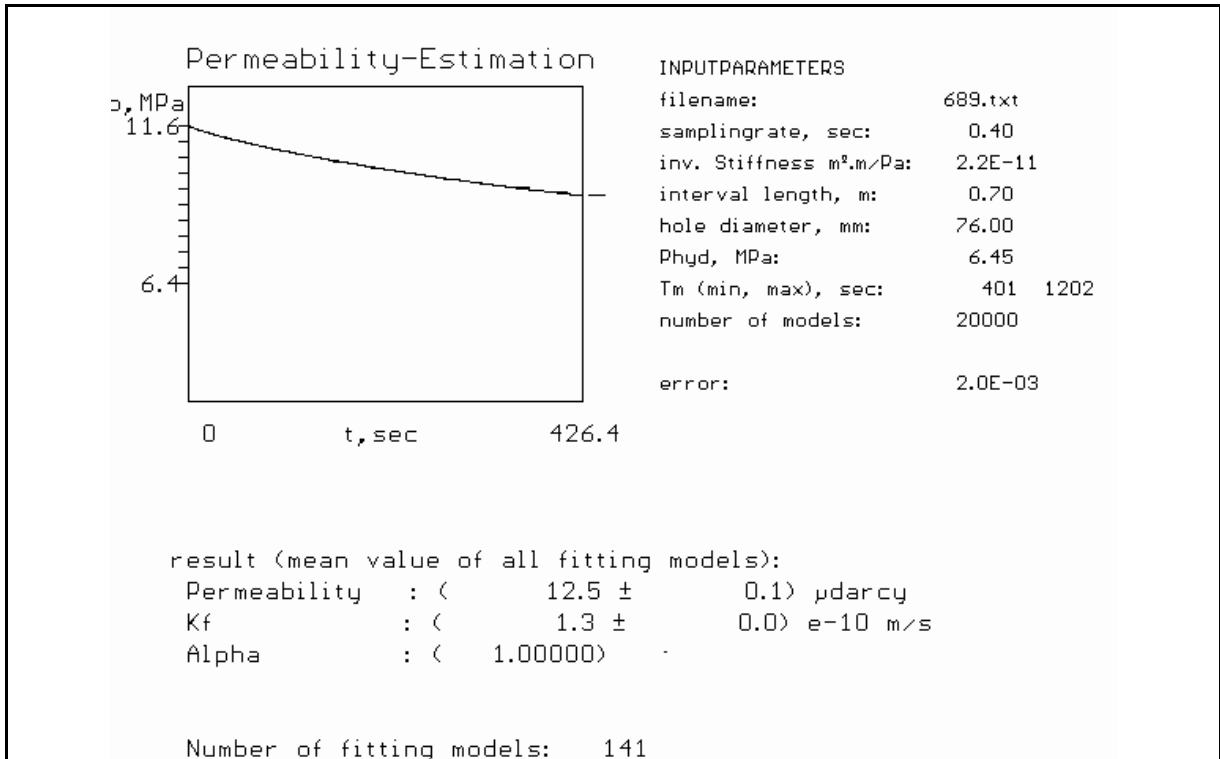
## TEST AT 604.2 m MD / 577.0 m TVD



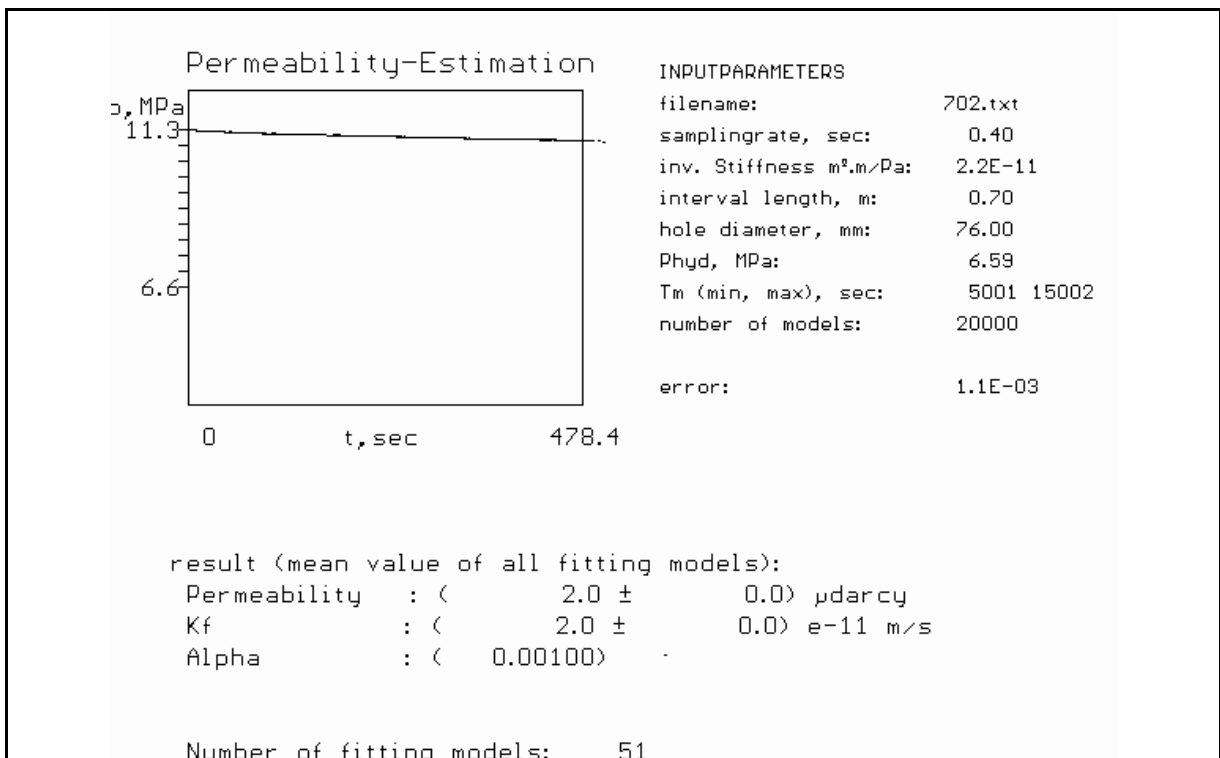
## TEST AT 681.0 m MD / 652.9 m TVD



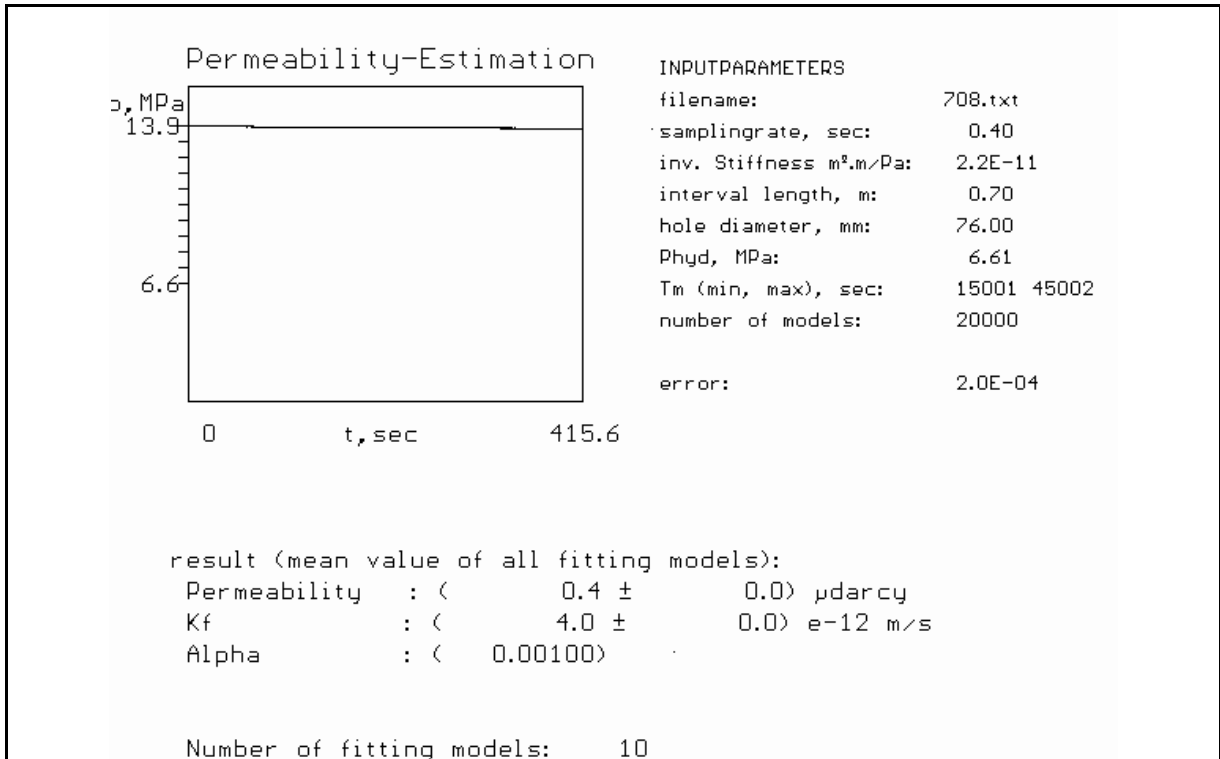
## TEST AT 689.5 m MD / 661.3 m TVD



## TEST AT 702.5 m MD / 674.2 m TVD



## TEST AT 708.0 m MD / 679.6 m TVD



## TEST AT 712.8 m MD / 684.8 m TVD

